PATHFINDER MINES CORPORATION SHIRLEY BASIN MINE

TAILINGS RECLAMATION

PLAN

OCTOBER 1993



VOLUME 1 SUA-442 DOCKET 40-6622

PATHFINDER

A Cogema Resources Company

September 23, 1999

Mr. Mohammad Haque
Uranium Recovery Branch
Division of Waste Management
Office of Nuclear Material Safety & Safeguards
Mail Stop T 7-J-8
U. S. Nuclear Regulatory Commission
11545 Rockville Pike
Rockville, MD 20852

Ref: Docket No. 40-6622

License No. SUA-442

Dear Mr. Haque:

Enclosed please find two current sets of the Shirley Basin Tailings Reclamation Plan as you requested. They represent the up-to-date version of the plan with all inserts. Note that any exhibits with a "9-99" revision date designation in the title block have been updated to reflect the five horizontal to 1 vertical reclaimed outslopes on the tailings embankments. Please call me if you have any questions.

Sincerely,

T. W. Hardgrove

Manager, Environmental and Regulatory Services

Enclosure



November 24, 1997

Joseph J. Holonich, Chief Uranium Recovery Branch Division of Waste Management Office of Nuclear Material Safety and Safeguards United States Nuclear Regulatory Commission Washington, D. C. 20555-0001

RE: Docket No. 40-6622, Source Material License SUA-442

Pathfinder Shirley Basin Tailings Reclamation

Dear Mr. Holonich:

Pursuant to your letter dated October 7, 1997, concerning hydrologic aspects of the tailings reclamation, Pathfinder Mines Corporation has developed the following information to address U.S. Nuclear Regulatory Commission staff concerns. Attachments include a report of the findings of an archaeological investigation, and a replacement Table 5-9, Figure 10-4 and page 5-37.

The following text addresses specific issues and comments directly. Reference is made to replacement pages where applicable.

Comment 1: Off-site Flooding, Marining's n and Cross Sections.

Response: The selected Manning's n of 0.02 was intended to reflect the extremely large flow depth and the nature of the floodplain. The intitial assumptions of PMP precipitation depth, PMP intensity and rangeland conditions were very conservative to remove all doubt concerning spring Creek PMP flood stage. The resulting maximum discharge of 112650 cfs would expand the conveyance area of the channel far beyond any identifiable floodplain. The Manning's n of 0.02 is considered representative of the primary spring Creek channel under flow depths in excess of 15 feet, and is also representative of the area above the primary channel where there is only sparse vegetation and limited sagebrush. Increased Manning's n values of 0.03 or 0.04 would be more applicable for a less severe storm where the relative roughness of elements on the floodplain is much greater. If the Manning's n is increased to 0.03 for the critical Spring Creek section, the maximum flood stage of 22 feet is still more than 150 feet from the covered tailings. At a Manning's of 0.04, the maximum flood



Joseph J. Holonich Page 2 November 24, 1997

stage of 23.9 feet is at least 50 feet from the covered tailings. The increasing Manning's n values are considered a very conservative approach to estimating flood stage for an already conservative flood discharge. Increasing Manning's n reduces the flow velocities.

The two cross sections were taken from the most constricted area of Spring Creek in the vicinity of the tailings to give conservative flood stage estimates. The top width of flow for the PMF discharge is in excess of 1000 feet for a drainage with a primary channel width of just a few feet. The use of two cross sections to represent the channel and channel slope is consistent with the scale of the PMF stage and flow area in the region of concern. The flow depth is so large that the general floodplain slope is more applicable than the bed slope of the primary channel. Cross sections outside of the constricted area will not have critical flood stages that could approach the covered tailings and were not used for this reason. The slope of the floodplain at the area of the constriction is just slightly greater than the slope of 0.003 ft./ft. that was used in the computations. The slope of the floodplain on either side of the constriction ranges from 0.005 ft./ft. to 0.01 ft./ft. which is significantly greater than that used in the computations. If the slope in the computations is increased, the flood stage will decrease, and the flow velocity will increase, which negates much of the effect of increasing Manning's n.

Comment 2: Lateral and Vertical Stability of Receiving Streams

Response: An archaeological investigation of the Spring Creek channel has revealed that the location of the limits of the floodplain has not changed in over 4800 years. The report of this investigation (three copies) is included in this submittal. Aerial photos and on-site observation have revealed that both channels have reached a level of maturity where there is only lateral meandering within a distinct floodplain. There are no significant headcuts or other evidence of vertical instability.

The reduction in drainage area for the total Spring Creek drainage at the tailings area is roughly 8% with some additional reduction occurring downstream of the tailings area. This reduction occurs far downstream of the springs on Spring Creek and Fox Creek which are the sources of perennial flow. Thus the reduction in



Joseph J. Holonich Page 3 November 24, 1997

drainage area will have little or no impact on sustaining vegetation. The flow attenuation characteristics of the tailings drainage design will also slow release of runoff to Spring Creek, further enhancing stability.

The methods used to evaluate the scour were recommended by NRC personnel, and included two methods intended for continuous flow. The third method had provisions for using a flow duration. The design discharges for the computations were taken from a PMF and represent an extremely severe short duration and highly improbable event. The use of a continuous flow scour prediction methodology is obviously a misapplication, but it should be an extremely conservative approach. When the method incorporates a flow duration, the severity of the PMF discharges lends a tremendous degree of conservatism. In addition, the methods were originally intended for discharges from culverts, which is a more radical transition in channel configuration than release from a riprap channel.

In addition to the scour predictions, a calculation of stable grade for the section of the channel downstream of the end protection should also be indicative of the maximum plausible scour depth. With the exception of a small and largely artificial discharge to the Mine Creek channel, the receiving drainage systems are highly ephemeral. The discharge to Mine Creek is supported by recharge systems which are operated as a portion of the ground-water restoration systems. The two channels of primary concern are those that drain to Spring Creek, and these are Basin 5-4 and Channel N. Under the design reclamation plan, Basin 5-4 drains to Mine Creek with a dramatically reduced drainage area. If the scour does reach the depth of the structure of 4.5 feet, the Mine Creek channel will have a relief of approximately 6 feet in over 1700 feet of length, or a slope of less than 0.4%. Channel N drains to a tributary of Spring Creek north of the tailings. The relief between the Channel N outlet and the confluence of the tributary and Spring Creek is roughly 28 feet over a distance of approximately 1300 feet. The design reclamation plan increases the measured drainage area to the tributary from 77 acres to approximately 133 acres, although the use of surge pond storage in the drainage design will likely reduce peak discharge substantially from the pre-mining condition. If the depth of the end protection structure is increased from 4.5 feet to 15 feet, (and the depth of scour reaches the base of the end protection structure) then the resulting channel slope is 1%. The



Joseph J. Holonich Page 4 November 24, 1997

tributary channel is currently stable at a slope of approximately 2.2%, and a reduction of slope to 1% should be more than adequate. Table 5-9 and Figure 10-4 have been revised to reflect these changes.

Please don't hesitate to contact me if you have questions or comments.

Sincerely,

T.W. Hardgrove

Coordinator of Mine Environmental Affairs

Enclosure: As stated

cc: E. L. Nugent

J.D. Wadsworth R.W. Poyser

Hydro-Engineering, LLC C. Cain, USNRC - Region 4

NOTE: Archaeological report is not included in copies.

REPLACEMENT AND ADDITIONAL PAGES AND EXHIBITS

The following table itemizes the enclosed revisions to the Shirley Basin Tailings Reclamation Plan and indicates the replacement pages for the document currently on file with the NRC.

Volume I Revisions

New Text/Exhibit

Replace(s)

Volume I Cover Page

Volume I Cover Page

Page 38

Page 38

Table 5-9, page 5-80

Table 5-9, page 5-80

Volume II Revisions

New Text/Exhibit

Replace(s)

Volume II Cover Page Figure 10-4, page 10-25 Volume II Cover Page Figure 10-4, page 10-25

The text is copied double-sided and, in many cases, the unmodified adjacent page is replaced along with the modified page to preserve the numbering sequence. The revision date for each page is located in the lower right hand corner of the page.

PATHFINDER

A Cogema Resources Company

September 10, 1997

Mr. Joseph J. Holonich, Chief Uranium Recovery Branch Division of Waste Management Office of Nuclear Material Safety & Safeguards U.S. Nuclear Regulatory Commission Washington, D.C. 20555

Re: Docke

Docket No. 40-6622 License No. SUA-442 Shirley Basin Mill

The attached comments and modified portions of the Tailings Reclamation Plan (five copies of each) are submitted in response to your letter dated May 12, 1997, concerning the Geotechnical Design and Radon Attenuation. The Soil Cleanup Verification and Sampling Plan was addressed in an earlier submittal. Comments 1 and 2 concerning the geotechnical design are addressed by the enclosed letter from Inberg-Miller Engineers. The cost estimate in reponse to Geotechnical Design Comment 3 is not yet complete, and it will be submitted to the NRC as soon as it is available. The balance of the comments are addressed by responses from Hydro-Engineering. Only the Hydro-Engineering responses include specific text or exhibit changes to the reclamation plan.

Please call me if there are any questions.

Sincerely,

T. W. Hardgrove

Coordinator of Mine Environmental Affairs

Enclosures

cc:

E. L. Nugent

Tom Farfor

J. D. Wadsworth

R. W. Poyser

P. Mackin, CNWRA

INBERG-MILLER ENGINEERS

1120 EAST "C" STREET

CASPER. WYOMING 82601-2195

307-577-0806

September 5, 1997

7294-CX

Man Park

Pathfinder Mines Corporation P. O. Box 730 Mills, WY 82644

ATTENTION: THOMAS HARDGROVE

RE: RESPONSE TO N.R.C. STAFF REVIEW COMMENTS

PATHFINDER MINES CORPORATION RECLAMATION PLAN

SHIRLEY BASIN URANIUM MILL SITE

SHIRLEY BASIN, WYOMING

Gentlemen:

This letter summarizes our response to the NRC's staff review comments (items No. 1 and 2) regarding the slope stability and liquefaction potential analyses for Dams No. 4 and 5 at the above-referenced project. The NRC is requesting additional information regarding: 1) documentation of soil parameters used in the slope stability analyses, and 2) additional consideration of the methodologies used in the stability analyses. More specifically, an evaluation of the stability analyses is requested regarding: i) the effects of variations associated with the geometry of the slopes, soil and rock parameters, and pore pressures acting within the slopes, and ii) effects of the assumptions inherent in the Modified Bishop's Method. In addition, further discussion of the liquefaction potential and the selection of soil parameters related to: i) material strength loss, and ii) pore pressure development due to dynamic loads.

DISCUSSION OF SOIL PARAMETERS

The soil parameters used in the stability analyses were obtained from information contained in Pathfinder Mine Corporation's (PMC) reclamation plan prepared by Hydro-Engineering, Dames and Moore's reports on the design and redesign of No. 5 Tailings Dam, and estimated values. The soil parameters for the various soil types are discussed below.

Sand Cover

The unit weight of the sand cover material was determined from Table 3-2 of the reclamation plan. Section 4.2 of the reclamation plan identifies Sample STH-7 as a typical material for use as the sandy cover materials. Table 3-2 lists the value of 95 percent of the maximum dry density to be 104.5 pcf. The moist unit weight was determined by using a moisture content of 6 percent as identified within Section 4.2. The cohesion value was conservatively assumed to be zero, even though Sample STH-7 is classified as a sandy, fat clay. Based on our experience at the Petrotomics Mine

Pathfinder Mines Corporation ATTENTION: THOMAS HARDGROVE September 5, 1997 Page Two

DISCUSSION OF SOIL PARAMETERS, Continued

Sand Cover, Continued

located adjacent to the Pathfinder Mine, surficial sandy soils that would likely be suitable for use as the sand cover materials contain less clay and exhibit significant angularity. Likewise, the phi value is based on these same observations and is assumed to be 35 degrees, a typical value of dense sand. It is recognized that if the clay content was increased, phi values would decrease. However, cohesion values would also increase with a greater clay content. Thus, the change within the shear strength of the soil would be minimized.

Clay Cover

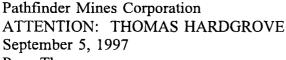
The moist unit weight of clay cover materials is, in our opinion, conservatively assumed to be 100 pcf. This value is based on the average of the 95 percent of maximum dry density for samples STH-2, STH-4, STH-5, STH-6, and STH-11, identified as typical clay cover materials in the reclamation plan, at a moisture content of 12 percent. The moisture content is conservatively lower than the estimated long term moisture content of 16.6 percent. The phi angle is conservatively assumed to be zero, even though these clay soils appear to contain some sand. An assumed value of 500 psf was used for the cohesion value. We believe this to be a conservative value for cohesion based on typical values for compacted lean (CL) and fat (CH) clays listed within NAVFAC Design Manual 7.2, May 1982, of 1800 psf and 2150 psf, respectively.

Sand Tailings

The moist unit weight of 105 pcf is based on the value used in the radon computer model in the reclamation plan of 1.59 gm/cm³ (99.3 pcf) dry density at 6 percent moisture. The average dry unit weight and natural moisture content of samples TW4-1B, TW4-4B, TW4-5B, TW4-1C, and TW5-2B, noted as characteristic samples of sand tailings, was 90 pcf and 23 percent, respectively, resulting in a moist unit weight of 111 pcf. Because long term moisture is likely to be considerably less than that observed in the samples the value for unit weight used within the radon model was considered most appropriate. Because little data is available on the strength characteristics of sand tailings, the cohesion and phi values were estimated from typical values of saturated clayey and silty sands.

Slime Tailings

The moist unit weight of 100 pcf is based on the average dry unit weight of samples TW4-2C and TW4-3C at an average moisture content of 71 percent. Based on conversations with Tom Michel of Hydro-Engineering and review of lab results, the slime tailings contains a significant amount of sand tailings. Therefore, phi and cohesion values were estimated from values typical of sandy clays. Due to the hydraulically



Page Three

DISCUSSION OF SOIL PARAMETERS, Continued

Slime Tailings, Continued

placed nature of the slime tailings, the cohesion value was significantly reduced below typical values for saturated sandy clays. The values are consistent with typical values of slime tailings contained in Steven G. Vick's "Planning, Design and Analysis of Tailings Dams" and a report entitled "Characterization of Inactive Uranium Tailings Sites: Shiprock, New Mexico" prepared for the Department of Energy, Albuquerque, New Mexico, written by the Geotechnical Engineering Program, Colorado State University, 1984.

Dam No. 4 Embankment

The moist unit weight of 115 pcf is based on the average values of samples obtained in borings performed by Inberg-Miller Engineers. The phi value is conservatively assumed to be zero and the cohesion value is based on the unconfined compressive strength test performed on Sample B-2-3.

Dam No. 4 Foundation Soils

The density and strength parameters for the upper and lower foundation soils are based on the results from laboratory testing of samples collected from borings performed by Inberg-Miller Engineers. The unit weight of 125 pcf for the lower foundation is based on testing on Sample B-2-14 and the cohesion of 2000 psf is based on the unconfined compressive strength of the same sample. Based on standard penetration testing and pocket penetrometer tests, the upper foundation soils exhibit a lower relative density and likely have a lower cohesion value. The phi value was conservatively assumed to be zero, and the cohesion estimated to be half of that of the lower foundation, or 1000 psf.

Dam No. 5 Embankment, Drain and Foundation Soils

The moist unit weight for the embankment and the drain are based on 95 percent of the maximum dry density as determined from AASHTO T-180 (modified proctor). The unit weight for the foundation is based on the average value of samples collected within a boring performed by Dames and Moore. The phi and cohesion values are based on the results of direct shear tests performed on recompacted clayey and silty sand samples for the embankment, recompacted sand tailing for the drain, and undisturbed clayey and silty sands for the foundation.

It is recognized that the foundation materials are variable as observed within the test boring performed by Dames and Moore for the design of Dam No. 5. Therefore, it is important to determine the foundation soils with the lowest shear strength. Based on Dames and Moore's test results, the maximum shear strength of the sandy foundation soils is less than the minimum



Pathfinder Mines Corporation ATTENTION: THOMAS HARDGROVE

September 5, 1997

Page Four

DISCUSSION OF SOIL PARAMETERS, Continued

shear strength of the clayey foundation soils. In light of this, Dam No. 4 was reanalyzed for the purpose of this response using the foundation soils from Dam No. 5. The results of the analysis are tabulated below, and plots of the analyses are enclosed.

Modified Dam No. 4 Factors of Safety		
Ground-Water Conditions	Static Factor of Safety	Seismic Factor of Safety
High	4.25	1.76
Low	4.04	2.13

It can be observed that the soil parameters used within the original analysis of Dam No. 4 yield more conservative results than that based on more rigorous laboratory defined samples used for the analysis of Dam No. 5. Therefore, in our opinion, the results from the soil parameters used in the previous report are conservative for the various stability analyses.

DISCUSSION OF SLOPE STABILITY ANALYSES

Effects of Variations in Embankment Parameters

In order to evaluate the conservative nature of variations in soil parameters, it is important to consider the results of the modified Dam No. 4 analysis discussed previously. Since the shear strength of sandy soils is less than the clayey soils, using the sandy foundation soils would result in the most conservative results. The Dames and Moore reports state that in the original design for Dam No. 5 the maximum shear strength of the sandy foundation soils is 4900 psf. However, from the original analysis of Dam No. 4, it can be observed that the bottom of failure surface for the high water seismic analysis is tangent to an elevation at approximately 7050. Shear strength at this depth is approximately 1300 psf. Any variations of soil parameters resulting in a shear strength greater than 1300 psf would exhibit conservative results.

Variations in geometry for Dam No. 4 would not effect stability analyses significantly because the critical failure surfaces are controlled by the shear strength of the foundation soils. However, variations in the geometry for Dam No. 5 would affect stability results. The stability analysis for Dam No. 5 was performed on the maximum cross-section. The most critical failure condition occurs at high water conditions under seismic loading. Additional stability analysis event at a downstream slope of 3:1 instead of the previously analyzed 4:1 slope, results in minimum factor of safety of 1.10. Additionally, we understand that the final reclamation plan, as currently being revised, flattens the down stream slope to 5:1 which would result in more conservative results than the 3:1 and 4:1 analyzed slopes.

Pathfinder Mines Corporation ATTENTION: THOMAS HARDGROVE

September 5, 1997

Page Five

DISCUSSION OF SLOPE STABILITY ANALYSES, Continued

Discussion of Stability Method Used

In our opinion, the modified Bishop's Method yields conservative results for both static and pseudo-static analyses. Due to both embankments being constructed of generally cohesive materials, it is reasonable to assume that a circular failure surface would occur within the embankments. The modified Bishop's Method is based on moment equilibrium. However, it is also useful to evaluate force equilibrium methods to compare the conservatism of the analyses. The most common force equilibrium method is the Janbu Method of analysis. The following tables summarize the minimum factors of safety based on the Janbu Method.

Dam No. 4 - Minimum Factors of Safety _(Janbu)			
Ground-Water Conditions	Static Factor of Safety	Seismic Factor of Safety	
High	3.51	1.26	
Low	3.71	1.40	

Dam No. 5 - Minimum Factors of Safety _(Janbu)		
Ground-Water Conditions	Static Factor of Safety	Seismic Factor of Safety
_. High	2.22	1.11
Low	2.49	1.26

Evaluation of Liquefaction Potential

While no specific laboratory tests were performed on samples of the embankment or foundation soils to determine liquefaction potential, it is our opinion that there exists a low potential for volume or strength changes to occur under seismic loading. As discussed in our November 15, 1996 Report, this conclusion is based on the type of materials that make up the dams, foundations and tailings, and the relatively low seismic potential of the area. As discussed earlier, we believe that by assuming relatively low values of soil strength to the static and psuedo-static analyses, we have provided a reasonable basis of evaluating a possible reduction in soil shear strength.

Pathfinder Mines Corporation ATTENTION: THOMAS HARDGROVE

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DISCUSSION OF SLOPE STABILITY ANALYSES, Continued

Evaluation of Pore Pressure Development

In order to evaluate variations in pore pressure within the embankments, it is important to consider conditions which would result in the greatest pore pressures. The greatest pore pressures typically occur upon rapid drawdown conditions, in which high water conditions occur within the dam and low water conditions exist outside of the embankment. Results of rapid drawdown analyses for each dam are tabulated below:

Rapid Drawdown Conditions- Minimum Factors of Safety			
Seismic Conditions	Dam No. 4	Dam No. 5	
0.2 g	1.53	1.27	
none	4.10	2.56	

The rapid drawdown conditions for Dam No. 5 were analyzed for a 4:1 downstream slope. The proposed 5:1 downstream slope would in our opinion yield even more conservative results. We also performed rapid drawdown analyses using Janbu, presented below.

Rapid Drawdown - Minimum Factors of Safety _(Janbu)		
Seismic Condition	Dam No. 4	Dam No. 5
0.2 g	1.40	1.14
none	3.71	2.25

As the dewatering of the tailings basins continue over time, the potential for pore pressure development to occur is reduced, as saturated soil conditions are replaced with unsaturated conditions. We have analyzed this unsaturated condition for both dams under the long-term conditions assumption of a low steady-state ground-water level.

Pathfinder Mines Corporation ATTENTION: THOMAS HARDGROVE September 5, 1997

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DISCUSSION OF SLOPE STABILITY ANALYSES, Continued

Selection of Seismic Coefficient

The stability analyses performed for our November 15, 1996 utilized a seismic coefficient of 0.2g, based on a review of the information available at the time. Since the date of the report, we have received a report from James C. Case, Head of the Geologic Hazards Section of the Wyoming Geological Survey. In that report, Mr. Case analyzes the seismic potential for the site, and concludes that the maximum credible "floating" earthquake for the site would have a magnitude of 6.25, with a peak horizontal acceleration of 0.15g. Because we believe that our existing analysis provides a reasonable assessment of the site, we do not recommend that the stability analysis be rerun with the lower acceleration. However, it should be recognized that by using the higher acceleration of 0.2g in our stability analyses, we believe that our overall approach to the seismic stability of the site is relatively conservative. A copy of Mr. Case's report is enclosed for your information.

We appreciate the opportunity to provide this additional information regarding slope stability analysis of the embankments. If you have any questions with the contents of this letter or enclosures, please call.

REVIEWED BY:

Steven F. Moldt, P.E.

Vice President

Sincerely,

INBERG-MILLER ENGINEERS

Shawn D. Steiner, P.E.

Geotechnical Engineer

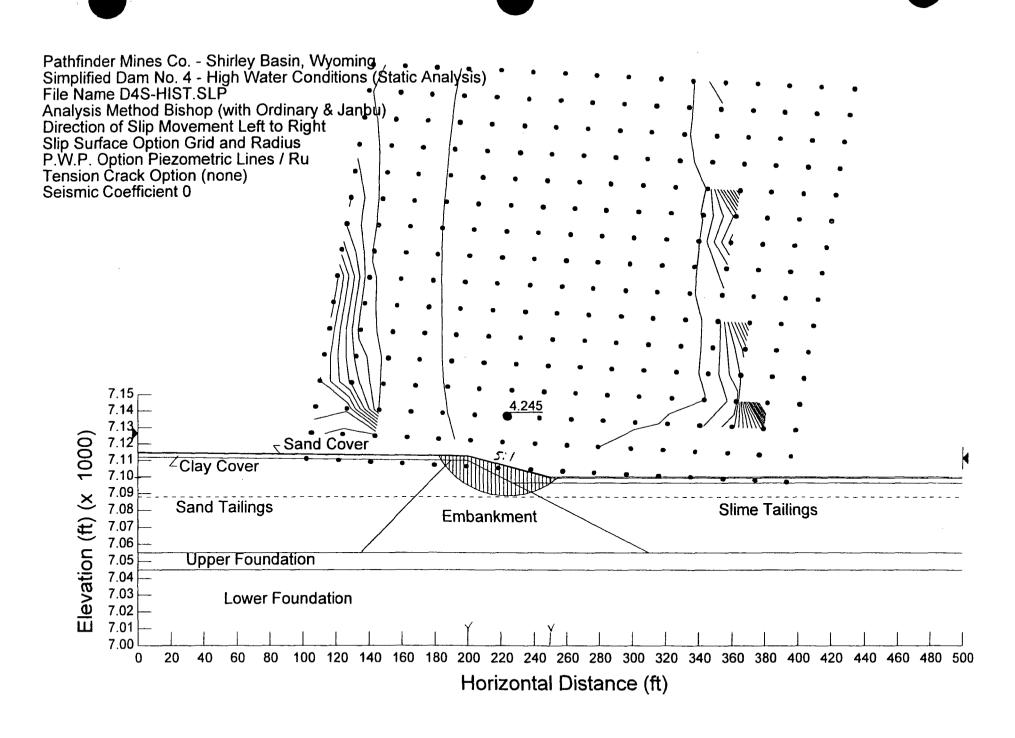
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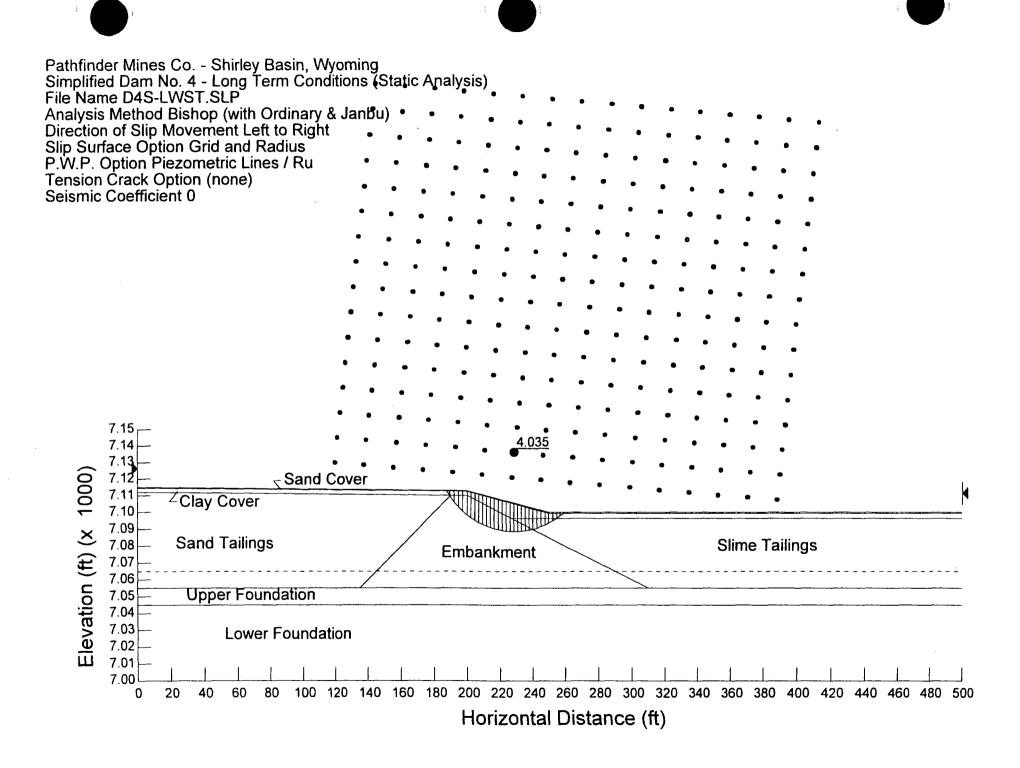
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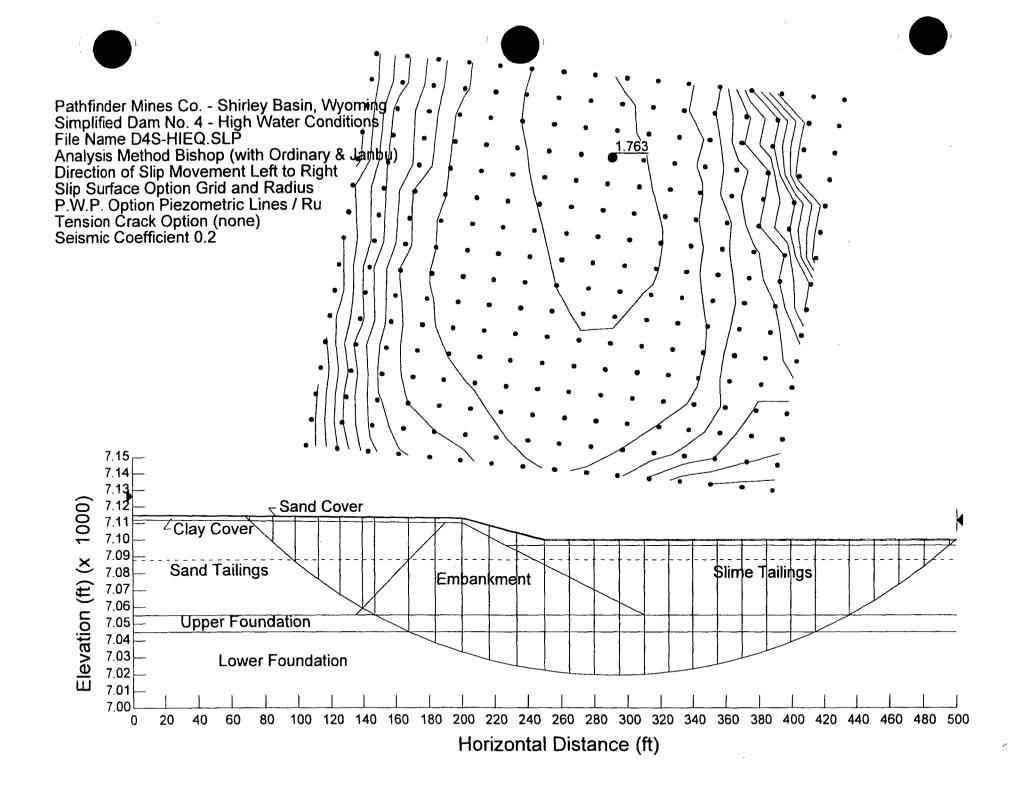
Enclosures: Slope Stability Analyses

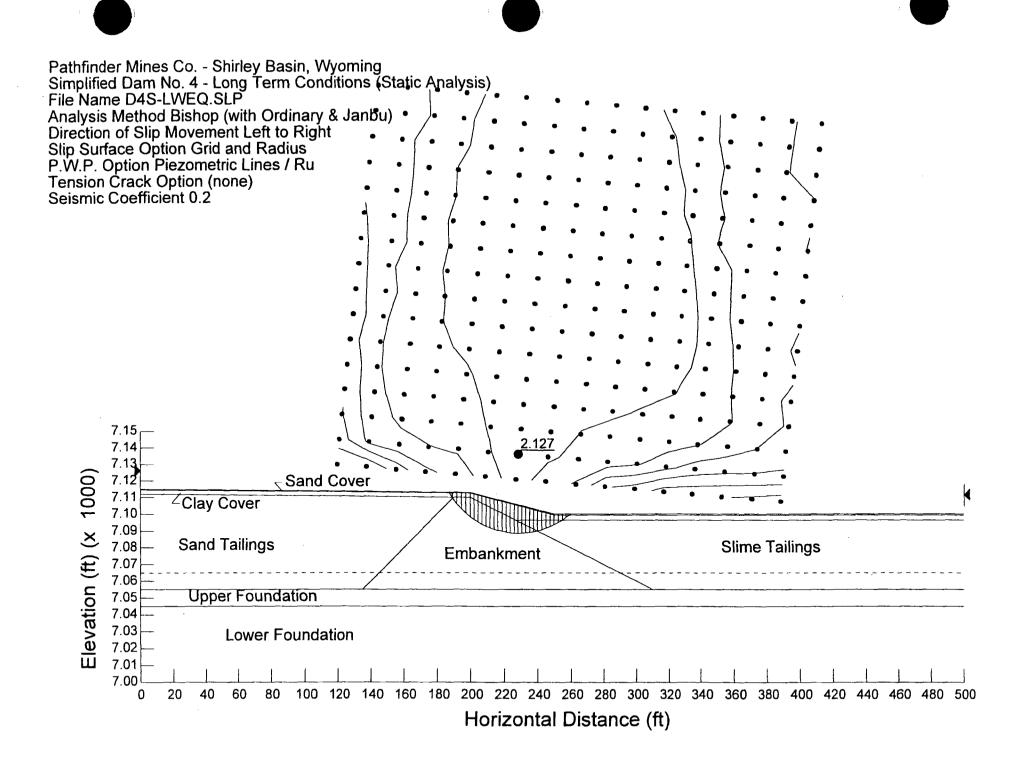
Copy of James C. Case August, 1997 Report

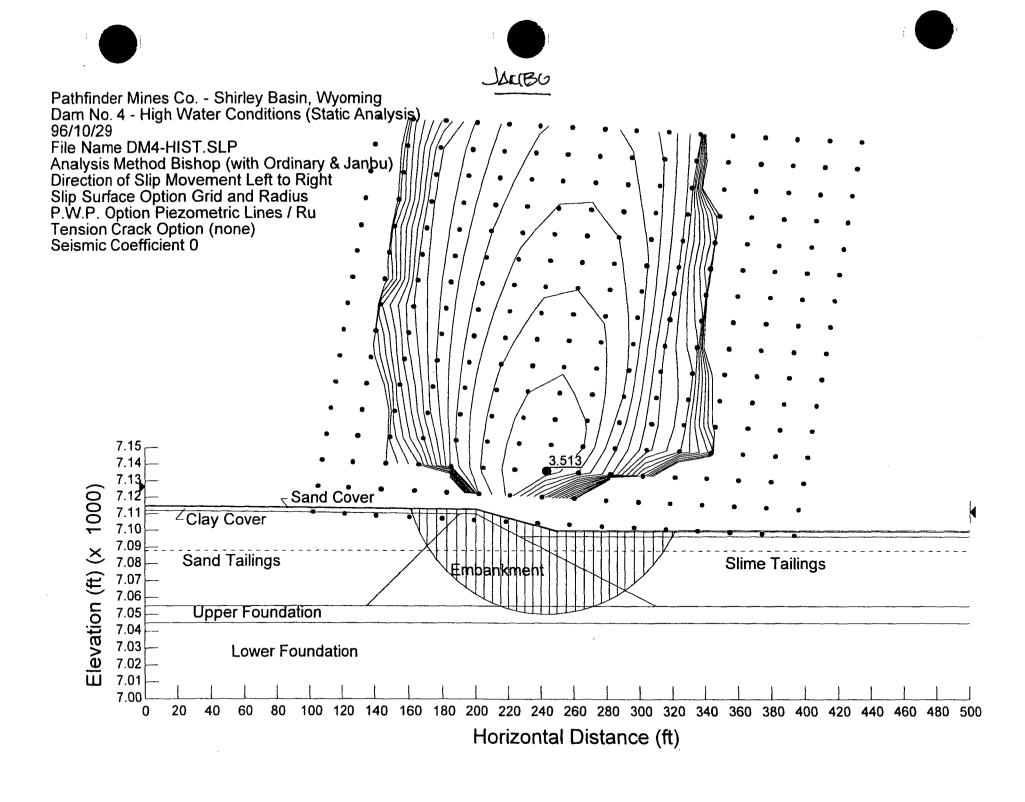
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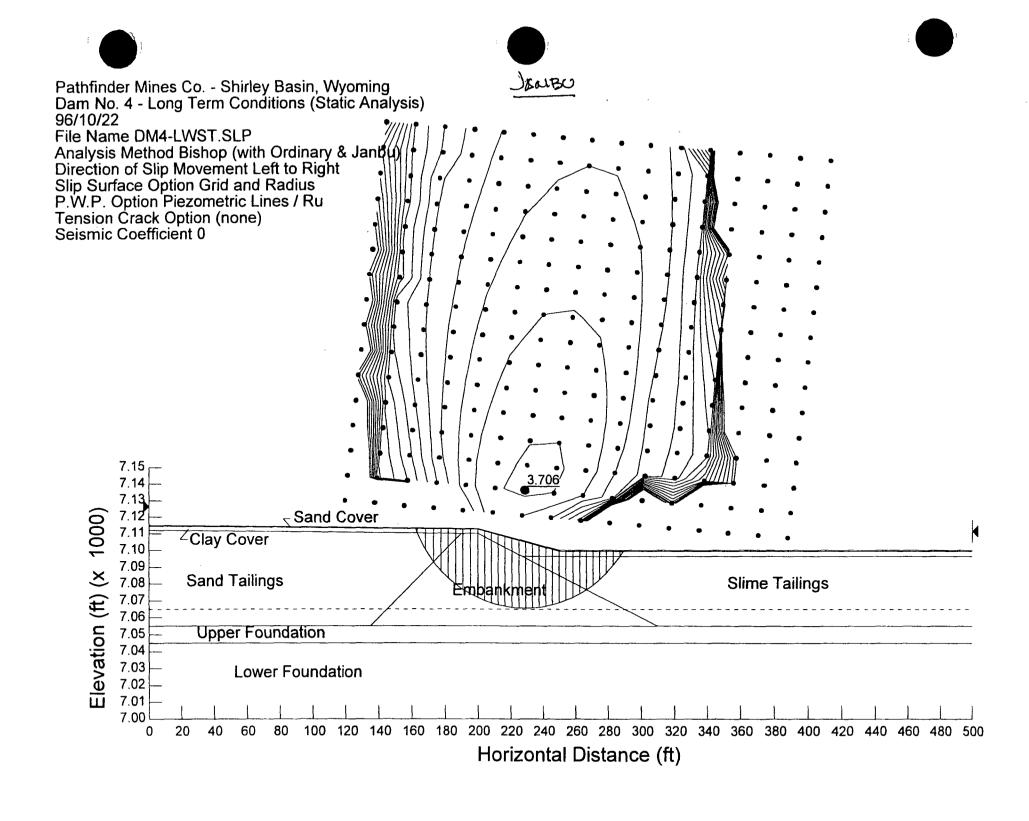


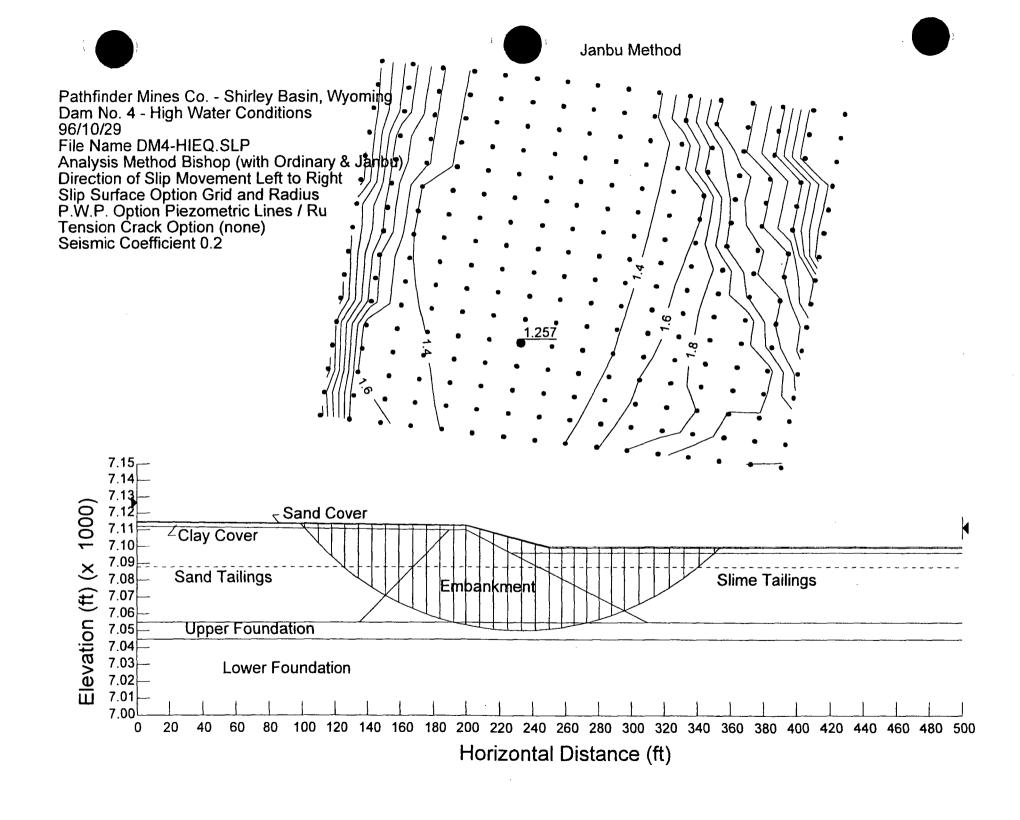


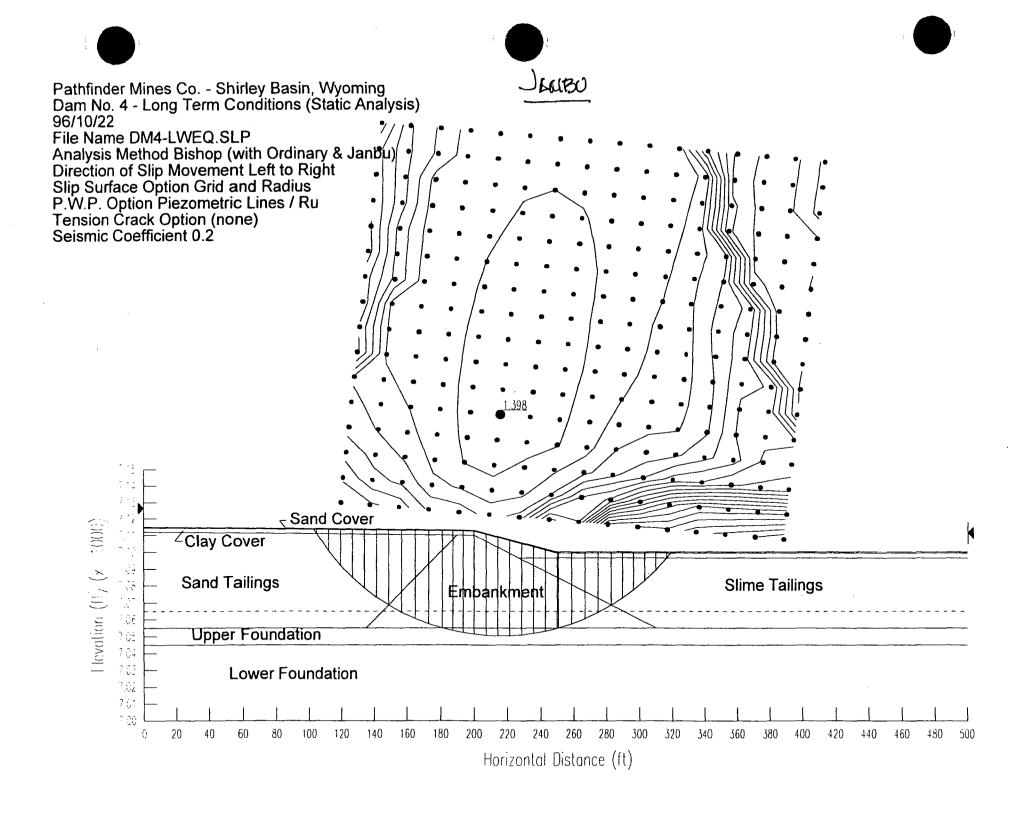


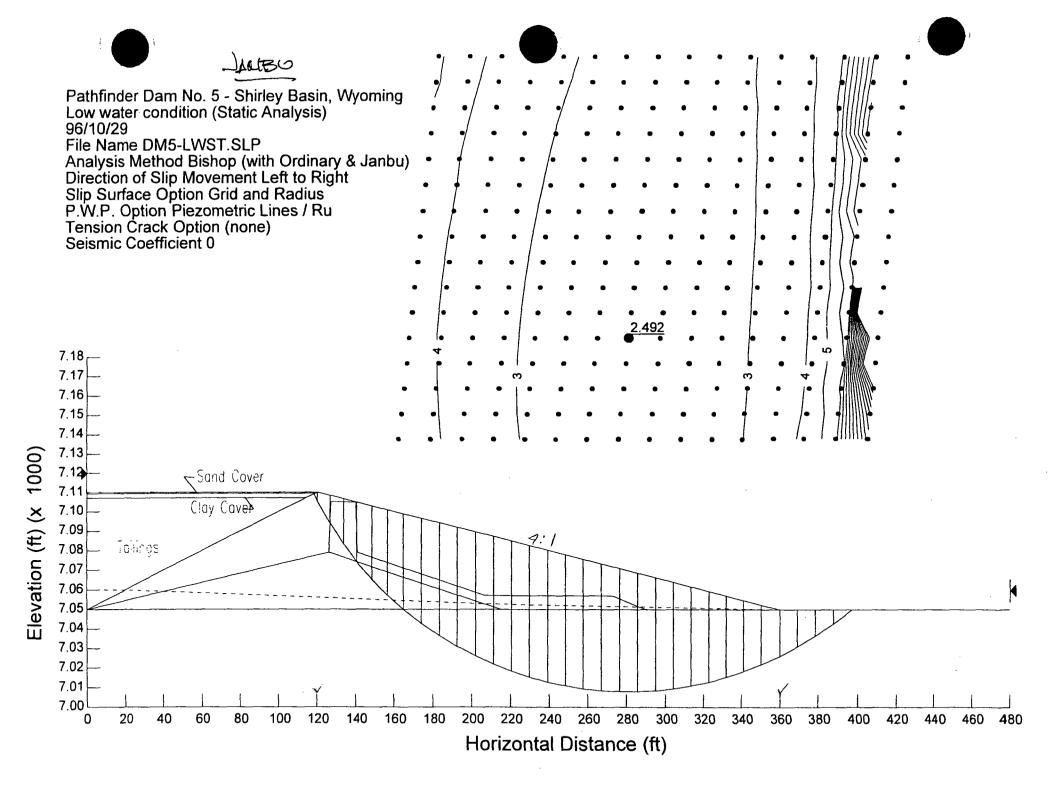


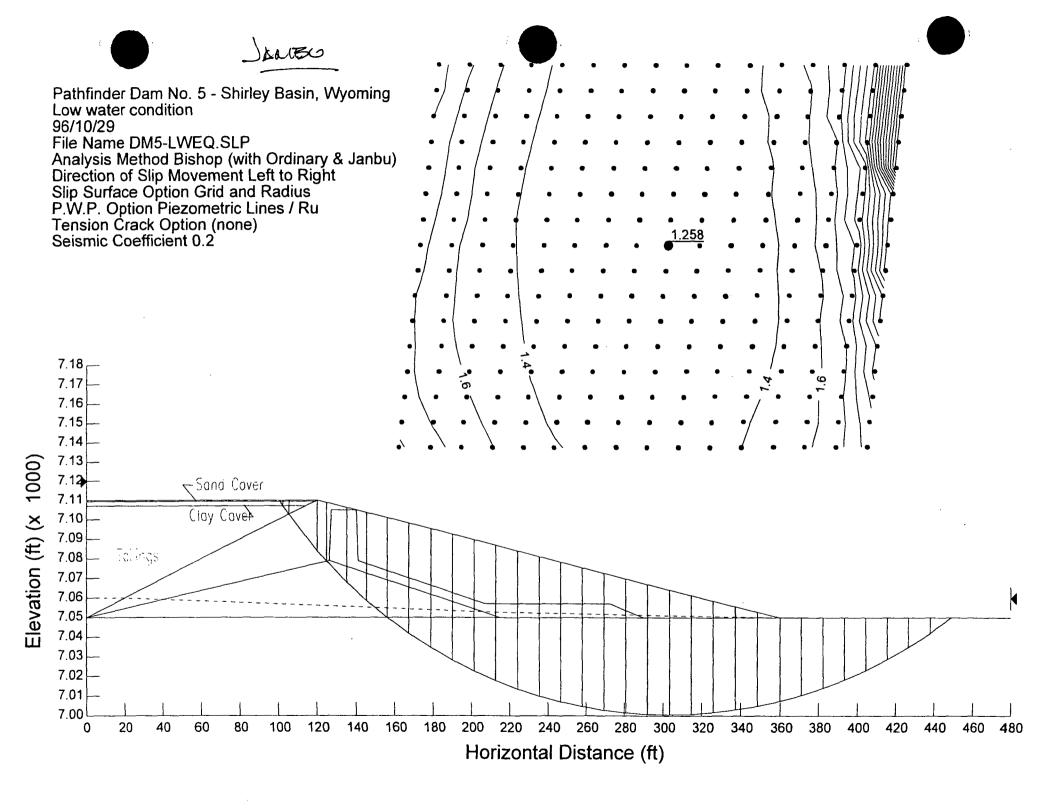


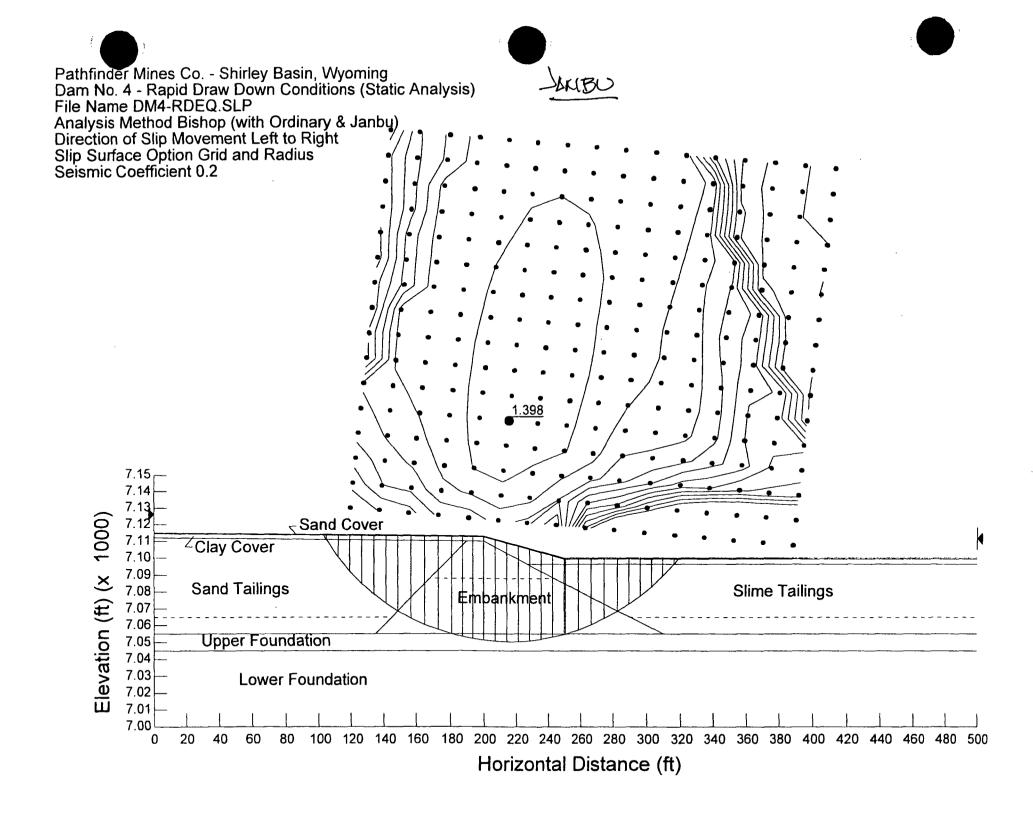


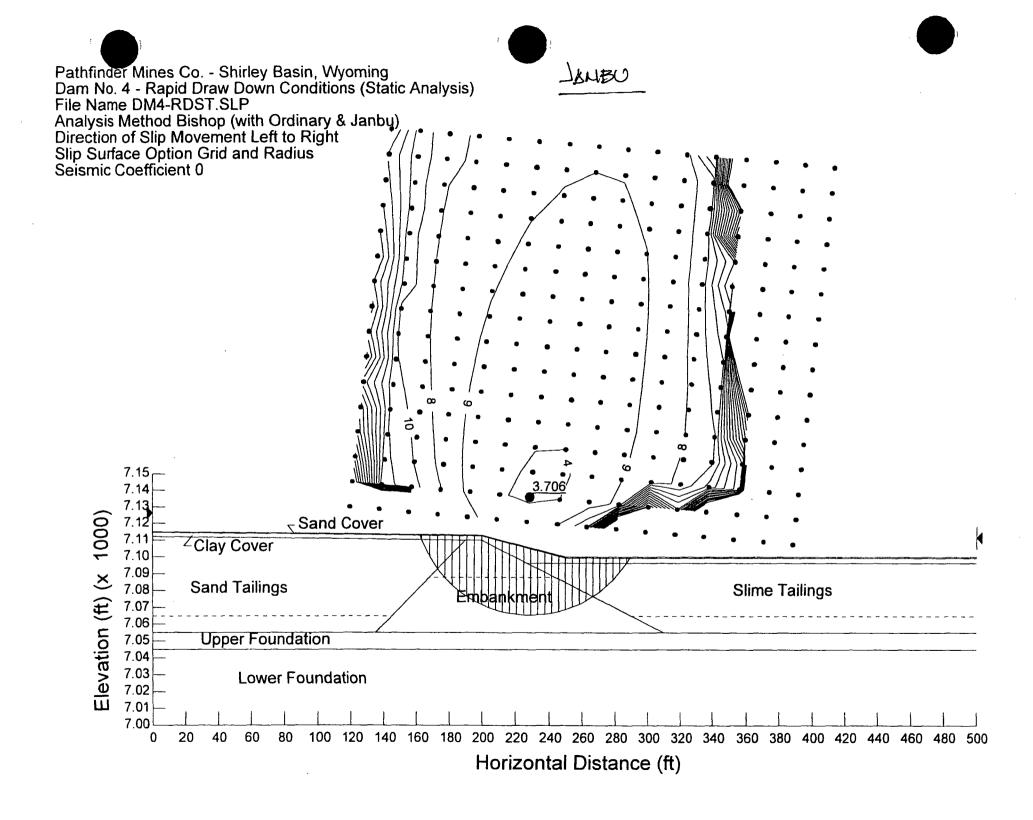












THIS PAGE IS AN OVERSIZED DRAWING OR FIGURE,

THAT CAN BE VIEWED AT THE RECORD TITLED:

"SUPPLEMENTARY EXHIBIT A, APPROXIMATE SAND AND SLIME TAILINGS LOCATIONS. EXH6-3.DWG"

WITHIN THIS PACKAGE... OR BY SEARCHING USING THE

References

- 1. "Foundations and Earth Structures-Design Manual 7.2", Department of Navy Naval Facilities Engineering Command, May 1982.
- 2. "Characterization of Inactive Uranium Tailings Sites: Shiprock, New Mexico", prepared for Department of Energy, Albuquerque, New Mexico, prepared by the Geotechnical Engineering Program, Colorado State University, 1984.
- 3. Vick, Steven G., "Planning, Design, and Analysis of Tailings Dams", John Wiley & Sons, 1983.

by

James C. Case Wyoming State Geological Survey August, 1997

Background

The U.S. Nuclear Regulatory Commission generated the "Final Standard Review Plan" to guide the reclamation of uranium mill tailings sites. The review plan clarifies federal regulations 10 CFR 40 Appendix A and 10 CFR 100 Appendix A. The Final Standard Review Plan has three steps to be completed in order to generate a seismotectonic stability analysis of a mill tailings site. The steps are as follows:

- 1) Determination of the Maximum Tectonic Province Earthquake (Floating Earthquake)
- 2) Identification of Capable Faults
- 3) Designation of Maximum Credible Earthquake

In order to satisfy the requirements of the NRC as set forth in the "Final Standard Review Plan", I have generated a basic analysis of the following items:

- 1) Deterministic Analysis on Nearby Active Faults with a Surface Expression
- 2) "Floating Basin Earthquake" Analysis for a Regional Tectonic Province
- 3) Designation of Maximum Credible Earthquake (Larger of Items 1 or 2)

Deterministic Analysis

In 1988, Geomatrix Consultants, Inc., completed a report titled "Seismotectonic Evaluation of the Wyoming Basin Geomorphic Province". The report area includes the Shirley Basin and Pathfinder's Shirley Basin Mill Tailings Site. Geomatrix assembled available information on known active faults in addition to conducting field investigations on poorly studied faults. Based upon the Geomatrix study, as well as unpublished information and reports at the Wyoming State Geological Survey, I have concluded that the active fault that could cause the greatest accelerations at the Shirley Basin Site is the Seminoe Mountain Segment of the South Granite Mountain Fault System. The nearest point on the Seminoe Mountain Segment is located approximately 53 kilometers west-southwest of Pathfinder's Shirley Basin Mill Tailings Site.

Geomatrix (1988) did not find evidence of late-Quaternary movement on the Seminoe Mountain Segment of the South Granite Mountain Fault System, and scarps that were present were found to be fault-line scarps due to differential erosion. The Seminoe Mountain Segment, however, is an extension of other segments of the South Granite Mountain Fault System that have been shown to be active in the late-Quaternary, and recurrently active over the last 500,000 years. The other segments of the South Granite Mountain Fault System are located to the west of the Seminoe Mountain Segment, and are all further from the mill tailings site.

Based upon the above analysis, I feel that the Seminoe Mountain Segment of the South Granite Mountain System is a conservative selection for the active fault nearest the site. Geomatrix (1988) did not assign a maximum magnitude earthquake to the Seminoe Mountain Segment, in large part because of poor

exposure of the fault, lack of measurable surface offsets, and uncertainty in the actual length of the segment. Geomatrix estimated the length of the Seminoe Mountain Segment to be 36 kilometers. Such a fault length would result in a magnitude 6.85 earthquake if the entire length ruptured (Wells and Coppersmith, 1994). All other active segments of the South Granite Mountain Fault System, however, have been assigned a maximum magnitude of 6.5 to 6.75. Due to the uncertainties associated with the Seminoe Mountain Segment, I feel that a maximum earthquake of magnitude 6.75 is more reasonable than one of magnitude 6.85.

A magnitude 6.75 earthquake originating on the Seminoe Mountain Segment, located 53 kilometers from Pathfinder's Shirley Basin Mill Tailings Site, should result in a peak horizontal acceleration of approximately 0.05g (Campbell, 1987) at the site. This acceleration is conservative, considering the uncertainties associated with the Seminoe Mountain Segment.

Floating Basin Earthquake for a Regional Tectonic Province

NRC's Final Standard Review Plan requires that "For those earthquakes not associated with known tectonic structures (i.e., "floating" earthquakes) the largest event that has occurred in each of the tectonic provinces expected to influence the seismicity of the site should be identified". In other words, the largest event that has occurred and has not been tied to a specific fault system, or related structure, should be considered the "floating" earthquake for the tectonic province. The "floating" earthquake may be larger than the largest historic earthquake, and should be the largest "random" earthquake thought to have occurred in the last 35,000 - 50,000 years.

Lawrence Livermore National Laboratory (Bernreuter and others, 1994) included the Shirley Basin in a "Central Wyoming Seismic Zone", defined by 109.7° West longitude on the west, 105.5° West longitude on the east, 41.5° North latitude on the south, and 43.0° North latitude on the north. This "Central Wyoming Seismic Zone" is within an even larger tectonic province, defined by Geomatrix (1988), called the "Wyoming Foreland Structural Province". The "Wyoming Foreland Structural Province" is approximately defined by the Idaho-Wyoming Thrust Belt on the west, 104° West longitude on the east, 40° North latitude on the south, and 45° North latitude on the north.

Geomatrix (1988) estimated that the largest "floating" earthquake that may occur in the "Wyoming Foreland Structural Province" would have a magnitude in the 6.0 - 6.5 range, and used a magnitude 6.5 earthquake in their analysis. The average of the range of magnitudes suggested by Geomatrix for a "floating" earthquake in the "Wyoming Foreland Structural Province" is magnitude 6.25. A magnitude 6.25 "floating" earthquake is suggested by Lawrence Livermore National Laboratory (Bernreuter and others, 1994) for the "Central Wyoming Seismic Zone. I used a magnitude 6.25 "floating" earthquake in my analysis of Pathfinder's Shirley Basin Mill Tailings Site.

Once the "floating" earthquake has been determined for a tectonic province, it must be arbitrarily placed at a certain distance from a site in order to estimate what accelerations may be felt if such an earthquake occurs. NRC's Final Standard Review Plan defines the site-to-source distance for "floating" earthquakes as 15 kilometers.

A magnitude 6.25 "floating" earthquake place 15 kilometers from Pathfinder's Shirley Basin Mill Tailings Site would result in a peak horizontal acceleration of 0.15g at the site.

Maximum Credible Earthquake

Based upon the guidance supplied in NRC's "Final Standard Review Plan", the maximum credible earthquake on the nearest identified active fault would have a magnitude of 6.75. The maximum credible "floating" earthquake, not associated with a specific active fault, would have a magnitude of 6.25. Because the nearest identified active fault is 53 kilometers from the site, design accelerations should be based upon the "floating" earthquake. A peak horizontal acceleration of 0.15g should be used in the design of Pathfinder's Shirley Basin Mill Tailings Site.

Summary

A basic seismological characterization was conducted for Pathfinder's Shirley Basin Mill Tailings Site. Based upon guidance provided in the "Final Standard Review Plan" (U.S. Nuclear Regulatory Commission, 1993), a peak horizontal acceleration of 0.15g should be used in the design of the mill tailings site.

References

Bernreuter, D., McDermott, E., and Wagoner, J., 1994, Seismic hazard analysis of Title II reclamation plans: Report prepared by Lawrence Livermore National Laboratory for the U.S. Nuclear Regulatory Commission, 145 p.

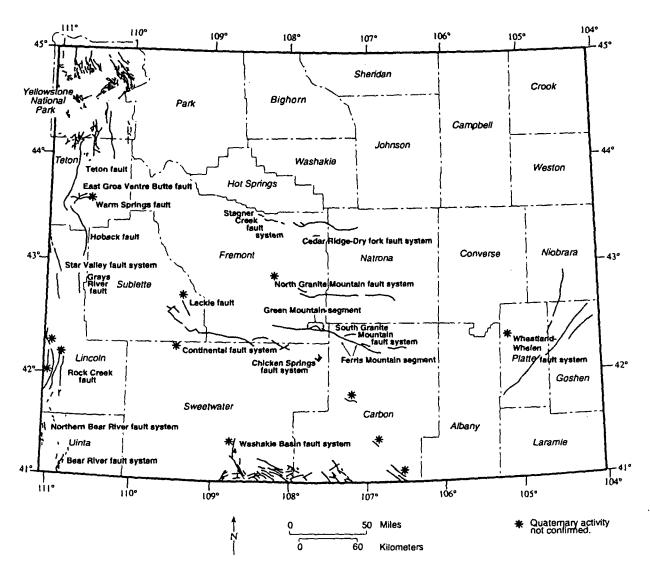
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Geomatrix Consultants, Inc., 1988, Seismotectonic evaluation of the Wyoming Basin geomorphic province: Report prepared for the U.S. Bureau of Reclamation, Contract No. 6-CS-81-07310, 167 p.

Wells, D.L., and Coppersmith, K.J., 1994, New empirical relationships among magnitude, rupture length, rupture width, rupture area, and surface displacement: Bulletin of the Seismological Society of America, Vol. 84, No. 4, pp. 974 - 1002.

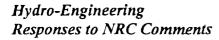
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SUSPECTED ACTIVE FAULTS IN WYOMING

References

- 1. Case, James C., "Basic Seismological Characterization for Pathfinder's Shirley Basin Mill Tailings Site", Report of Wyoming Geological Survey, August, 1997.
- 2. "Foundations and Earth Structures-Design Manual 7.2", Department of Navy Naval Facilities Engineering Command, May 1982.
- 3. "Characterization of Inactive Uranium Tailings Sites: Shiprock, New Mexico", prepared for Department of Energy, Albuquerque, New Mexico, prepared by the Geotechnical Engineering Program, Colorado State University, 1984.
- 4. Vick, Steven G., "Planning, Design, and Analysis of Tailings Dams", John Wiley & Sons, 1983.



Section 1- Geotechnical

Comment 4. Technical basis for settlement and subsidence calculations.

Response: It is important to point out that the predictive consolidation modeling was done with a very simplistic approach because it was intended for comparative purposes only. NRC personnel have repeatedly emphasized that demonstration of achievment of t_{90} with monitoring is the accepted standard which must be met before construction of the final cover. The following text addresses specific aspects of the predictive settlement calculation that were brought up in the comments.

- (i) The compressive indices varied with the nature of the tailings but a universal composite compressive index was used to simplify calculations. The hydraulic spiggoting of tailings from many locations results in a tremendously heterogeneous tailings pile and it simply is not practical to characterize the tailings with the resolution that would allow a very local prediction of settlement. It is possible to distinguish between areas that are primarily slimes and primarily sands, as well as using gamma logs from wells to determine a rough proportion of sand or slimes at other locations. This information is used in developing a settlement monitoring program. However, because of the known heterogeneity and variability in the tailings placement, it is not worthwhile to do an elaborate predictive settlement analysis or sensitivity analysis. Rather, the NRC has indicated that it is preferrable to monitor settlement.
- (ii) Like compressive index, the void ratio showed tremendous variability between tailings samples. A typical composite value was selected for simplicity.
- (iii) At the time that the samples were taken and the subsidence analysis was done, much of the No. 4 and No. 5 Ponds was still covered by tailings solution, hence the total stress and pore water pressure estimates were based on assumptions of extreme initial and final conditions. The initial total stress was estimated as a linear stress distribution from the surface with an overburden unit weight of 125 lb/ft³. The initial pore water pressure was assumed to be a hydrostatic pressure distribution for a completely saturated tailings profile. The final total stress was assumed to be a linear stress distribution from the surface with an overburden unit weight of 100 lb/ft³. The final pore water pressure was assumed to be negligible throughout the profile.

Regarding the potential for differential settlement, the committment to delay construction of the final cover until settlement monitoring indicates t_{90} has been reached is considered adequate protection against cover damage by settlement. There is currently

Hydro-Engineering Responses to NRC Comments

an aggressive tailings dewatering and solution evaporation program underway to accelerate consolidation. As discussed above, the heterogeneity of the tailings in combination with extremely complex drainage conditions makes local prediction of the rate and magnitude of settlement very difficult. As such, the settlement monitoring program appears to be a far more reliable means of insuring that the magnitude of differential settlement is acceptable. The only appreciable dynamic loading that will occur on the radon barrier will be during construction of the barrier and the overlying erosion protection layers. During the construction process, general grading and appropriate routing of construction traffic will eliminate areas where the cover could be compromised by the dynamic loading. With the demonstrated essential completion of primary consolidation, the strain on the cover will be very limited and cracking of the cover will be unlikely. In addition, the potential impacts on radon flux of small, strain induced cracks in the cover are trivial. As an example, the bare source flux for scenario #1 of the radon modeling in the TRP is 43 pCi/m²/s while the presence of the cover limits the radon flux to 6.0 pCi/m²/s. Given this ratio of radon flux rates, roughly onehalf of the surface area of tailings would have to be completely exposed by cracks to exceed the limiting flux of 20 pCi/m²/s. This radon flux from the exposed area is subject to the nature of the tailings and cover, but it is difficult to imagine a scenario where the area of exposed tailings due to cracks would be measurable, much less constituting a signficant part of the total tailings cover area.

Comment 5. Problems with Table 5-12 and compressive index.

Response: Table 5-12 contains errors in the alignment of heading labels and is being replaced. As mentioned above, a single universal compressive index was used in the predictive analysis in the interest of simplicity.

Comment 6. Proposed embankment slopes.

Response: The embankment slopes will be reduced to 5H:1V.

Comment 7. Field testing of permeability.

Response: In addition to Table C.2-4, the reviewer is referred to Table C.2-1 which contains permeability results for samples taken from the South Dump test holes. The largest of the permeabilities from this sampling program was used in the infiltration calculations. There has always been some concern about validity of laboratory testing of permeability and it naturally follows that the focus of the concern is with situations where a very small permeability is desired. Unfortunately, satisfactory field testing of permeability requires post-construction testing or creation of a test pad. Even under

field conditions, a permeability test causes some disturbance and may introduce substantial error, and it seems unlikely that the error would give a smaller indicated permeability. If the permeability used in the infiltration modeling is increased to 10^{-7} cm/sec, the maximum infiltration depths for the locations in Table 5-13 of the TRP increases to 3.01 inch. This depth of penetration is still acceptable and this calculation indicates that the permeability of the barrier material is substantially smaller than necessary. It should also be noted that the depth of infiltration is an iterative calculation, and the depth of penetration for locations in Table 5-13 of the TRP will converge to slightly smaller values if the number of computations is increased. It is unfortunate that, as the laboratory tests indicate less permeable material, the results become more and more suspect.

Section 2- Radon Attenuation

Comment 1. Quantities of clay barrier material.

Response: The South Test Hole (STH) and North Test Hole (NTH) sampling programs were intended to determine both the quantity and quality of the planned barrier material. The lithology logs for 11 test holes on the South Dump are presented in Appendix A and were used to determine the thickness of layers of suitable material. Cross-sections were developed using the test hole logs and this allowed computation of the available volume of suitable clay. This was done informally during the development of the TRP and is presented in a more formal manner in an attached report. The test holes covered an area that was expected to yield sufficient clay for the barrier but only covered a small percentage of the actual footprint of the south dump. Since the planned barrier material originated as a thick aquitard between the White River and Wind River aquifers, there is every reason to believe that the same clay is present in large quantities throughout the dump. If necessary, the borrow area can be expanded to provide clay for the barrier. The required volume of clay for the barrier is presented on page 10-2 of the TRP.

Comment 2. Radiological properties of tailings.

Response: With the recent coverage of the former pool area on the No. 5 Pond, additional sampling is possible and a minimum of eight radon flux measurements will be taken. The details of this sampling program are discussed later in this response. The thickness of the interim cover will also be determined and included in the radon modeling. A substantial thickness of interim cover has been applied to the former pool area of the No. 5 Pond, and it may be possible to reduce the planned three foot clay radon barrier thickness to the 2.5 foot clay thickness planned for the remainder of the No. 5 Pond. Current sampling coverage in Pond No. 4 is sufficient and operational

adjustments will be made in cover placement or the placement of general fill if unexpected conditions are encountered. In order to clarify the methods used in radon modeling in the TRP, the following discussion describes the reasoning used in the original sampling and modeling and addresses some of the concerns of the reviewer.

In the tailings well sampling program (TW series), sampling at less than three feet was not done to prevent potential dilution of the samples with interim cover. Many of the pit samples were taken from shallower depths (see Table 2-1 of the TRP) and the greatest Ra226 activity for samples taken from depths less than three feet for the pit samples was 65.8 pCi/gm. The average Ra226 activity for all of the Pond 4 and Pond 5 shallow (less than three feet) pit samples was 29 pCi/gm, hence the reasoning for excluding samples with a Ra226 activity of less than 20 pCi/gm and the concerns for sample dilution by non-tailings material. Because the pit samples were located on the periphery of the ponds to determine the extent of tailings, they also represent limited tailings depths, and were not included in the radon emanation modeling for this reason. If all of the Pond 4 and Pond 5 tailings well and bit camples less than 20 feet from the surface are included, the average activity is 72 pCi/gm. This is still substantially below the value of 107 pCi/gm (average activity of 65 pCi/gm. plus one std. deviation) used in the scenario #1 modeling and corresponds closely with the average activity for scenario #1 of 65 pCi/gm. It should be noted that there are two errors in Table 3-1 of the TRP. The Ra226 activity for sample TW4-1B (25'-27') should be 728 pCi/gm rather than 7.28 pCi/gm and the sample for well TW4-1B (3' to 5') was omitted. This additional sample gave a total of eight that were actually used in calculating the average Ra226 activity. This table is corrected and replaced in this submittal.

The gamma logs were used as a very strong indicator of the nature and activity of tailings at the well or sampling locations, and were used in deciding which samples should be used in the radon modeling. As an example, Figure A.2-3 of the TRP presents the geophysical log for well TW4-1C and Figure A.3-3 presents a predicted Ra226 activity based on the gamma log. A sample was taken from this well at the 3 feet to 5 feet depth with a resulting Ra226 activity of 53.6 pCi/gm. This was a typical sandy tailings sample, and the gamma log indicates that the material is similar to a depth of roughly 18 feet. Below this there is a transition zone to what appears to be a slime zone extending from 25 feet to 45 feet depth. This is supported by the lithologic log on page A.1-3 of the TRP, although there is sometimes considerable lag and mixing of cuttings for tailings wells, as demonstrated by the five foot discrepancy between lithologic and geophysical logs. When this occurs, the geophysical log is considered more reliable in indicating depths of specific materials. The gamma log in Figure A.2-3 also presents a fairly typical tailings deposition pattern for the central tailings area. The depostion of fines (slimes) at great depth from the surface likely occurred early in the life of the pond, when spiggoting was done from the dam or extreme periphery of the pond. At this time, much of central tailings area was under ponded water. As the tailings sand filled in on the beach areas along the pond periphery, the spiggoting was moved inward on the pond. This crowded the pool area into its location at the time of development of the

TRP, and overlaid much of the initial slime pool with substantial depths of sandy tailings. The tailings dams were constructed across the upper end of the Mine Creek channel, and the presence of channels and other flow diverting features at the base of the ponds further complicates depositional patterns. With this in mind, an additional exhibit (Supplementary Exhibit A) has been developed showing areas of primarily sandy tailings, primarily slime tailings, and slime tailings overlain by significant depths of sandy tailings.

Most of the tailings well samples were taken from the top eight feet of tailings. The exceptions to this were when the gamma logs indicated a substantial change in the properties of the tailings, and two samples were taken from slime layers at depths greater than 25 feet to determine slime properties. The radon flux is more directly affected by the source materials near the surface unless the underlying materials have a substantially greater Ra226 activity. None of the gamma logs indicated a substantially greater activity for tailings at depths of 3 to 15 feet from the surface, so sampling was concentrated in the top eight feet. A brief sensitivity analysis shows that a two-fold increase in the Ra226 activity for the top eight feet of tailings for radon modeling scenario #1 of the TRP increases exit radon flux from 6.6 pCi/m²/s to 12.4 pCi/m²/s. If the two-fold increase in Ra226 activity is moved to the bottom 8.4 feet, the increase in exit radon flux is from 6.6 pCi/m²/s to 7.4 pCi/m²/s. It is apparent that, unless there is a reason to believe that the Ra226 activity is much greater at depths of 8 to 16 feet, the properties of tailings from the 0 to 8 feet depth has more impact on the radon flux.

The measured emanation coefficients for the tailings samples are generally lower than published values for tailings. A series of simulations were conducted with the default emanation coefficient of 0.35 to evaluate potential impacts if the measured emanation coefficients are anomalously small. These simulations used average cover moisture content for the cover material, and average radium activity for the tailings. The remainder of the parameters were the same as the original modeling. The exit flux for scenario #1 increases to 8.3 pCi/m²/s and the exit flux for scenario #2 increases to 30.3 pCi/m²/s with these revisions and the dramatic change in emanation coefficient. Neither of the simulations includes the interim cover that is in place over much of the tailings. The recent placement of interim cover over slime areas of Pond No. 5 has added a minimum of one foot of clay to the tailings. If this interim cover is included in the scenario #2 comparative modeling with a conservative interim cover moisture content of 15%, the exit flux of 30.3 pCi/m²/s is reduced to 19.9 pCi/m²/s. With either situation, if the proportion of the tailings area where slimes are present near the surface (roughly 40%) is considered, the average radon exit flux is still well below 20 pCi/m²/s and ranges from 12.9 pCi/m²/s to 17.1 pCi/m²/s. The radon flux from the chimney drain is not included in this evaluation because the average depth of cover over the cycloned tailings in the chimney drain will likely exceed ten feet.

The preceding discussion indicates that there are sufficient conservatisms incorporated in the radon modeling to insure that radon flux at the surface will be limited

to less than 20 pCi/m²/s. Additional sampling on Pond No. 5 as discussed below will allow further evaluation of the proposed cover configuration, but at present, the planned cover configuration is considered more than adequate.

A minimum of eight radon flux measurements will be taken on the No. 5 Pond. These samples will be distributed over the entire pond area, but there will be some concentration of sampling within the former pool area of the pond. These samples will be taken on top of the interim cover, which is constructed from a material similar to that of the eventual radon barrier, but is not subject to the quality control or compaction of the final cover. These samples should be a much more reliable means of predicting the eventual radon emanation from the tailings area. A radon flux measurement integrates all of the tailings and cover material properties that affect radon flux, and avoids concerns about quantifying those properties.

Comment 3. Sampling locations.

Response: The OP- sample designation refers to ore pad test holes where only gamma logs were taken. The locations of these test holes are shown on Exhibit 2-2 of the TRP and the gamma logs are presented in Appendix A. Text on Page 2-5 incorrectly states that OP- hole locations are shown on Exhibit 1-1. This text is corrected in this submittal. The NS- designation refers to two surficial wells that were installed near the north end of Pond No. 5. The location of these two wells was inadvertantly left off of Exhibit 1-1, and this exhibit is replaced in this submittal. Only geophysical logs were taken for these two wells, and these logs are presented in Appendix A of the TRP.

Comment 4. Buildings.

Response: No buildings will be left within the restricted area.

Comment 8. Radon barrier parameters.

Response: The cover material sampling plan was intended to identify quantity and quality of the available material. The test hole drilling logs were used in identifying the location and quantity of suitable materials, and then samples from the drill holes were tested. Quality control criteria were developed and included in Section 11.2. Samples were excluded from computations for the clay radon barrier if: they did not meet the quality control criteria, they were taken from the north dump which is only considered an auxiliary source, or the analysis was not complete. As an example, none of the three CP samples meets the gradation criteria which requires that 83% of the barrier material pass a #200 sieve, while the Set samples are lacking Atterberg limits testing. The ND

and NTH samples were not included because they were taken from the North Dump. The remaining samples represent a fairly uniform clay material present in large quantities in the south dump. If all of the samples that meet the QA/QC gradation and density criteria (with the exception of those which do not have an organic matter content) are included in the averaging, the resulting mean moisture content is 21.59% and the mean maximum dry density is 96.54 lb/ft³. This dry density has declined slightly from 99.26 lb/ft³ for the original sample set, while the mean predicted long-term moisture content has increased substantially from 18.74%. If these values are substituted into the radon modeling, the increase in moisture content will more than offset the slight decrease in density, and the radon flux will decrease substantially. As a result, the selection process for the original radon barrier sample set excluded superior barrier materials based on location or incomplete testing results, as well as excluding unsuitable material. If the excluded superior barrier material samples represent a large volume of clay in the borrow area, the clay radon barrier will be of better quality than that used in the modeling.

Comment 9. Mill and ore pad area cover thickness.

Response: The mill rubble was buried in trenches and then covered with a minimum of three feet of the spoil from the trenches. Characterization of the mill rubble as an average "source" is not feasible, but the equivalent Ra226 activity for the mill rubble and overlying cover is expected to be much lower than that for the scenario #1 radon modeling. Gamma logs for the ore pad test holes (Appendix A of the TRP) indicate varying predicted Ra226 activites ranging from less than 5 pCi/gm to approximately 160 pCi/gm with source layer thicknesses ranging from 0 to approximately 9 feet. The "worst-case" ore pad radon source situation appears to be hole OP-7 presented in Figure A.3-23 of the TRP with an average predicted Ra226 activity of 160 pCi/gm extending to a depth of approximately 2 feet. If this source layer is substituted in the scenario #1 modeling with the maximum measured emanation coefficient for ore materials on the low grade waste pile of 0.166, the resulting exit flux is 10.2 pCi/m²/s. The reclamation design for the ore pad area is being revised to leave more of the ore pad in place with appropriate cover. Radon flux measurements for the mill and ore pad area will be conducted shortly, and the results will be incorporated into the TRP. In conjunction with the radon flux measurements, samples of the existing mill area cover will be analyzed for moisture content. Like those from the No. 5 Tailings Pond, these radon flux measurements are expected to be a much better measure of the eventual radon emanation from the tailings than predictions based on materials properties.

Pathfinder Mines Corporation Shirley Basin Mine Clay Borrow Volume Estimates

In the process of developing the Tailings Reclamation Plan (TRP) for Pathfinder Mines Corporation's (PMC) Shirley Basin Mine, an investigation of potential clay sources for the radon/infiltration barrier was undertaken. Eleven test holes were drilled in the Area 3 South Dump (see Figure 1) which provided the necessary lithology and samples to estimate the available volume of suitable material. The test hole logs are presented in the TRP and the information is only summarized on cross-sections in this report. Seven cross-sections were developed and presented in Figure 1. These cross-sections were constructed by superimposing test hole lithology for up to four holes that fell roughly in a line. Surface elevations were estimated from topography when the holes were drilled, and the eventual reclamation surface at each hole was also estimated and is presented in Figure 1. Since the holes are used in more than one cross-section, there was no labeling of individual cross-sections to avoid confusion. Rather, the sequence of hole numbers is used to indicate the cross-section location.

Samples from the test holes were used to estimate properties of the suitable material. The specifications for the radon/infiltration barrier are presented in the TRP and were derived from sample properties. Materials that meet the specifications can be used for the radon/infiltration barrier, while reject materials may be used for general fill or for the sandy capillary barrier layer.

The volume of available material was estimated by determining the thickness of suitable material from the cross-sections, and then interpolating the thickness between test holes. A grid system was then developed over the planned borrow area and the thickness and area of each cell were used to determine the volume of clay in each cell. The sum of the volumes for the cells was approximately 1.5 Myd³. This estimate was done in a conservative manner but is still slightly greater than the required volume as estimated in the TRP. If additional clay is required, the expansion of the borrow area would be southeast from hole STH-3. The log of hole STH-3 indicates that there is a substantial thickness of clay at that location, and extension of the borrow in this area should yield a large volume of clay with only minor adjustments in the post-reclamation configuration.

REPLACEMENT AND ADDITIONAL PAGES AND EXHIBITS

The following table itemizes the enclosed revisions to the Shirley Basin Tailings Reclamation Plan and indicates the replacement pages for the document currently on file with the NRC.

Volume I Revisions

New Text/Exhibit	Replace(s)
Volume I Cover Page	Volume I Cover Page
Page 2-5	Page 2-5
Page 3-5	Page 3-5
Page 5-83	Page 5-83
Exhibit 1-1	Exhibit 1-1

The text is copied double-sided and, in many cases, the unmodified adjacent page is replaced along with the modified page to preserve the numbering sequence. The revision date for each page is located in the lower right hand corner of the page.



May 29, 1997

Mr. Joseph J. Holonich, Chief Uranium Recovery Branch Division of Waste Management Office of Nuclear Material Safety & Safeguards U.S. Nuclear Regulatory Commission Washington, D.C. 20555

Re: Docket No. 40-6622 License No. SUA-442 Shirley Basin Mill

The following comments and modified text pages (five copies of each) to the Tailings Reclamation Plan and supplemental Soil Cleanup Verification and Sampling Plan are submitted in response to your letter dated May 12, 1997, concerning the Soil Cleanup Verification Plan and associated items. Other aspects of the May 12 letter will be addressed in subsequent submittals. Reference below to "supplemental text" means the text of the soil cleanup verification plan.

Radon Attenuation.

Comment 4: The plan does not mention whether or not buildings will remain on the site.

Response: No buildings (excluding temporary huts over wells) will remain within the restricted area after decommissioning. In the area of windblown tailings cleanup, decontamination will be done under such temporary buildings during the general cleanup.

Comment 5: The proposed soil background Ra-226 value is not substantiated.

Response: The original data set has been modified to reflect samples only taken from the upper 15 cm. to 20 cm. of soil and at locations that should be reflective of background as seen on undisturbed ground and in mine related areas beyond the general area of the proposed cleanup. Based upon this narrower data set it appears that a background value of 3.0 pCi/g Ra-226 is still appropriate. This site is very proximate to an area of mine disturbance encompassing in excess of 3,000 acres that bounds the restricted area on three sides. As a result it is very appropriate to consider Ra-226 levels in the soils of the mine area when one considers background for the mill/tailings site. One would expect a disproportionate number of elevated Ra-226 shallow soil sample sites relating exclusively to the mine. Informational copies of the three earlier reports that have been utilized in



developing the revised data set are included with this submittal. See modified text pages 2-2 and 2-3.

Comment 6: The plan does not address other radionuclides that may require cleanup.

Response: This site maintained close control of source/byproduct material handling such that no problems with uranium or thorium are anticipated in the general cleanup area. Virtually all contamination directly relates to wind dispersed tailings, and the Ra-226 cleanup criterium should also insure the cleanup of other radionuclides of interest. There were no releases of solution that would result in elevated U-nat or Th-230 levels. However, PMC will randomly select twenty percent of the final verification samples for analysis of U and Th-230 to demonstrate adequate cleanup of those radionuclides. See modified supplemental text page 10 and revised procedure 03.022.

Comment 7: Inadequate Gamma Survey Information.

Response: See modified text page 2-4 which references the Soil Cleanup Verification and Sampling Plan.

Comment 10: Section 11.8 does not reflect the Cleanup Verification Plan.

Response: It does not appear that the reviewer had the most current Section 11.8 (dated 5/20/96) which refers to a planned appendix that provides details of the soils cleanup verification. See enclosed modified text page 11-7 which formally incorporates the cleanup plan.

Soil Cleanup Verification and Sampling Plan.

Comment 1: PMC should indicate which document contains the background soil Ra-226 data.

Response: The background soil Ra-226 data is presented in the Reclamation Plan and is further addressed in Comment 5 under Radon Attenuation in this submittal. See modified supplemental text page 1.

Comment 2: PMC should confirm that the height of the detector that is mounted on the four-wheel drive vehicle is 18 inches, the same as the hand-held Ludlum 44-10.

Response: The detectors are mounted at a height of 18 inches above the ground surface on the four-wheel drive vehicle which is the same height as that used for the hand held correlation studies. See modified supplemental text page 5.



Comment 3: PMC should explain how the second study - meter at "normal height" and 36 inch diameter area sampled (page 3), relates to demonstrating that the average Ra-226 value in 100 m² area meets the standard.

Response: The study at specific points was done in uniformly contaminated and highly characterized areas. This was done in order to provide additional data to support our action levels. The point measurement locations were selected using the following criteria: a) the point measurement location must be in an area that has uniform count rates and therefore must not be affected by shine from nearby highly contaminated areas, b) the detector readings at the points must be in the anticipated range of interest. Since the soils directly beneath the detector have the primary influence on detector, the soil within the 36-inch diameter circle beneath the detector was carefully characterized by taking five samples from this circular area. The correlation was then done between the detector reading and the soil concentration.

In some ways this method of arriving at an action level more properly reflects what one expects upon final cleanup of a windblown tailings contaminated area. First, grid blocks sampled after the cleanup will normally be free of hot spots (uniformly contaminated or at background levels). Therefore by taking more samples near the detector in a uniformly contaminated area during the study, the sampling error has been minimized and the Ra-226 concentration is known very accurately. Also, the count rate can be determined very accurately by integrating the count over one minute or more. For exposure rate meters, the average exposure rate could be determined by watching the meter for a period of time or by writing down several measurements and taking the average. It might be argued that the point measurement study is the benchmark to which all other studies should be compared.

In the study of the entire 100 m² plots, the individual plots were not uniformly contaminated. This leads to concerns about sampling errors (and thus erroneous Ra-226 concentrations) and determining the average count rate or exposure rate. In some sense, this may represent the worst case situation after cleanup (rather than the norm). If this is the case, an action level determined from such a study should be highly conservative and apply to all situations within the windblown area with greater than a 95 percent confidence level.

As can be seen from the study results, both methods predicted essentially the same action level. This provides a high level of confidence that the action level has been accurately determined at the 95 percent confidence level.

Comment 4: PMC (page 6) considers two of the Ra-226 QA values determined by ELI (vendor lab) as outliers because they don't agree with other data trends.

- a. PMC should indicate how future QA data will be discarded and why they have confidence in the vendor lab's QA/QC program.
- b. PMC should have another lab analyze the two questionable samples.
- c. PMC should insure that the soil sample mixing procedure is adequately implemented.
- d. PMC should indicate what degree of agreement with the QA Ra-226 data is required to consider the gamma spectrometer data valid.

Response: PMC consulted with the vendor laboratory and believes that their sample aliquoting procedure was the cause for the different values for the two samples. Because of the large discrepancy between the results for these two samples, it is highly probable that the major contribution to the difference arises from an aliquoting error. Therefore an analysis by another laboratory would not be revealing. PMC believes that the problem has been resolved and has focused on establishing a very intensive QC check program for the verification samples as described below.

For future analyses, the vendor laboratory has agreed to process the entire sample before taking an aliquot for radiochemical analysis. PMC proposes to use the more costly radiochemical analysis method as the primary verification method (rather than the gamma spectral analysis as stated in PMC's April 3, 1997 Soil Cleanup Verification and Sampling Plan, Procedure 03.022.01) since good agreement with the field gamma spectrometry laboratory and samples sent to ORISE has been demonstrated. As a check on the primary vendor lab's QA/QC program, PMC will split all verification samples and submit twenty percent of the samples to another vendor laboratory for Ra-226 analysis as QC samples. PMC is not aware of any other cleanup program that submits twenty percent of the verification samples to a second vendor laboratory for analysis as a QC check on the primary vendor laboratory. This, along with the results from the on-site laboratory for each sample, should provide assurance that the laboratory results are accurate. SOP No. 03.022.01 has been revised to incorporate these changes and is enclosed.

The results from the two vendor laboratories will be evaluated by assuring that the error bars overlap at the three standard deviation level for all samples having measured Ra-226 concentrations greater than 1 pCi/g. That is, if the sample results for laboratories A and B are reported as $C_A \pm 3\sigma_A$ and $C_B \pm 3\sigma_B$, where σ is the standard deviation, PMC will conduct an investigation if the following condition is not met: $|C_A - C_B| \le |3\sigma_A + 3\sigma_B|$. The investigation may include having one or both laboratories repeat their analyses. The reason for not including results for less than 1 pCi/g is that agreement at these very low levels is normally not a good indicator of laboratory quality and the above test almost always is met because of the large relative errors.



PMC and its on-site contractor have instituted a technician qualification program to assure that proper sampling and sample preparation procedures are followed.

Comment 5: Page 7 states that PMC will decide on the gamma survey method. PMC should indicate if the hand held Ludlum Model 19 data, using verification procedures, is as accurate and precise as the GPS-detector system.

Response: The Model 19 data is not as precise as the GPS-detector system. The primary reason for including this instrument in the study was to determine whether it was appropriate for use in excavation control activities. The Model 19 will not be used for verification measurements. See modified supplemental text page 8.

Comment 6: PMC proposes (page 9) to soil sample the 3 grids with the highest gamma level out of each 232 grids (1.3 %). This approach has been approved by NRC for other sites that used the GPS-gamma recording system. PMC should clarify what pattern the technician walks within the grid when using the hand-held instrument and justify that the one minute scan per grid detects sizable hot spots (adequate meter/operator response time).

Response: The GPS data is obtained by walking or driving parallel lines covering large areas with the position and count rate recorded and downloaded into a computer. The grid blocks are established within the GIS software. The traverse distance must be sufficient to obtain the minimum number of data values per grid block. In order to avoid repeating the survey, the traverse distance and speed are adjusted so that the data are uniformly spread across the grid blocks and that there are ample data points so that failure to meet the five (or seven in some cases) data points per grid block is very improbable. Normally, more than twelve data records per grid block are posted on the data maps.

To date, the one-minute integrated count method has not been used for verification. While demonstrated in this study to be equivalent to the GPS-recorded data, it will be used only if a few grids require verification and the GPS-recording system is not available. In that case, PMC will train the technician to scan the entire area looking for hot spots and remove the hot spots prior to making the measurements or taking the samples. In addition, gamma count rates will be recorded using the Ludlum Model 44-10 detectors at each of the sampling locations to document that the area has a uniform count rate.

Comment 7: Page 9 states that for windblown areas, a minimum of 5 data records have been shown to be sufficient. PMC should reference this demonstration and indicate if this statement is true for the Ludlum Model 19.

Response: PMC's on-site contractor, Environmental Restoration Group (ERG), applied this technique to another mill site (Homestake Mining Company, Grants Uranium Mill) where more than 1,000 acres were remediated using similar criteria. This criterion is being applied



at PMC's Lucky Mc Mill site which is in the final stages of verification. This statement does not apply to the Model 19. Also see responses to Comments 5 and 6.

Comment 8: PMC should indicate if the data will be determined to be normally distributed before the "student t" test (page 10) is used.

Response: PMC will determine if the data are normally distributed. See modified supplemental text page 10.

Comment 9: PMC should indicate what subsurface soil sampling will be done besides the 3 feet or deeper areas mentioned in procedure 03.022.01.

Response: The reference to three feet in the procedure was a depth at which back filling would normally be done. Where back fill will be applied, the 15 pCi/g plus background Ra-226 concentration limit will be applied. This procedure was written for application to these irregularly shaped excavations. Unless there is reason to suspect that there have been manmade disturbances in an area, there are no plans to sample below the top six-inch layer. It has been our experience (e. g. ground-water cleanup piping trenches at the Lucky Mc Mill Site) that it is quite evident from examining the gamma contour maps where disturbances have resulted in contaminating subsurface soils.

Comment 10: Procedure 03.020.01 (page 3) indicates a minimum of 7 gamma records MAY be required near the mill site or other areas where localized "hot" spots may be present. PMC should consider revising this to WILL be required.

Response: This change has been incorporated into the enclosed revised procedure.

Comment 11: Procedure 03.020.01 indicates action levels for shielded detectors that do not agree with values on page 5 of the plan. PMC should provide a page correction for the procedure.

Response: See the enclosed revised procedure 03.020.01.

Comment 12: PMC should address reporting requirements. For example, Ra-226 data should include the counting error at the 95 percent confidence level and the minimum detectable concentration should be reported for the gamma spectrometer. Also, any grids requiring further cleanup based on soil sampling, should be reported for inclusion in the Completion Report.

Response: PMC will provide this in the Completion Report.



Comment 13: PMC should address the delineation of and surveying of the unaffected area. The discussion should mention any off-pile disposal, releases, wind rose, haul routes, and prior survey results.

Response: The boundaries of the contaminated areas that were cleaned will be shown on the final gamma survey maps included in the Completion Report. Gamma data will be provided beyond the cleaned areas for a minimum distance of 250 feet. Where appropriate, other supporting data will be provided to demonstrate that the affected area has been delineated. Other site features such as haul routes, off-pile disposal areas, and structures will be addressed in the Completion Report.

Comment 14: PMC performed the gamma-Ra-226 correlation with a 9 sample soil composite, but the verification procedure indicates a 5-sample composite. PMC should indicate why the correlation derived gamma action level is appropriate to apply to the verification procedure.

Response: The correlation studies were conducted in the windblown areas of the Shirley Basin site after removing a very thin vegetation layer. The reason for the removal of the vegetation layer was to provide a more homogeneous contaminant distribution, representing more closely the conditions of the final radiological survey. This study had one source of error that had to be considered. The scraped areas were 100 m² in size and normally were not uniformly contaminated. This leads to a relatively large sampling error. The condition that is expected to exist after cleanup is that each grid block will be at background or have uniformly distributed residual contamination. Sampling such a grid block can be done with a very small sampling error, regardless of the number of samples obtained. PMC therefore wanted to design the experiment such that the sampling error for the study was approximately the same as the sampling error anticipated in the final verification surveys. We compensated for the heterogeneity of the study plots by increasing the number of samples to nine rather than five, thus reducing the sampling error in the study. believed, however, that the sampling error associated with the studies will still be larger than the sampling error during the verification sampling. This is acceptable in that it leads to a conservatively derived action level.

We should note that the uniformity of the contamination in the areas that have been cleaned will be assured by examining the gamma data at levels near the action level. Indicated hot spots above the action level will be removed.

Comment 15: Given the low coefficients of correlation and the low Ra-226 values used in the gamma-Ra-226 correlation, PMC should indicate what checks on the correlation will be performed during verification.

Response: PMC believes that these studies were well designed and executed. The results



reflect the normal errors one would anticipate, considering all the known influences. The study conditions were carefully designed to approximate those anticipated after cleanup. The action levels were obtained from the calculated 95 percent CL line and therefore considers the data scatter and lack of a perfect correlation. The results of all studies were internally consistent where expected, providing additional confidence that the study design was proper and the results valid.

While the average Ra-226 concentration for the plots was below the cleanup limit, the 95 percent CL action level determined from extrapolation agreed with that determined from the "point studies" which did have Ra-226 concentrations above the cleanup limit.

During cleanup, PMC expects that there will be very few grid blocks at or above the action levels. However, PMC will continue to evaluate the action levels based on the correlation with the results from the on-site gamma spectrometer. PMC recognizes that it is cost effective to reduce the action level early in the cleanup, if necessary, rather than run the risk of failure of the verification samples.

We hope that the above responses will lead to a quick approval of the Soil Cleanup Verification and Sampling Plan for the Shirley Basin mill site. In order to meet the license mandated schedule for completion of the windblown tailings retrieval we must begin actual cleanup activities next month. Pathfinder is particularly concerned about the potential for entering into an extended debate with the NRC about an appropriate background Ra-226 value. We are convinced that the suggested 3.0 pCi/g is reasonable and defendable. ERG will begin the initial gamma survey of the site on June 2, 1997. Please call me if there are any questions.

Sincerely,

T. W. Hardgrove

Coordinator of Mine Environmental Affairs

Enclosures

cc:

E. L. Nugent

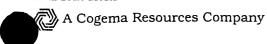
J. D. Wadsworth

R. W. Poyser

K. R. Baker, ERG

P. Mackin, CNWRA

PATHFINDER



November 20, 1996

Mr. Joseph J. Holonich, Chief Uranium Recovery Branch Division of Waste Management Office of Nuclear Material Safety and Safeguards U.S. Nuclear Regulatory Commission Washington, DC 20555-0001

Re:

Docket No. 40-6622

License No. SUA-442

Dear Mr. Holonich:

Enclosed please find five copies of an evaluation of the static and seismic slope stability and liquefaction potential of the previously submitted Shirley Basin Tailings Reclamation Plan. Accompanying the evaluation are revised reclamation plan text sheets for pages 6-3 through 6-5 and Section 9.0 which reference the evaluation. The seismic analysis represents one of the specified additional submittals to which you referred in your letter dated June 14, 1996. The soils cleanup protocol is currently under development and will be submitted in the near future.

We look forward to the receipt of NRC's review comments on the enclosed materials.

Sincerely,

T. W. Hardgrove

Coordinator of Mine Environmental Affairs

Enclosures

cc:

E. L. Nugent

J. D. Wadsworth

R. W. Poyser

Hydro-Engineering

Inberg-Miller Engineers w/o encl.

PATHFINDER

🕡 A Cogema Resources Company

May 22, 1996

Joseph J. Holonich, Chief Uranium Recovery Branch Division of Waste Management Office of Nuclear Material Safety and Safeguards U.S. Nuclear Regulatory Commission Washington, DC 20555

Re:

Docket No. 40-6622

License No. SUA-442

Dear Mr. Holonich:

Enclosed please find five sets of a revision to Pathfinder's Shirley Basin Tailings Reclamation Plan. This revision reflects changes that are appropriate in light of the NRC's approval of Pathfinder's Lucky Mc Mine Tailings Reclamation Plan, particularly as it applies to cover design and erosion protection. The Shirley Basin plan was modified so that the design methods are consistent between the two sites. This should facilitate the review and approval of the Shirley Basin plan. The concept of making the two plans consistent was previously discussed with Ted Johnson of your staff.

The identification and cleanup protocol for contaminated soils will be the subject of additional field investigation at Shirley Basin during the summer of 1996. A proposal on contaminated soils cleanup will be submitted to the NRC later this year. Additionally, a consultant is presently evaluating the seismic stability of the reclamation plan, and a submittal on that subject will be made upon completion of the evaluation.

Refer to the enclosed replacement directions sheet for the proper integration of the revisions into the previously submitted plan. Pathfinder is ready to aid the NRC staff in whatever way we can to facilitate its review of the enclosed plan.

Sincerely,

T. W. Hardgrove

Coordinator of Mine Environmental Affairs

Enclosures

CC:

E. L. Nugent

Tom Hadge

J. D. Wadsworth

R. W. Povser

Hydro-Engineering

REPLACEMENT AND ADDITIONAL PAGES AND EXHIBITS

The following table itemizes the enclosed revisions to the Shirley Basin Tailings Reclamation Plan and indicates the replacement pages for the document currently on file with the NRC.

Volume I Revisions

New Text/Exhibit Replace(s)

Volume I Cover Page Volume I Cover Page Volume I Table of Contents Volume I Table of Contents Pages 1-1 through 1-4 Pages 1-1 through 1-4 Exhibit 1-1 Exhibit 1-1 Exhibit 1-2 Exhibit 1-2 Pages 2-1 through 2-8 Pages 2-1 through 2-8 Pages 4-1 through 4-4 Pages 4-1 through 4-4 Entire Section 5 incl. Figures, Entire Section 5 incl. Figures, Tables and Exhibits Tables and Exhibits Pages 6-1 through 6-6 Pages 6-1 through 6-6 Pages 8-1 and 8-2 Pages 8-1 and 8-2

Pages 8-1 and 8-2
Pages 9-1 through 9-4
Entire Appendix E
New Appendix F

Pages 8-1 and 8-2
Pages 9-1 through 9-4
Entire Appendix E
Entire Appendix E

Volume II Revisions

New Text/Exhibit Replace(s)

Volume II Cover Page
Volume II Table of Contents

Volume II Table of Contents

Pages 10-1 through 10-4

Volume II Cover Page
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Pages 10-13 through 10-26

New Figure 10-6 (page 10-27)

New Figure 10-7 (page 10-28)

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New Exhibit 10-8
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New Figure 11-1 (page 11-8)

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SHIRLEY BASIN MINE TAILINGS RECLAMATION PLAN

SOURCE MATERIAL LICENSE NO. SUA-442

VOLUME I

SUBMITTED TO:

U.S. NUCLEAR REGULATORY COMMISSION OCTOBER, 1993

REVISED:
MAY 20, 1996
NOVEMBER 20, 1996
MAY 29, 1997
SEPTEMBER 10, 1997
NOVEMBER 24, 1997
SEPTEMBER 8, 1999

PREPARED FOR:

PATHFINDER MINES CORPORATION SHIRLEY BASIN MINE SHIRLEY BASIN, WYOMING

PREPARED BY:

HYDRO-ENGINEERING

GEORGE L. HOFFMAN

HYDROLOGIST

THOMAS G.

ENGINEER/HYDROLOGISTG

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1.0 INTRODUCTION

This reclamation plan has been developed with respect to the requirements in 10 CFR 40, Appendix A. Guidelines from the Nuclear Regulatory Commission (NRC) have also been used in the preparation of the reclamation plan. This plan covers the reclamation of Pathfinder Mines Corporation's Shirley Basin mill area, tailings facilities, and solution pond. Volume II, which contains Sections 10, 11 and 12, represents complete construction specifications for the project.

1.1 Description of Mill and Tailings Disposal Sites

Exhibit 1-1 shows the location of Pathfinder's Shirley Basin mill and tailings sites. The tailings area is directly in the center of the map, with solution pond No. 3, Tailings Pond No. 4, and Tailings Pond No. 5 distinguished by a heavy line around each of these three areas. Solution Pond No. 3 is in the southwest portion of the tailings and is the site presently being used for byproduct material disposal. Tailings Pond No. 5 is the eastern most pond and Tailings Pond No. 4 is between Ponds 3 and 5. mill area is adjacent to and just southwest of the Tailings No. 4 and No. 5. Decontamination and decommissioning of the mill is described in the mill decommissioning plan submitted to the NRC in June, 1992. The No. 3 pond basin has been used to contain only tailings solution, but this pond, as well as portions of pond basin 4, will be used for byproduct material disposal and will continue to be used as such until closure of the site. At that time, the byproduct material disposal area will be covered and included in the covered tailings area. Tailings thicknesses in Ponds 4 and 5 range up to 54 ft. The mill area to be reclaimed with cover material includes approximately 13.7 acres, and post-reclamation tailings piles No. 4 and 5 cover 110.7 and 149.5 respectively. Approximately 14.1 acres of No. 3 Pond will be

included in the tailings encapsulation system. All other areas will be restored to the radium cleanup standard. The proposed restricted area boundary is shown on Exhibit 1-1 relative to the cleanup areas.

The mill area includes an ore pad immediately south of the mill. The ore pad was mined prior to cessation of ore processing. any residual contaminated material in the ore pad area will be excavated and placed in the tailings during reclamation activities. The mill ore pad is approximately 600 feet wide and 900 feet from northwest to southeast. A low-grade waste pile, which has some contaminated materials in it, is located immediately to the west of Solution Pond No. 3. This material will also be placed in the tailings pond. The windblown area is to the northeast of the No. 5 tailings. An area of cleanup exists between the mill and the Area 2/8 pit. The majority of this area has been affected by the haulage of the ore from the pit to the mill.

Uranium milling started at this site in 1971 and continued until the last ore was processed in 1992. A total of some 8,564,130 tons of ore were milled at the Pathfinder Shirley Basin site. The mill utilized conventional acid leaching process. Exhibit 1-1 also shows the restricted area boundary associated with the facility.

1.2 Summary and Reclamation Plan Objectives

The objectives of the reclamation plan are to develop a reclamation scheme that will stabilize the tailings for at least 1000 years, and restore the mill and tailings pond area to 10 CFR Part 40, Appendix A standards. This plan also involves the restoration of areas which have been contaminated by windblown tailings or activities associated with processing of ore. Those areas which will be reclaimed to meet 10 CFR Part 40, Appendix A protection standards are presented on Exhibit 1-2. The restoration

areas are delineated by approximate boundaries. Ground survey control during construction will be used to precisely identify contaminated areas. The cross hatched area will be covered to meet the reclamation goals while the dotted area will be cleaned up to meet the release standards. The areas shown with pluses will be cleaned up with the mine reclamation.

The reclamation plan objectives will be met in the mill area by covering materials with elevated radioactivity. Mill rubble will be buried in trenches and covered with general fill or possibly low grade ore overlain by the tailings radon/infiltration barrier cover system. The ore pad will be excavated to reduce radioactivity to 8 pCi/g. The excavated materials will be placed in tailings. The low grade waste pile will be placed in tailings and that site reclaimed to 8 pCi/g.

The depth of tailings materials in the western portion of Tailings Pond No. 4 is expected to be relatively small. Those materials will be excavated and placed in other portions of Tailings Pond No. 4 or in Tailings Pond No. 5. Pond No. 3 will also be restored in the same manner with the byproduct disposal area being covered to meet the reclamation goals.

The reclamation plan for the solid tailings area utilizes present topography as much as practicable and add necessary cover materials of compacted clay, topsoil and/or rock mulch to produce an erosionally stable surface. The drainage design is intended to limit contributing areas to the tailings and to limit the size of individual drainage basins. This will minimize overland and channel flows. Portions of Tailings Pile No. 4 and Tailings Pile No. 5 with very flat slopes will be reclaimed without rock mulch cover. More than one-half of the No. 4 tailings pile will be rock mulched to maintain the stability of the cover. A larger portion of the No. 5 tailings area will be covered with rock mulch. The dam outslope areas for the No. 3 and No. 5 dams will be reduced to

a maximum of 5H:1V and covered with rock mulch to protect the slopes. Rock riprap will be used to stabilize all channel sections in the immediate area of the tailings and on steep channel sections in surrounding areas. Controlled discharge rock channel sections will be used the outlet from the No. 3 Solution Pond, and the north outlet from the No. 5 Tailings Pond. These structures will reduce downstream flows during peak flow periods and enhance erosional stability of the area. Drainage from areas north and south of the tailings is diverted away from the tailings, with incorporation of surge pond storage to enhance erosional stability.

An area of windblown contamination exists to the east and northeast of the No. 5 tailings. This material will be excavated and placed in one of the tailings piles.

THIS PAGE IS AN OVERSIZED DRAWING OR FIGURE,

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"EXHIBIT 1-2, RECLAMATION AREAS FOR THE SHIRLEY BASIN MILL AND TAILINGS, EXH1-2.DWG"

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"EXHIBIT 1-1, LOCATION MAP OF THE SHIRLEY BASIN MILL AND TAILINGS WITH SAMPLING SITES, EXH1-1.DWG"

WITHIN THIS PACKAGE... OR BY SEARCHING USING THE

2.0 RADIOLOGICAL SURVEY

In order to estimate average background radiological activity in the vicinity of the mill, tailings disposal and solution pond areas, and to define areal extent of elevated radioactivity, a radiological survey was undertaken. Included in this effort were a surface gamma survey, a test-hole drilling and logging program and a backhoe (test) trenching and radiometric testing program. Tailings samples were collected with Shelby tubes advanced with a drill rig. Bagged samples were collected from test holes and backhoe pits during the course of these programs for laboratory analysis.

Exhibit 1-1 is a map of the restricted area showing locations of test holes, test pits and other soil and tailings sample sites. The following naming sequence was used to distinguish sample types:

Prefix TW- indicates a tailings well drill site. Two wells were completed at most locations with the drill holes approximately 20' apart.

Prefix STH- indicates an Area 3 south dump test hole for cover material analysis.

Prefix NTH- indicates an Area 3 north dump test hole for cover material analysis.

Prefix NS- indicates a surficial well drill site on the north side of the No. 5 tailings.

Prefix P- indicates a backhoe test pit.

Prefix LGW- indicates a low grade waste pile test hole.

Prefix BP- indicates a below pond tailings sample.

Prefix TS- indicates a topsoil sample.

Prefix BGS- indicates a background gamma and radium sample.

Prefix GS- indicates a surface gamma and radium sample.

Prefix O- indicates a low grade waste pile backhoe pit.

Prefix OP- indicates an ore pad test hole.

Suffix R indicates an air quality sampling location.

Two hand-held gamma meters were used during the course of this study. Both instruments measure in microrems per hour $(\mu R/hr)$. Instrument #1, as indicated in Table 2-1, was used for the test pit gamma survey and instrument #2 was used for all remaining gamma surveys. Prior to the beginning of this study, instrument #2 was calibrated by Romaelab.

2.1 Background Radiological Analysis

Α background Ra-226 activity is needed allow to differentiation of naturally occurring radiological activity and that resulting from uranium milling activities. In order to determine the background activity, a sampling program was undertaken and past radiological studies were reviewed. The sample results are summarized below (see Exhibit 1-1 for sample site Samples designated "R" were collected routinely in locations). fulfillment of the environmental monitoring program. Location 07R is at the Heward Ranch some three miles east of the mill site.

LOCAT	ION	RA-226 (pCi/g)	LOCATION R	A-226 (pCi/g)
BGS-1		0.32	From Skinner 1982:	
BGS-2		0.35	BG (<2.0 mm)	3.0
BGS-3		0.43	D ₄ (<2.0 mm)	3.0
BGS-4		0.48	D _s (<2.0 mm)	2.0
04R	(1992)	1.6	• • •	
07R	(1992)	1.7	From Whicker 1981 an	d 1982:
04R	(1993)	4.4	Background	2.54
07R	(1993)	2.2	Sample #1	3.1
04R	(1994)	2.0	Sample #42	3.84
07R	(1994)	1.8	Sample #45	2.29
04R	(1995)	2.0	Sample #4	10.65
07R	(1995)	2.1	Sample #24	21.83
04R	(1996)	2.2	Reclamation Area (Sr) 9.73
07R	(1996)	1.8	•	

[See Exhibit 2-1 for location]

Overall Average for the Above Data = 3.56 pCi/g Ra-226.

In addition to the results on the left of the above table, the results of two previous studies are summarized on the right. Skinner (1982) presents a background Ra-226 activity of 3 pCi/g for the Shirley Basin site. Whicker (1981) presents a background soil Ra-226 activity from 2.29 to 3.84 pCi/g (samples "Background" and numbers 1, 42, and 45). The samples from mine reclamation areas as reported in Whicker (1981 and 1982) are represented by sample numbers 4, 24, and S_r . The overall average of the three sets of data is representative of a significant amount of sampling of background conditions at this site. As one might expect at a mill site located very close to an uranium mine, there is a great deal of variability in soil Ra-226 levels. Due to the proximity of the mine it is appropriate to factor the Ra-226 content of mine soils into the determination of background. The average of the three sets of data is 3.56 pCi/g. The radiological analyses support a background Ra-226 activity of 3 pCi/g. Therefore, the proposed release standard is background activity (3 pCi/q) plus 5, or 8 pCi/g.

2.2 Surface Gamma Survey

Hand-held gamma instruments were used to conduct a preliminary gamma survey for the mill, tailings disposal and solution pond areas. A series of transects were established between points of known location. Gamma measurements were taken at 100-foot intervals along each transect. Additional measurements were taken at points of interest such as high readings. Exhibit 2-1 presents the surface gamma data measured for the Shirley Basin site.

Samples of surface soil were collected in conjunction with the surface gamma survey. These samples were analyzed for Ra-226 activity. A correlation and regression analysis was done to relate laboratory Ra-226 activity, in pCi/g, to field hand-held gamma activity, in microrems per hour (μ R/hr). This relationship was developed to enable estimation of the areal extent of contamination and provide initial guidance for determining areas to be reclaimed. Actual reclamation extent will be confirmed based on analyses made during reclamation. Figure 2-1 shows a plot of field gamma versus lab Ra-226 with a line of best fit. For this particular fit, the largest gamma and radium value was eliminated as a high leverage point. Elimination of this point moved the intercept at gamma = 0

from 7.96 to -0.104 pCi/q, which gave a much better relationship for values of gamma less than 50 μ R/hr, which is the area of interest for windblown cleanup. The correlation coefficient for this relationship is 0.92, and the standard error of estimate is 16.6 pCi/q. Based on this relationship, the reclamation criterion Ra-226 activity of 8.0 pCi/g corresponds to a hand-held gamma meter Exhibit 2-1 presents the surface gamma value of 19.5 μ R/hr. readings collected at this site with the 20 μ R/hr contour. that this was a preliminary evaluation. A more detailed evaluation of the gamma/Ra-226 relationship at the site was conducted during September of 1996. The results of that evaluation are found in the supplemental "Soil Cleanup Verification and Sampling Plan for the Shirley Basin Mill Tailings Site", April, 1997. provides the guidance for the cleanup of contaminated soils at the Shirley Basin site.

2.2.1 Mill Area

Exhibit 2-1 presents gamma values in the mill area. The Ra-226 activity reclamation criterion is 8.0 pCi/g.

The ore pad drilling program was undertaken to determine the depth of contaminated material. Sixteen holes were drilled throughout the ore pad area to a depth of 22 feet, and gamma logs were taken for each hole. The location of ore pad test holes is presented in Exhibit 2-2. Gamma logs and predicted radium activity are presented in Appendix A. The gamma logs indicate depth of elevated radioactivity ranges from a few inches to approximately 8 feet. This material will be excavated until the radium level reaches 8 pCi/g and placed in tailings. The area near the mill (see Exhibit 2-1) will be reclaimed as part of the tailings area, therefore no gamma survey was done.

The gamma survey indicated the area west of the mill and south of the tailings ponds is contaminated in some regions. The expected depth of contamination is small and this area is proposed for cleanup. Contamination of windblown tailings in the area west of the mill is expected to be very thin because the prevailing winds are from the southwest. Mine haulage through this area had

the greatest potential for contamination of the area. Cleanup within the restricted boundary will be treated as part of the tailings reclamation while cleanup south of the restricted boundary will be part of the mine reclamation. The windblown area west of the solution pond will be reclaimed to the edge of the Area 2/8 pit limit as part of the tailings reclamation, while the area below the pit crest will be part of the mine reclamation. The low grade ore pile located between the tailings and the Area 2/8 pit will be excavated and placed in tailings.

2.2.2 Windblown Tailings

Surface gamma readings from the area surrounding the edge of the tailings are presented on Exhibit 2-1. The windblown areas appear to be east and north of the No. 5 tailings area. Gamma values in this area are generally less than 16 $\mu R/hr$. Exhibit 1-2 presents the preliminary limits of the area contaminated with windblown tailings which require restoration to the 8.0 pCi/g standard of radium. The estimated average cleanup depth of the windblown areas is 2 inches, but the limitations of machinery used to remove this material will likely necessitate removal of a slightly greater thickness.

2.3 Test Holes

Fifteen test holes were completed in tailings on Ponds 4 and 5, three were completed adjacent to the tailings, sixteen were completed in the ore pad area, five were completed in the low-grade waste pile and the rest were advanced in the nearby overburden piles (North and South dumps). Exhibits 1-1 and 2-2 show the locations of all 59 holes.

Samples of drill cuttings were collected at 5-ft intervals for tailings and some of the cover materials, and Shelby tube samples were taken in the tailings ponds test holes.

Table 2-1 presents a summary of all radiological analyses for test-hole samples. Corresponding values of downhole gamma are also presented on Table 2-1. Copies of laboratory reports are included in Appendix D. Appendix A contains lithologic logs, geophysical logs and downhole profiles of estimated Ra-226 activities.

A correlation and regression analysis was made relating downhole gamma measurements, in counts per second (cps), to laboratory measured Ra-226 activity (pCi/g). This analysis was done to provide guidance for reclamation planning purposes on the vertical extent of contamination. Figure 2-2 is a plot of Ra-226 versus gamma with two lines of best fit. The relationship for gamma values less than 560 cps was determined with a zero intercept linear regression, and the relationship for gamma exceeding 560 cps was determined with a standard linear regression. Based on these relationships, a gamma reading of approximately 565 cps corresponds to the restoration standard for Ra-226 of 8.0 pCi/g.

2.4 Test Pits

Forty-seven backhoe pits were dug in the mill and tailings area to determine the location and depth of tailings or contaminated material (see Exhibit 1-1 for locations). Typically, pits were dug to a depth of 5 to 7 feet. Hand-held gamma readings were taken at 0.5' and 1' intervals and some bagged samples were collected. Twenty-four of the backhoe pit bagged samples were sent to the laboratory where they were analyzed for radium (Ra-226) activity. Table 2-1 presents a summary of laboratory results for the backhoe pit samples. Copies of the laboratory radiological reports are included in Appendix D and the backhoe pit gamma profiles are presented Appendix B.

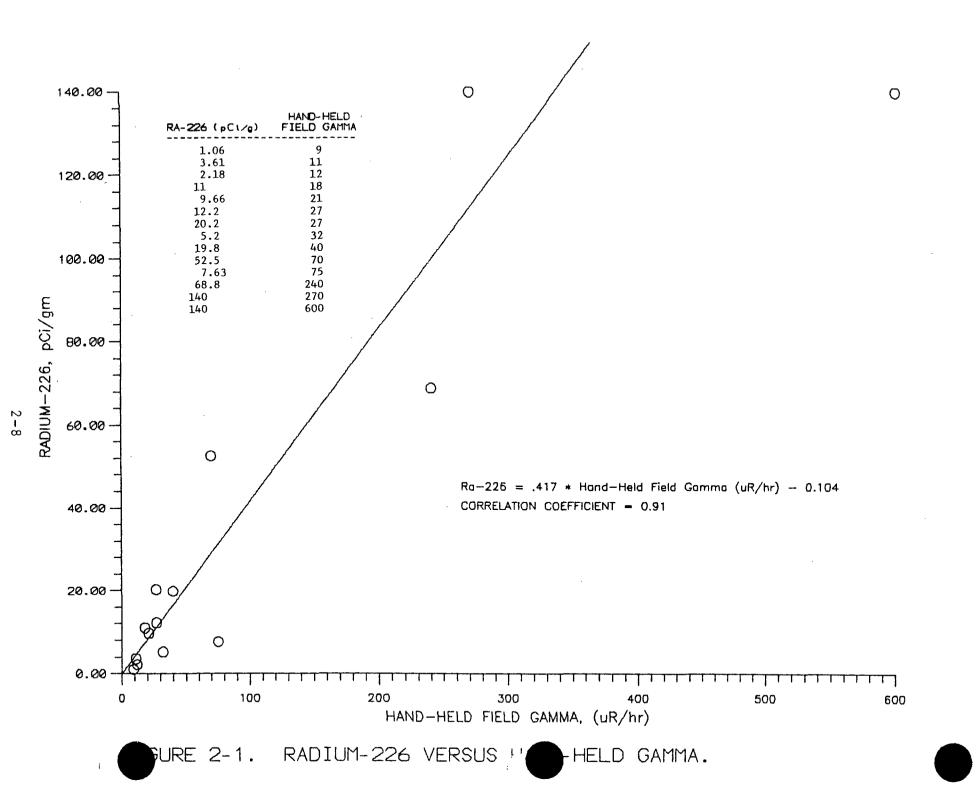
A correlation of radium-226 and the hand-held gamma readings from the backhoe pits was attempted. However, the smearing effect of gamma values on the exposed pit face made such an analysis meaningless. The gamma values and corresponding radium-226 values are presented in Figure 2-3 and it is apparent that readings for a specific layer are biased by the activities in adjacent layers. With this in mind, the gamma profiles were used qualitatively as an indication of the presence of tailings and the depth of tailings. For example, Pit P4-11A indicates the presence of tailings to a

depth of at least 3' with a transition to background gamma between 3' and 5' in depth. This particular area will be progressively excavated until the tailings are removed as indicated by final radium-226 sampling. At other locations, the tailings depth was too great to allow cost effective removal, and these were included within the covered tailings area.

The backhoe pits served to identify areas where the depth of tailings or depth of contamination is limited. In particular, the gamma profiles from the test pits indicate that there is only a small thickness of contaminated material in the No. 3 solution pond and on the west side of Tailings Pond 4. Contaminated soils in these areas will be excavated to meet the radium standard. The area reclaimed in this manner will not be included in the covered tailings area which is subject to the radon barrier standard. The limit of subsoil contamination from the pits below the ponded surface of Pond No. 4 on the west side is unknown at this juncture because the tailings solution prevents access to areas below the water line. Therefore, the restoration area shown in Exhibit 1-2 will be modified as necessary to ensure restoration to the appropriate standard.

2.5 BELOW POND SAMPLING

A program to sample the tailings that are presently covered by ponded water was conducted. Twelve samples were taken from a boat with a tube sampling device. The pond sampling locations (BP-) shown on Exhibit 1-1 are approximate due to the nature of the sampling technique. The samples indicate Ra-226 activities for the material near the base of the ponds ranges from 34.6 to 737 pCi/g. The results of this sampling program are discussed in section 4.



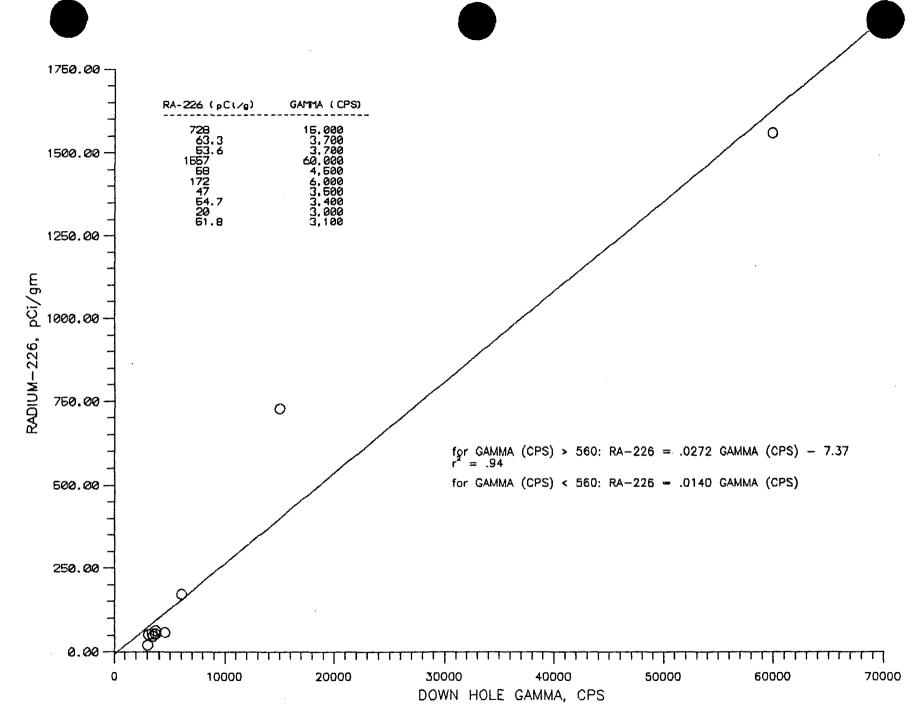


FIGURE 2-2. RADIUM-226 VERSUS DOWN HOLE GAMMA.



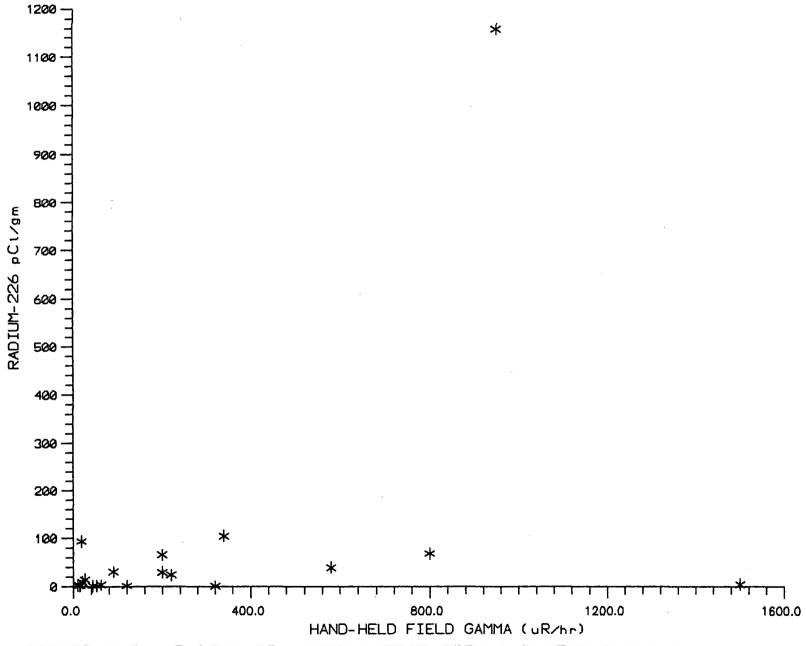


FIGURE 2-3. RADIUM-226 VERSUS TEST PIT HAND-HELD GAMMA READINGS.

TABLE 2-1. SUMMARY OF GAMMA AND RADIUM-226 ANALYSES.

SAMPLE ID	RA-226 CHEMICAL pCi/gm	U-NAT FIELD pCi/gm	FIELD FIELD pCi/gm (µR/hr) (ins. #1)		DOWN HOLE GAMMA cps
TW4-1B 25′-27′	728				15,000
TW4-1B 3´-5´	63.3		,		3,700
TW4-1C 3´-5´	53.6				3,700
TW4-2C 4Ø^-42^	1557				60,000
TW4-4B 5'-7'	58		<i>:</i>		4,500
TW4-5B 5.51-81	172				6,000
TW5-1B 4'-6'	47				3,5 <i>00</i> **
TW5-1B 6'-8'	54.7				3,4 <i>00</i> **
TW5-2B 3´	2Ø				3,000
TW5-3 6'-8'	51.8				3,100
0–1 9ø"–96"	15	43.5	9Ø		
0–2 1 <i>0</i> 8"–114"	194	158	28Ø		
0–2 66"–72"	32.2		14Ø		
P3-1 Ø"-6"	14.9	1.2	26		
P3-1 48"-54"	1.9	1.6	14		

TABLE 2-1. SUMMARY OF GAMMA AND RADIUM-226 ANALYSES (continued).

SAMPLE ID		U-NAT GAMMA GAMMA DOWN HOLE FIELD FIELD FIELD GAMMA pCi/gm (µR/hr) (µR/hr) cps (ins. #1) (ins. #2)
P4-12B 18"-24"	1.7	43
P4-12B 78"-84"	Ø.3	1Ø
P4-12C 24"-3Ø"	3Ø.9	9Ø
P4-1ØA Ø''-6"	29.9	200
P4-1ØA 78"-74"	1.9	120
P4-6A 6Ø''	1.Ø	32Ø
P4-8A 24''-3Ø''	40	58Ø
P4-5D 42"-48"	4.7	1500
P4-12B 36"-42"	9.6	2Ø
P4-12C 6"-12"	4.2	62
P4-12C 48"-54"	23.8	220
P4-12A 36"-42"	93.5	18
P4-8B 48"	1Ø5	34Ø
P4-8A Ø"-6"	65.8	200
P4-5D 38"-42"	1157	95Ø

TABLE 2-1. SUMMARY OF GAMMA AND RADIUM-226 ANALYSES (continued).

SAMPLE ID	RA-226 CHEMICAL pCi/gm	U-NAT FIELD	GAMMA FIELD (µR/hr) (ins. #1)	$(\mu R/hr)$	GAMMA
P4-4D 36"-42"	68.7		800		
P4-12B 48"-54"	2.1	20.4	14		
P4-12B 72"-78"	4.2	2.3	1Ø		
P4-12C 72"-78"	1.8	11.9	52		
P4-8A 48"-54"	1.0		42		·
P4-8A 6Ø"-66"	Ø.4		42		
P4-8A 78"-84"	Ø.3		26		
STH-6 20^-27^	2.0/5				
NTH-1 60'-67'	2.99				
STH-5 40^-47^	2.26				
GS-1 surface	1.Ø6			9	
GS-2 surface	12.2			27	
GS-3 surface	5.2			32	
GS-4 surface	7.63			75	
GS-5 surface	140			6ØØ	

TABLE 2-1. SUMMARY OF GAMMA AND RADIUM-226 ANALYSES (continued).

					======
SAMPLE ID	RA-226 CHEMICAL pCi/gm	pCi/gm	FIELD (µR/hr) (ins. #1)	(µR/hr)	AMMA
GS-6 surface	68.8			24Ø	
GS-7 surface	11			18	
GS-8 surface	9.66			21	
GS-9 surface	20.2			27	
GS-1Ø surface	19.8			4Ø	
GS-11 surface	3.61			11	
GS-12 surface	2.18		·	12	
GS-13 surface	52.5			7Ø	
GS-14 surface	14Ø			27Ø	
BP-1	214			35*	
BP-2	34.6	,		14*	
BP-3	66		•	16*	
BP-4	1Ø2			14*	
BP-5	55			14*	
BP-6	438			2Ø*	
BP-7	2Ø6			22*	
BP-8	481			3∕2≯	
BP-9	394			6Ø*	

TABLE 2-1. SUMMARY OF GAMMA AND RADIUM-226 ANALYSES (continued).

==========					===
SAMPLE ID	RA-226 CHEMICAL pCi/gm	U-NAT FIELD pCi/gm	GAMMA FIELD (µR/hr) (ins. #1)		
BP-1Ø	414			27*	
BP-11	737			120*	
BP-12	565			31*	
BGS-1	Ø.32			1Ø	
BGS-2	Ø.35			11	
BGS-3	Ø.43			11	
BGS-4	Ø.48			9	
STH-6 20~-27~	2.Ø5				
NTH-1 60'-67'	2.99				
STH-5 40′-47′	2.26				

^{*} Gamma readings taken outside of bagged samples.

^{**} Estimated from adjacent well log.

TABLE 2-2. DEPTHS OF TAILINGS IN TEST HOLES HOLE TAILINGS DESIGNATION DEPTH (FT) TW4-1C 44 TW4-2C 44 49 TW4-3C TW4-4C 28 34 TW4-5C TW5-1C 53 TW5-2C 38 TW5-3 43

3.Ø MATERIALS PHYSICAL PROPERTIES

Physical and radiological properties of tailings, and various cover materials have been determined. Results of these determinations are presented and discussed below. Laboratory reports of materials' physical properties are presented in Appendix C, and radiological reports are included in Appendix D. The sampling programs were discussed in Section 2.0.

3.1 Properties of Tailings

Samples of tailings were collected by a variety of methods, including shelby tube samples taken during drilling, grab samples taken during test pit analysis, and below pond samples taken from a boat. A summary of the tailings physical and radiological properties is presented in Table 3-1. The shelby tube samples are designated with a TW prefix, the pit samples are designated with a P prefix for the tailings area and an O prefix for the low-grade waste pile, and the below pond samples are designated with a BP prefix. For the purposes of this analysis, the tailings pit samples (P prefix) were considered tailings if the radium 226 activity was in excess of 20 pCi/gm. The 20 pCi/gm threshold was intended to exclude samples which were diluted with non-tailings material from the radon flux simulations.

Those samples which can be considered clayey tails or slimes are TW4-2C, 4Ø-42' and P4-5D, 36"-42". Sample TW4-3C, 42-43' is also considered a slime layer, but no radiological properties were determined for it. The below pond samples (BP prefix) all appeared to be made up of very fine materials, but some of the samples were taken from an area where very little tailings deposition occurred and the proportion of tailings material within the sample is unknown. The actual sample may have been a combination of tailings fines and natural soils. This is expected to be the case with samples BP-2 through BP-5 which were taken from the west side of Tailings Pond No. 4 where no sand tailings were placed. The pit samples from the low-grade waste pile (O prefix) represent residual ore and other contaminated materials.

The value of the emanation coefficient for the samples ranged from Ø to Ø.1934. There appeared to be no correlation between the gradation of the material and the value of the emanation The emanation coefficient for the low-grade waste coefficient. pile samples was more consistent and slightly greater than average emanation coefficients for tailings samples. Duplicate emanation coefficient measurements were made at reconstituted moisture contents for four of the tailings samples (TW4-1B, 25'-27'; TW4-5B, 5.5'-8'; TW4-2C, 40'-42'; and TW5-2B,3'). This was done because Nielson et al. (1982) indicates that the emanation coefficient varies with moisture content of the sample. The emanation coefficient for the original samples was determined at the asreceived moisture content, which was well in excess of 6% for all The emanation coefficients for each sample changed samples. dramatically when duplicate measurements were made, but there did not appear to be any correlation with change in moisture content. The differences in emanation coefficients for the duplicate samples are attributed to natural variations inherent in the measurement technique. It is interesting to note that the average emanation coefficient for those samples with duplicate measurements was Ø.Ø71 for measurements at natural moisture content and 0.084 for measurements at reconstituted moisture content. Both values are reasonably consistent with the overall sample average of Ø.086.

3.2 Properties of Cover Materials

Potential sources of cover material include the South Dump and North Dump. The cover material properties are presented in Table 3-2. The samples designated with a CP, Set, and STH prefix were taken from the South Dump and the samples designated with a prefix of ND or NTH were taken from the North Dump. The STH and NTH samples were taken in a drilling program for the dump piles and lithology for each of the holes is presented in Appendix A. The remainder of the samples were taken from backhoe pits or at the surface. Based on the lithologic logs and the properties presented in Table 3-2, it appears that a bluish gray clay is present in the

South dump in adequate quantities to be used as the primary cover material. The same material is present in the North dump in lesser quantities, but the primary borrow area is the South dump and the bluish gray clay in the North dump will not be used as cover unless volume or economic considerations make it a viable alternative. Those samples which are representative of the proposed clay cover material are the STH-2, STH-4, STH-5, STH-6 and STH-11 samples presented in Table 3-2. The NTH-5, 20'-27', sample is representative of the same type of material from the North Dump.

Rawls and Brakensiek (1982) presented an empirical technique for determining long-term volumetric moisture content of soil corresponding to the 15-bar soil water retention value, which is as follows:

 $VMC = \emptyset.026 + \emptyset.005 z + \emptyset.0158 y$ where:

VMC = volumetric moisture content, z = % of clay in the soil, and y = % of organic matter in the soil.

Volumetric soil moisture content can be converted to moisture content by weight (Wc) by utilizing the following equation:

Wc = 100 (VMC)(pw) / pc
where:

pw = density of water (unity) and
pc = density of soil.

The predicted long term moisture content for each cover material as calculated by this method is presented in Table 3-2. The predicted long-term moisture content for the five samples representative of the proposed cover material varied from 16.6 to 21.7% with a mean of 18.74%.

3.3 Rock Testing

Rock quality testing for two general rock sources was conducted on a total of four samples. Petrographic analysis was

conducted on one sample of granite and one sample of sandstone. The petrographic analysis indicated that the granite was of good quality with no serious deleterious conditions, while the sandstone was of questionable quality.

Rock durability analysis was conducted on three samples of the granite and one sample of the sandstone. Tests conducted were: Los Angeles Abrasion (100 revolutions, ASTM C-535), Specific Gravity (surface saturated dry, ASTM C-127), Absorption (in conjunction with Specific Gravity test), and Sodium Sulfate Soundness (5 cycles, ASTM C-88). The results are presented on Table 3-3 with calculated rock durability scores. Values from Table D1 in the Staff Technical Position (STP) (NRC, 1990) were interpolated in computing these scores. The durability testing indicates the granite is of very good quality while the sandstone (Rock D) was of fair to good quality. The granite will be used for rock mulch and riprap in the tailings reclamation area.

The proposed filter material for rock mulch rock and riprap is crushed granite and finer screen materials from the rock processing operations. Rock and filter sizing and gradation criteria are discussed in Section 11.

TABLE 3-1. TAILINGS PHYSICAL AND RADIOLOGICAL PROPERTIES.

SAMPLE	NATURAL MOIST (%)	RA-226 (pCi/g)	DENSITY (lb/ft3)	DENSITY (g/cm3)	CALC. POROS- ITY	-200 SIEVE PERCENT	NAT. MOIST EMAN- ATION COEFF	RECON. MOIST. EMAN- ATION COEFF
TAILINGS S	SAMPLES							
TW4-1B 25' - 27'	36	728	85	1.36	0.49	50	0.0166	0.1513 @ 20%
TW4-4B 5' - 7'	20	58	93	1.49	0.44	8	0.0552	
TW4-5B 5.5' - 8'	16	172	91	1.46	0.45	16	0.1934	0.099@6%
TW4-1C 3' - 5'	20	53.6	94	1.51	0.43	5	0.1246	
TW4-2C 40' - 42'	80	1557	55	0.88	0.67	77	0.0041	0.087 @ 20%
TW4-3C 42' - 43'	62		62	0.99	0.63	100		
TW5-2B 3'	27	20	89	1.43	0.46	6	0.0699	0 @ 6%
TW4-1B 3' - 5'		63.3					0.1423	
TW5-1B 4' - 6'		47					0.1005	
TW5-3 6' - 8'		54.7					0.0254	
TW5-1B 6' - 8'		51.8						
PIT SAMPL	.ES							
P4-12C 24" - 30"		30.9						
P4-10A 0" - 6"	# 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	29.9					series Roje S	
P4-8A 24" - 30"	and the second second	40				Marine Sank in a	er e e e e e e e e e e e e e e e e e e	· · · · · · · · · · · · · · · · · · ·

TABLE 3-1. TAILINGS PHYSICAL AND RADIOLOGICAL PROPERTIES (continued).

SAMPLE	NATURAL MOIST (%)	RA-226 (pCi/g)	DENSITY DENSITY (lb/ft³) (g/cm³)	CALC POROS- ITY	-200 SIEVE PERCENT	Nat Noist. EMAN- ATION COEFF	Recon. Moist. EMAN- ATION COEFF
P4-12C 48"-54"		23.8	·				
P4-12A 36"-42"		93.5					
P4-8B 48"		1Ø5				· ·	
P4-8A Ø"-6"		65.8					•
P4-5D 36"-42"		1157					,
P4-4D 36"-42"		68.7	·	v v			
ORE SAMPLES							
0-1 9ø"-96"		15				Ø.153	
0-2 1Ø8"-114"		194				Ø.166	
0–2 66"–72"		32.2	·			Ø.113	
BELOW POND SAL	PLES :			* 4 **			
BP-1		214					e de la companya de l
BP-2		34.6			•		
BP-3		66		•			
BP-4	1	1Ø2			(Ø.Ø221	
BP-5		55				terniciae de la certe de recolera.	The first of the f
BP-6		438					

TABLE 3-1. TAILINGS PHYSICAL AND RADIOLOGICAL PROPERTIES (continued).

SAMPLE		DENSITY DENSITY (1b/ft³) (g/cm³)	CALC POROS- ITY	-200 SIEVE PERCENT	Nat Noist. EMAN- ATION COEFF	Recon. Moist. EMAN- ATION COEFF
BP-7	2016					5
BP-8	481					
BP-9	394			•	Ø.1658	
BP-1Ø	414					
BP-11	737				Ø.Ø34	
BP-12	565					

TABLE 3-2. COVER MATERIAL PHYSICAL PROPERTIES.

	Gradation													
		Mat.	Org.	Liquid	Plast.		-#4	- (Class.	100%	95%	Cla	y Spec	. Rawls Rawls
Sample	Depth	Moist	Matter	Limit	Index	+84	+\$200	-\$200		Proctor	Proctor	002	Grav.	Theta W.C.
	(ft)	(%)	(%)	(%)	(%)	(%)	(%)	(%)		(1b/ft3)	(lb/ft3)	(%)		(%) (%)
CP-1	10	14		56		8	25	75	CL	92.8	88.16	8	2.63	13.1 9.2
CP-2	19	16	3.72	50		8	25	75	CL	96.4	91.58	6	2.61	11.4 7.8
CP-3	10	15	2.76	52	27	8	23	77	CL	94.9	90.155	8	2.67	10.9 7.5
Set 1	1		8.8			Ø	13	87	CŁ	92.5	87.875	48	2.68	40.3 28.6
Set 2	1		8			Ø	9	91	CL	93	88.35	48	2.672	35.1 24.8
Set 3	1		7.1			ø	11	89	CL	95	90.25	62	2.702	44.7 30.9
STH-2	69-67	18	7.2	54	34	Ø	17	83	CH	102	96.9	24		25.8 16.6
STH-4	30-37	15	6.2	61	40	Ø	8	92	CH	100.7	95.665	38		31.3 20.4
STH-5	49-47	9	6.2	58	35	8	7	93	CH	99.2	94.24	41		32.8 21.7
STH-6	20-27	6	5.9	45	25	Ø	17	83	CL	103.1	97.945	30		26.8 17.1
STH-7	48-47		4.2	57	37	Ø	45	55	CH	110	104.5	12	2.36	15.2 9.0
STH-11	20-27	13	10.4	64	37	ø	4	96	CH	91.3	86.735	12		24.8 17.9
ND-1	19	30	3.6	59		1.	22	77	CL	92.4	87.78	6	2.68	11.2 8.0
ND-2	19	25	2.7	64		ø	27	73	CL	89.4	84.93	7	2.69	19.3 7.6
ND-3	10	35	3.7	75		1	23	76	CL	83.2	79.04	5	2.6	10.9 8.6
ND-4	10	36	3.1	69		Ø	25	75	CL	82.7	78.565	5	2.66	9.9 7.9
ND-5	10	23	3.3	61		9	14	86	CL	94.5	89.775	26	2.66	20.7 14.4
ND-6	19	23	2.6	65		1	27	72	CL	91.2	86.64	7	2.63	10.2 7.3
NTH-1	69-67			65		0	15	85	CH	96.8	91.96	29	2.31	17.1 11.6
NTH-3	140-14	6		42		10	61	29	CL	117.7	111.815	1	2.36	3.1 6.0
NTH-5	29-27	16	10	65		ø	10	90	CH	94.1	89.395	31		33.7 23.5
NTH-5	120-12			55		7	45	48	CH	108.5	103.075	2		3.6 6.0
				•••		•						-		

TABLE 3-3. ROCK DURABILITY PROPERTIES AND SCORING.

SAMPLE	L.A. ABRASION * LOSS	SODIUM SULPATE SOUNDNESS * LOSS	S.S.D.: SPECIFIC GRAVITY	ABSORPTION
Rock A				
(Granite)	2.2	0.94	2.64	0.12
Rock B				
(Granite)	2.8	0.11	2.64	9.14
Rock C				
(Granite)	2.8	9.98	2.65	9.98
Rock D				
(Sandstone)	4.2	0.18	2.65	2.47

ROCK SCORING

SAMPLE	L.A. ABRASION SCORE x NT	SODION SULFATE SOUNDNESS SCORE x NT	S.S.D. SPECIFIC GRAVITY SCORE x WT	ABSORPTION SCORE x WT	TOTAL	SCORE X
Rock A (Granite)	9.4 x 1	1 6 x 11	7.8 x 9	9.9 x 2	209.4	91
Rock B (Granite)	9.1 x 1	10 x 11	7.8 x 9	9.8 x 2	208.9	90.8
Rock C (Granite)	9.1 x 1	10 x 11	8 x 9	10 x 2	211.1	91.8
Rock D (Sandstone)	8.4 x 1	10 x 11	8 x 9	2 x 2	194.4	84.5

4.0 RADON BARRIER

The tailings area radon barrier will consist of a layer of compacted clay as described in Section 3.2 and an overlying sandy layer which will act as a capillary barrier. The cover material properties were summarized in Table 3-2. The radon barrier cover will be applied to tailings pile areas, byproduct material disposal areas, mill site, and mill rubble disposal areas. The cover will be designed to limit the radon surface flux when averaged over the entire cover area to $20 \text{ pCi/m}^2/\text{s}$.

The RADON computer program, described in NRC Regulatory Guide 3.64 (1989), was used to predict the cover thickness required to achieve the radon flux standard. Topsoil materials and rock mulch/filter materials were not included in the radon barrier simulations.

4.1 RADON MODELING TAILINGS PHYSICAL AND RADIOLOGICAL PROPERTIES

Because of the variability in tailings properties, two scenarios were considered in the radon flux modeling for the tailings area. The first scenario uses the near surface samples taken during the well drilling program. This is considered representative of the currently exposed tailings radon flux. The second scenario uses the tailings properties indicated by the below pond sampling program. The measured radiological properties of tailings samples were summarized in Table 3-1. A summary of the results of each simulation is presented in Table 4-1.

4.2 RADON MODELING COVER MATERIALS

The clay cover material is best represented by samples STH-2, 60'-67'; STH-4, 30'-37'; STH-5, 40'-47'; STH-6, 20'-27'; and STH-11, 20'-27' as presented in Table 3-2. These samples appear to be representative of the bulk of the material in the South Dump that

will be used to cover tailings. Sample STH-7, 40'-47' was deliberately selected as a sandy lense within the borrow area and is an example of the type of material which could be used as the sandy capillary barrier at the top of the radon barrier. The lithologic logs for the South Dump borrow area test holes indicate that selective handling of the cover material will be required.

Model inputs for the cover material properties were computed as conservative average properties from the borrow area samples. The average long-term moisture content as calculated by the method presented by Rawls and Brakensiek (1982) for the five clay samples indicated above was 18.74%. The standard deviation of the moisture content for the same five samples was 1.97%. The minimum estimated long-term moisture content is 16.6% for the samples and the average moisture content minus one standard deviation gives a moisture content of 16.77%. The moisture content of the sandy cover material was estimated as 6% to give a worst case scenario. Since the RADON model is particularly sensitive to moisture content of the cover material, this represents a very conservative condition.

The average density of the clay cover material was estimated as 1.5 gm/cm³ (based on 95% proctor density of samples). The average density of the sandy cover material was estimated as 1.59 gm/cm³. Porosity of the clay cover was calculated as 0.43 and the porosity of the sandy cover was assumed to be 0.40. The diffusion coefficient for tailings and cover materials is calculated by the model.

4.3 RADON MODELING SCENARIO ONE

The first scenario was based on properties for the exposed near-surface tailings as determined from shelby tube samples taken during the tailings drilling program. For the purposes of this modeling, the properties of samples TW4-2C, 40'-42'; and TW4-1B, 25'-27' were excluded from computations. This was done because

these samples were taken from depths of 25 feet below the surface and greater and are not considered representative of near-surface tailings. Gamma logs for each of the tailings wells (Appendix A) indicate that the near-surface samples are representative of the gamma emission to a depth of at least 20 ft.

The average radium activity for eight tailings well samples (TW prefix) with the exclusion noted above was 65.05 pCi/gm with a standard deviation of 42.2 pCi/gm. The average emanation coefficient for all samples was 0.086, and a value of 0.10 was considered a conservative input for the model. The average plus one standard deviation was considered a conservative estimation technique for the radium activity, yielding an activity of 107 pCi/gm as an input for the RADON model. The average density of the tailings was assumed to be 1.59 gm/cm³ and the porosity was assumed to be 0.40. The long-term moisture content of the tailings was assumed to be 6%.

The clay cover material was assumed to have a moisture content of 16.77% (the average moisture content minus one standard deviation), a density of $1.5~\rm gm/cm^3$ (95% proctor for proposed cover material) and porosity of 0.43 (calculated from other cover material properties). The results in Table 4-1 indicate that $2.5~\rm ft$ of clay cover overlain with $0.5~\rm ft$ of sandy cover is sufficient to limit radon flux for this scenario to $6.6~\rm pCi/m^2/s$.

4.4 RADON MODELING SCENARIO TWO

The second modeling scenario utilizes the average activity of 309 pCi/g for the below pond tailings sampling in tailings ponds 4 and 5. These samples represent the more active fine materials and the tailings moisture content was assumed to be 12%. The thickness of these fine materials was assumed to be 500 cm, although it is unlikely that the fines would be deposited to this thickness. All other properties were the same as those described in scenario one.

The resulting radon flux with the a cover consisting of 3 feet of clay overlain by 0.5 feet of sandy cover is $14.7 \text{ pCi/m}^2/\text{s}$.

4.5 RESULTS AND DISCUSSION

The results of the RADON model runs are included as Table 4-1. A three feet thick cover system consisting of a 2.5 feet thick clay cover and a 0.5 feet thick sandy capillary barrier will be more than adequate for the exposed tailings. This cover system will also be used in the mill area. An additional 0.5 feet of clay will be added for areas of the tailings that are currently below the pond water level.

The chimney drain for the No. 5 tailings dam was constructed with sand tailings and must be addressed in terms of radon flux modeling. The chimney drain was constructed as shown in Figure 4-1 and the sandy tailings will be excavated as shown to provide a minimum of 3.5 feet of cover with reclamation contours. section shown in Figure 4-1 is approximately the location where the maximum depth of tailings will have to be excavated. An additional RADON simulation was conducted to demonstrate that excavation of tailings at the top the dam followed by covering with material graded from the dam outslope will limit radon flux to below 20 $pCi/m^2/s$. The tailings properties were assumed to be the same as those used in scenario one, and the cover material was assumed to be a very sandy material (moisture content = 6%) to give a very conservative radon flux condition. The resulting radon flux for 3.5 feet of cover with graded sand from the dam outslope is 17.8 pCi/m²/s. The majority of the chimney drain will have 15 to 20 feet of cover, and the radon flux will be much less than the maximum predicted value of 17.8 pCi/m²/s.

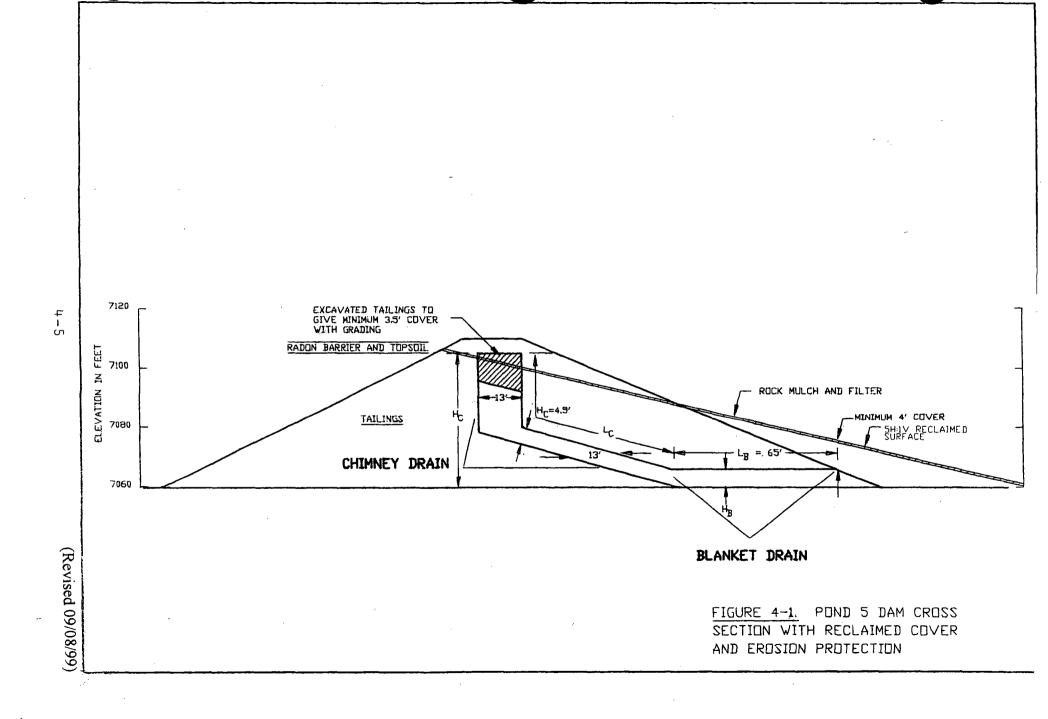


TABLE 4-1. RADON PROGRAM INPUT PARAMETERS AND RESULTS.

SCENARIO LAYER	THICKNESS (ft)	POROSITY	MASS DENSITY R (g/cm³)		EMAN- ATION (g) COEFF		EXIT RADON UX FROM LAYI (pCi/m²/s)	
1 TAILINGS-SAND	16.4	. 4Ø	1.59	1Ø7	Ø.1ØØ	6	13.Ø	
CLAY COVER	2.5	. 43	1.5	Ø	Ø.35Ø	16.77	6.7	
SAND COVER	Ø.5	. 4Ø	1.59	Ø	Ø.35Ø	6	6.6	
2 TAILINGS-SAND	16.4	. 4Ø	1.59	3Ø9	Ø.100	12	36.Ø	
CLAY COVER	3.Ø	. 43	1.5	Ø	Ø.350	16.77	14.8	
SAND COVER	Ø.5	. 4Ø	1.59	Ø	Ø.350	6	14.7	
CHIMNEY DRAIN TAILINGS-SAND COVER	16.4 3.5	. 4Ø . 4Ø	1.59 1.6Ø	1Ø7 Ø	Ø.100 Ø.350	6 6	25.1 17.8	

5.0 RECLAIMED SURFACE DESIGN

The tailings reclamation plan is designed to minimize the areas of cut and fill and to utilize existing topography to form drainages. The majority of the tailings surface will be protected with rock mulch cover and the remainder will consist of very flat non-rock mulch areas at the crest of some drainage basins on the tailings surface. The drainage area from the mill, tailings disposal and adjacent drainages has been divided into five major basins with 34 subbasins. The reclaimed surface maps, Exhibits 5-1, 5-1A and 5-1B, show the drainage basin and subbasin divides. Exhibit 5-2 presents major drainage basins designated by color and settlement monument locations. The major basins are designated: West Tailings Drainage Area, South Drainage Area, North Drainage Area, East Tailings Drainage Area, and Southeast Tailings Drainage Area.

Those subbasins in the mill area and other areas south of the tailings have been given a DM- prefix, followed by a sequence number. Similarly, those subbasins on Ponds 3, 4, and 5 have been given D3-, D4-, and D5- prefixes, respectively, followed by sequence numbers. The subbasins in the north drainage areas have been designated with a N- prefix. None of the N drainage areas drain or discharge runoff across tailings. Likewise, subbasins DM-1 through DM-6A are routed to the south away from tailings. Non-tailings drainage area which is incorporated into the actual tailings drainage is very limited.

A non-rock mulch cover is planned for subbasin D4-1 on the No. 4 tailings. This non-rock mulch cover extends into adjoining portions of subbasins D4-2, D4-3, D4-5 and D5-1 (See Exhibits 5-1, 5-1A and 5-1B). A non-rock mulch cover is also proposed for subbasins D5-3 and portions of subbasins D5-1, D5-4, D5-2A, D5-2B, and D5-5. In the mill area, a portion of drainage areas D5-7, DM-7 and DM-9 will be reclaimed with the clay cap and a non-rock mulch

cover. The remainder of the tailings and byproduct disposal areas will be covered with rock mulch. With the exception of side slopes on control structures, the maximum slope on the reclaimed surface of the tailings area is 4H:1V. The 4H:1V slopes will be present only on the channel side slopes. The outslope on the tailings No. 5 dam and the upstream side of the solution pond No. 3 dam will be reduced to a steepest slope of 5H:1V and will be covered with appropriately sized rock mulch for erosion protection.

A drainage channel is included on the northern boundary of the tailings No. 5 dam and the north side of the solution pond No. 3. These channels are formed by excavating through the dam and are the outlets for Pond 5 and Pond 4, respectively. An excavated channel on the west side of the Industrial Pond will limit maximum permanent water level elevation in the pond to 7110 ft-msl. The permanent pond will serve as a source of recharge to the Surficial aquifer and as a surge and detention pond for runoff. The west side of Pond 4 will also contain a depression which will retain water for some time following a runoff event. This depression will be located over a non-tailings area and will also provide recharge to the Surficial aquifer.

5.1 Cut and Fill Areas

The majority of the rock mulch areas will not require a significant amount of cut or fill, except for the radon barrier cover placement. A large amount of fill is planned for the southern portion of subbasin D4-3 and the lower portions of subbasin D5-1 (see Exhibit 5-1). This fill will consist of the low-grade waste material located to the west of Solution Pond No. 3, windblown and other cleanup materials, residual ore, equipment and rubble, and excess tailings from surface contouring.

In areas where cut and fill activity is limited, the design contours will closely approximate the existing contours plus the cover thickness. These areas include the majority of the windblown cleanup area, and subbasins DM-1, DM-3, DM-4, DM-5, DM-6, and DM-7. A detailed discussion of volumes is included in Section 8.0.

5.2 Drainage Design

Objectives in the drainage design included: reduction of basin drainage area to decrease the potential for erosion, diversion of non-tailings runoff away from the tailings, maximization of contributing drainage areas for the Area 2/8 and Area 3 reservoirs, and utilization of runoff storage areas as surge ponds to reduce peak flows. Discharge is routed off of the tailings in several directions to achieve these objectives.

Runoff from the tailings, mill, and adjoining areas discharged at several points. The majority of the surface runoff from the tailings area discharges to the Area 2/8 reservoir through the channel section which passes through the former No. 3 Solution Pond and down the reclaimed mine slope. The center section of Pond 5 (subbasin D5-1) discharges through the channel to the north with eventual discharge to Spring Creek. Runoff from the No. 5 dam outslope (basin D5-4) also discharges to the Spring Creek drainage. Runoff from the southern portion of Pond 5 and portions of the mill area discharge to the east into the mine Area 3 reservoir. areas south of the mill discharge into the Area 3 reservoir. a very small portion of the covered mill area discharges to the southeast and this area is considered a non-tailings drainage area. The specifics of the channelized flow runoff analysis are included in Section 5.2.2.2 and Appendix E. A general flow schematic is given in Figure 5-4.

The general drainage pattern for discharge to the Area 2/8 reservoir is as follows:

Runoff from subbasins DM-2 through DM-6A is collected and routed through storage in the Industrial pond and a surge pond formed by a channel/berm. Runoff from subbasin DM-1 flows into the south drainage channel downstream of the surge ponds.

Runoff from subbasin D5-2A is routed through storage in subbasin D5-2A and into subbasin D5-2B.

Runoff from subbasins D4-1, D4-2, D4-3, D4-4, D4-5, D4-6, DM-7, DM-8, D5-2A, D5-2B, and D3-1 is collected and routed through storage in Pond 3. Subbasin D4-4 has a small depression which will retain water outside of the covered tailings area during smaller runoff events.

Runoff from the north subbasins N-5 through N-12 will be collected and routed through a surge pond designated as subbasin N-13. Subbasins N-5 and N-6 are routed through storage in shallow depressions within each respective basin. Subbasins N-5 through N-8 are also routed through a constructed surge pond in subbasin N-8.

The general drainage pattern for discharge to Spring Creek is as follows:

Runoff from subbasin D5-1 will be routed through storage in Pond 5.

Runoff from subbasin D5-5 will be combined with runoff from storage in Pond 5 and discharged to Spring Creek.

Runoff from subbasin D5-4 will be discharged to a channel at the toe of the No. 5 dam outslope for eventual discharge to Spring Creek through the old Mine Creek channel.

The general drainage pattern for discharge to the Area 3 reservoir is as follows:

Runoff from subbasin DM-9 will be routed through storage in subbasin DM-9.

Runoff from subbasins D5-3 and D5-7 and runoff from storage in subbasin DM-9 will be routed through storage in the subbasin D5-3 channel.

Runoff from subbasin D5-6 will be routed through storage and combined with runoff from storage in the subbasin D5-3 channel and runoff from drainage basin DM-10. Eventual discharge is to the Area 3 reservoir.

Runoff from the dump piles (North dump and South dump) will be routed through storage south of the mill and eventually discharged to the Area 3 reservoir.

5.2.1 Design Storm

The design storm to qualify for 1000 year protection, as required by the NRC Staff Technical Position, is the PMP (Probable Maximum Precipitation). The PMF (Probable Maximum Flood) is derived from a critical combination of PMP and storm distribution. The 1-hour, 1-sq. mi. storm was selected as a conservative precipitation event for the Shirley Basin mill and tailings area. The depth of precipitation for this event is 10.15 inches as presented by Hydrometeorological Report 55A (HMR-55A) (Hansen and others, 1988). An adjustment was made for altitude based on an average Shirley Basin mill area altitude of 7100 ft+msl. The adjustment factor for this altitude was 0.89, giving a PMP of 9.03 inches. No adjustment was made for basin area.

The storm distribution for overland flow computations was taken from HMR-55A (page 200). This storm distribution places the highest intensity interval at the beginning of the storm with

decreasing intensity as the storm continues. The tabulated values of percentage of precipitation occurring in a given interval are presented below.

Duration	(hr)	Fraction	of	1	hr	Precip.
1/4		0.68				
1/2	0.86					
3/4		0.94				
1		1.00				

The values above were fitted with a polynomial to determine cumulative precipitation for time periods less than one hour. This in turn gave average intensities for periods up to one hour. The polynomial equation is:

$$P = 9.03 * (0.076222 * t - 0.00265185 * t^2 + 4.34568E-05 * t^3 - 2.63374E-07 * t^4)$$

where: P is the precipitation in inches t is the time of concentration in minutes

The intercept for the polynomial fit was very small and was discarded.

Cumulative precipitation values were used in conjunction with the time of concentration of overland flow to determine required precipitation intensities. This storm distribution was used only for overland flow computations, and is very conservative for the short time of concentration characteristic of most overland flow paths. The minimum time of concentration for intensity calculations as recommended by NUREG/CR-4620 is 2.5 minutes, giving a maximum intensity of 37.85 in/hr. The cumulative precipitation curve is presented in Figure 5-1.

A significantly different precipitation distribution was used for channelized flow design. This storm distribution placed peak intensity near the middle of the storm, with a general bell shaped

intensity curve (Figure 5-2). This storm distribution gives very conservative runoff values when the time of concentration is longer, as is the case with small drainage basins on the tailings surface.

A 1000-year return period storm was also considered in the tailings drainage design. This design storm was considered for rock sizing on areas adjacent to the tailings where a failure of the drainage system would not have impacts on tailings stability. The precipitation for the 1000-year return period storm was determined using rainfall records from the Shirley Basin townsite. The data used were collected by the National Weather Service, a division of the U.S. Department of Commerce, National Oceanic and The approach taken was to Atmospheric Administration (NOAA). search hourly precipitation records published by NOAA for maximum yearly one-hour precipitation events from 1962 to 1984. Each value was arranged in descending order and a plotting position was calculated. The plotting position used is that recommended by Yevjevich (1972, p. 90):

$$P = m/(N+1)$$

where:

P = plotting position,

m = ordered sequence of values, and

N = sample size of ungrouped data.

The inverse of the plotting position is the recurrence interval. Figure 5-3 presents listings of m, plotting position, recurrence interval and maximum hourly precipitation value for the Shirley Basin Townsite. The one-hour, 1000-year rainfall depth from this analysis is 3.2 inches. The storm distribution used in the HEC-1 analysis was a hypothetical storm (PH designation) which produces a triangular precipitation distribution. Table 12 of

Miller et al. (1973) was used to determine the five-minute and 15-minute rainfall depths of 0.928 and 1.824 inches, respectively.

5.2.2 Runoff Calculations

5.2.2.1 Overland Flow

Runoff from overland flow on rock protected areas was calculated with the Rational Formula:

O = CiA

where:

C = runoff coefficient

i = rainfall intensity, in/hr

A = drainage area, ac

The C value was estimated as 0.8 and Kirpich's (1940) method was used to calculate the time of concentration for flow paths. Kirpich's equation is expressed as:

$$tc = (11.9 L^3 / H)^{.385}$$

where:

tc = time of concentration in hours

L = drainage length in miles

H = elevation difference (in feet)

The C value of 0.8 was selected as representative of a rock mulch layer underlain by a relatively coarse filter material. In reality, the rate of infiltration into the rock mulch and filter material should be so great that several inches of precipitation would be required to produce any runoff at all. The C value of 0.8 is representative of the rock mulch and filter in a nearly saturated condition prior to the PMP, a very conservative combination of events.

A series of flow paths was selected on the proposed reclaimed tailings and mill area surface. In general, these represented a

more extreme overland flow condition based on slope and slope length. The paths were also selected to achieve complete areal coverage for reclaimed surfaces. Each flow path was divided into sections of uniform slope for the runoff analysis. Flow paths were extended to the approximate point where overland flow would meet channel flow under an estimated channel flow depth of one foot. The Rational Formula was used to compute discharge for each segment. The time of concentration and area/unit width was summed while moving down gradient on each flow path, thus providing a cumulative discharge. For flow paths with very short time of concentration, this is essentially a summation of peak flows. As total time of concentration increased, the precipitation intensity decreased slightly, and thus the rate of discharge is slightly less than a summation of peak flows.

The overland flow paths for rock mulch and non-rock mulch areas are presented in Exhibits 5-1, 5-1A and 5-1B. characteristics and design rock sizes are presented in Table 5-2. The rock design methods are discussed in Sections 5.3.1 and 5.3.2. An additional analysis was conducted by determining the maximum unit discharge that specific rock sizes could withstand on the channel side slopes. This analysis revealed: the rock mulch $(D_{50}=0.150 \text{ ft})$ will withstand a unit discharge of 0.200 cfs/ft on a 4H:1V slope and a unit discharge of 0.630 cfs/ft on a 8H:1V slope, the small riprap ($D_{50}=0.400$ ft) will withstand a unit discharge of 0.880 cfs/ft on a 4H:1V slope, and the intermediate riprap ($D_{50}=0.600$ ft) will withstand a unit discharge of 1.62 cfs/ft on a 4H:1V slope. These guidelines were employed in sizing rock in the channels on the tailings surface. In some cases, the side slopes of channels on the tailings area are much less than 10H:1V. The maximum unit discharge flowing into a 4:1 channel was estimated to be 0.334 cfs/ft (overland flowpath O5-4A) and rock sizes for the

channels upstream of hydrologic cross-sections HC4-2 and HC5-9 was increased to the small riprap size as a measure of conservatism.

The Horton/NRC method (Horton, 1936) was used to design non-rock slope areas. Both non-rock and rock-protected slopes are presented in Exhibit 5-1. A C value of 0.9 and a concentrating factor (F) of 2.5 were used in the analysis. The method of determining time of concentration and precipitation intensity was discussed earlier. Manning's roughness coefficient was set at 0.025, representing a relatively smooth earthen surface (Chow, 1959). The Horton/NRC equation was rearranged to allow computation of the critical or allowable slope length for a given combination of slope, precipitation intensity, and allowable shear stress. This equation is expressed as:

 $L = (65 t^{(5/3)})/(P F n Ss^{(7/6)})$

where:

L = critical or allowable slope length in ft

 $t = allowable shear stress (lb/ft^2)$

P = design precipitation intensity

F = flow concentration factor

n = Manning's roughness coefficient

Ss = stable slope in ft/ft

The solution is iterative due to dependence of the design precipitation intensity on slope length. A spreadsheet program was developed to determine the allowable slope length for a series of flow paths on overland areas. These paths were selected to represent maximum slope and slope length, while providing sufficient coverage of the area. The actual slope length is then compared to the allowable slope length to assure non-exceedance.

The proposed final cover material (radon barrier) is a clay overlain by a sand layer, filter, and rock mulch or topsoil. Several samples of the clay cover were analyzed for physical properties, including gradation, Atterberg limits, dispersion, permeability, etc. Four topsoil samples from two stockpiles were

also analyzed for gradation and Atterberg limits. Based on the results of these analyses, the topsoil material ranges from a silty sand to a sandy lean clay. Using the relationships from Temple (1987), the computed allowable topsoil shear stress ranged from 0.074 lb/ft^2 to 0.097 lb/ft^2 . The parameters used and calculation for each sample are:

TS-1, Silty Sand (SM), PI=31, Estimated void ratio = 0.48 $C_e=1.42-0.61$ e = 1.127, $\tau_{ab}=0.058$, $\tau=0.074$ TS-2, Silty Sand (SM), PI=21, Estimated void ratio = 0.48 $C_e=1.42-0.61$ e = 1.127, $\tau_{ab}=0.058$, $\tau=0.074$ TS-3, Clayey Sand (SC), PI=25, Estimated void ratio = 0.48 $C_e=1.42-0.61$ e = 1.127, $\tau_{ab}=0.076$, $\tau=0.097$ TS-4, Sandy Lean Clay (CL), PI=19, Estimated void ratio = 0.56 $C_e=1.42-0.61$ e = 1.08, $\tau_{ab}=0.0706$, $\tau=0.082$

The average allowable shear stress for the four topsoil samples is $0.082 \, \mathrm{lb/ft^2}$ and this value was selected for use in the Horton/NRC equation.

Actual and allowable slope lengths for non-rock mulch areas are presented in Table 5-3. For the purposes of computation, some contributing rock mulch areas were included in the flow paths. In particular, the downstream side of rock mulch berms were included in some paths and do not meet the allowable slope-length criteria for non-rock mulch areas. Rock mulch will be applied to the top and immediate downstream areas of these berms to a point at least 10 feet beyond the break in slope, thereby reducing slope length and runoff rate to acceptable levels. Some of the very mildly sloping areas on the tailings have differential elevations which are not adequately shown by the contour interval, and these elevation differences are presented in Table 5-3 for appropriate paths.

5.2.2.2 Channelized Flow

Peak flows and runoff hydrographs resulting from the design storm (see Section 5.2.1)(1-hr, $1-mi^2$ PMP = 9.03 in.) were determined for the mill, tailings and solution pond area subbasins using the U.S. Soil Conservation Service (SCS) Dimensionless Unit Hydrograph option of the Army Corps of Engineers' (ACOE, 1985) HEC-1 computer model. The cumulative 1-hr PMP rainfall distribution (see Section 5.2.1 and Figure 5-2) was input to the model in one-minute increments.

The SCS Dimensionless Unit Hydrograph option of HEC-1 requires input of basin time of lag (tL) and SCS runoff curve number (CN). The time of lag for each subbasin was estimated using the following equation:

tL = 0.6tc

where:

tL = time of lag, in hours,

tc = time of concentration, in hours.

Time of concentration was determined using the Kirpich (1940) formula described in Section 5.2.2.1.

The runoff curve number was developed by SCS to define infiltration losses during a precipitation event. CN is dependent on the hydrologic soil group as defined by SCS, vegetation, land use and antecedent moisture condition (AMC). The majority of the non-tailings drainage areas are mine overburden piles or surface disturbance areas. The exposed materials range from well drained sands to poorly drained clays. For these areas, hydrologic soil group C was judged to be a conservative soil type. Hydrologic soil group C was also considered to be an appropriate soil classification for tailings areas where topsoil will be placed. Because the rock mulch and underlying sandy filter blanket will be

well drained, hydrologic soil group A is considered to be appropriate in the areas where it will be placed.

SCS presents tables of representative curve numbers in TR-55 (SCS, 1986) based on average runoff condition (AMC II), cover type, hydrologic condition and hydrologic soil group. SCS' National Engineering Handbook, Section 4, Hydrology (SCS, 1972) presents ranges of curve numbers for varying antecedent moisture conditions. For estimating peak flows from the PMP, it is deemed appropriate to use AMC III, indicating wet conditions prior to occurrence of the PMP event.

In areas where native material is prevalent, or where the ground surface will be reclaimed with native materials, a CN of 91 was selected. Where rock mulch will be placed, a CN of 85 was used. In drainage basins where more than one cover material type will be placed, a weighted average based on relative areas was used. The CN used for each subbasin is given on Table 5-1.

The HEC-1 model allows determination of runoff hydrographs from individual subbasins, combining hydrographs from the separate subbasins, and routing individual or combined hydrographs to downstream points. Channel routing was considered, but the incorporation of surge pond storage and flow control structures tended to make lag due to channel routing inconsequential. However, the severity of the PMF event does result in very large flow depths for overland flow. These flow depths represent runoff storage which is not incorporated in the HEC-1 modeling. This results in extremely conservative (large) peak flowrates for channels.

Table 5-1 presents basin characteristics for each mill, tailings disposal pond and adjacent area subbasin. Appendix E presents HEC-1 input files and schematic diagrams of the stream network systems as they were modeled. Hydrographs for the 1-hr,

1-mi² PMP runoff event for the mill, tailings disposal and solution pond area subbasins are also presented in Appendix E.

In storage or surge pond areas, level-pool flood routing was used in the HEC-1 modeling. The storage volume was determined by measuring areas and depths within the ponded areas. These areas were taken directly from contours on Exhibit 5-1. The stage/discharge relationship for the flow control structures was determined with Manning's equation, using a constant n value of 0.05. A comparison was made with stage/discharge as predicted by HEC-2, with no significant differences (see Section 5.2.3.2 for more details).

5.2.2.3 Adjacent Area Drainage

The north area drainage (subbasins N-5 through N-14) and south area drainage (subbasins DM-1 through DM-6A) are adjacent to the tailings, but do not discharge to the tailings area drainage system. These channels are designed to accommodate a less severe storm than the PMF without impacting tailings drainage.

Potential impacts to the tailings are eliminated in one of two ways. In the case of the south drainage, the low slope drainage channel to the south has sufficient capacity to accommodate PMF flows. A berm between this drainage and the tailings area precludes flow from the southern drainage to the tailings area. Migration of any gullies will be along the channels or directly away from the channels and consequently, away from the covered tailings.

In the case of the North drainages, there is sufficient channel capacity to accommodate a 1000-year storm without overtopping. A more severe event will overtop either the surge pond in subbasin N-8, or the channel at the base of subbasin N-11. The lowest point on the berm that forms the surge pond in subbasin N-8 is at the north end of the basin, Failure at this point will

discharge water down the reclaimed overburden pile north of the tailings. Fortunately, this overtopping point is at the upstream end of the surge pond, and a short-term overtopping would not short circuit the north drainage. Overtopping of the channel at the base of subbasin N-11 will eventually result in gully migration away from the tailings, However, the allowed overtopping in subbasin N-8 limits the flow through subbasin N-11 and should prevent overtopping in the lower channel under almost all circumstances.

Subbasin N-14 drains to the north through a constructed upgradient of the reclaimed access road. This channel is protected by a series of rock checks. These rock checks will be constructed with the intermediate riprap as shown in Figure 10-7. They will impound very small volumes of water upstream of the check and will serve as effective grade control for the minor channel. The reclaimed access road is deliberately sloped into the outslope of the reclaimed overburden pile both for safety reasons and to create an overbank channel. This channel will have sufficient capacity to carry the PMF. A small V-channel is used to capture runoff from subbasin N-12. The drainage area for this channel is very small. In the unlikely event of channel failure, migration of gullies would be directly away from the tailings.

5.2.3 Channel Conveyance Characteristics

Locations of channel cross-section sites and channel control structures in the mill and tailings disposal pond areas are shown on Exhibit 10-2. The channel cross-section sites have been designated with an "HC" prefix, which indicates hydrologic cross section. The "HC" prefix is followed by an "N", "4", "5", "S" or a "T", which indicates whether the site is in the north drainage, Pond 4 area, Pond 5 area, the south drainage which flows from the industrial pond area to the 2/8 reservoir or the confluence of all drainage to the 2/8 reservoir ("T" represents total), respectively. Finally, the cross-section designation is completed with a sequence

number, which generally increases while moving downstream on the drainage. Stream profiles and their locations are presented in Exhibit 10-5.

5.2.3.1 Manning's Channel Conveyance

Manning's equation was used to estimate channel conveyance characteristics at each hydrologic cross section. This equation is as follows:

$$Q = (1.49/n) R^{2/3} S^{1/2} A$$

where:

 $Q = discharge, in ft^3/sec,$

n = Manning's roughness coefficient,

R = hydraulic radius, in feet,

S = channel slope, in ft/ft, and

A = cross-section area, in ft².

Hydraulic radius is defined as the area of flow divided by the wetted perimeter. Manning's roughness coefficient was calculated using the Abt (1987) method for channels with a slope less than 10 percent, and it was calculated using the U.S. Army Corps of Engineers (ACOE) method for channels with slopes greater than 10 percent. The Abt method of determining Manning's n is as follows:

$$n = 0.0456 (D_{50} \times S)^{-0.159}$$

where:

13

n = Manning's roughness coefficient

 D_{50} = the screen diameter through which 50% of the rock

passes, in inches

S = channel slope in ft/ft

The ACOE method of determining Manning's n is as follows:

$$n = R^{.1667}/(21.95(\log_{10} (R/D_{50})) + 23.85)$$

where:

n = Manning's roughness coefficient

 D_{50} = the screen diameter through which 50% of the rock

passes, in feet

R = the hydraulic radius, in feet

The Stephenson (1979) method was used to determine rock sizes for slopes greater than or equal to 10% and the Safety Factors method was used to determine rock sizes for slopes less than 10%. Section 5.3 should be reviewed for a detailed discussion of riprap characteristics.

For each of the outlet locations, there is at least one area potential surge pond area upstream of a channel Channel-control structures or low-slope channels will be constructed at these sites in order to dampen peak flows. No permanent ponding over covered tailings will occur. there will be the potential for permanent ponding in the existing Industrial pond and the depression in basin D4-4. control site HCT-1, retention time following the PMP runoff event is estimated to exceed 30 hours. However, the time that water will be impounded over buried tailings upstream of control site HCT-1 is estimated to be less than 20 hours. This is the longest time period for which ponding is expected over covered tailings. The hydrographs for subbasins and combinations of subbasins are presented in Appendix E.

Channel geometry and conveyance characteristics for all of the cross-section locations are presented in the Table 5-4 and the Manning's section of Table 5-5. The column labeled 'energy gradient' in both tables was taken from HEC-2 modeling of non-uniform flow. The rock sizing in Table 5-4 was done by the methods discussed in following sections using the energy gradient rather than the channel slope. At cross-section location HC4-13, the design rock is slightly larger than the target rock D_{50} . This cross section is located with the surge pond at maximum PMF stage, and a

very slight undersizing is not significant. At two cross-section locations (HC4-9 and HC5-11) the required rock D_{50} is larger than the target rock D_{50} . Rock at both of these locations was sized by the Safety Factors Method using the energy gradient rather than the channel bottom slope. In both cases the channel bottom slope is much smaller than the energy gradient and the target rock size would be more than adequate if the channel bottom slope were used rock sizing. The use of the energy gradient in the Safety Factors method is highly questionable, and it typically gives very conservative rock sizes in gradually to rapidly varied flow regimes. For this reason, the Stephenson method was used for rock sizing presented in the Manning's section of Table 5-5. porosities were used in these calculations. A rock porosity of 32.5% was considered appropriate for channel riprap, while a porosity of 45% provides a more conservative rock sizing.

The primary tailings area drainage features are the V-notch control sections located at cross-sections HC4-5, HC4-7, HCS-6, HCT-1, HC5-3, HC5-10, HC5-11, and HC5-13. With the exception of sections HC4-7 and HC5-13, the control structures are 50+ feet long V-notch sections with 2:1 side slopes and a base slope of 0.002 ft/ft. Sections HC4-7 and HC5-13 are relatively minor features and are at least 30 feet long. Channel sections HC5-1 and HC5-2 have side slopes that are much milder than 20H:1V and are essentially a swale type design. The channel sections downstream of crosssections HC4-11 and HC4-12 are at very low slopes and the runoff will overtop the channel and be stored in the overbank area. remainder of the channels will be simple and trapezoidal in shape with 4H:1V side slopes and bottom widths ranging from 20 to 50 feet. Figure 10-1 presents a channel cross section depicting the typical trapezoidal shape, 4H:1V side slopes and variable bottom width. Low slope channels on the tailings surface which serve as overland flow collectors have a less defined channel shape and

typically have side slopes that are much less than 4H:1V. Riprap is also shown on this figure in a conceptual manner, with details of rock type and required size being given for each channel reach on Table 5-4. Figure 10-2 is a cross section sketch of HCT-1, the V-notch control with 2:1 side slopes.

Exhibits 10-2 and 10-5 through 10-8 show channel reaches where riprap will be required in the mill and tailings disposal pond areas. The differing patterns on these exhibits depict different rock types which will be used. Sections 3.3 and 5.3 present more detailed information on riprap properties and characteristics.

The control section HC4-7 is intended to allow discharge to the small depression in subbasin D4-4 during low flow events and is only two feet in height. During more extreme events, the entire control structure will be submerged by storage upstream of control section HCT-1.

There are three locations where major channels converge at angles between 45° and 135°. The first is immediately upstream of cross section HC5-11 where the channel is discharging into a surge pond upstream of a V-notch control. The second is the confluence of channels D, I, and M in the surge pond in the former No. 4 Pond. The potential for erosion by flowing water at these confluences is eliminated by ponding. The third location is the confluence of channels A and D. Channel A is a very minor channel, and it is unlikely that the short duration peak flow could create a significant wave affect.

5.2.3.2 HEC-2 Channel Conveyance

In order to simulate PMF water-surface profiles in channels in the tailings reclamation area, the U.S. Army Corps of Engineers model, HEC-2, was utilized (ACOE, 1982). The HEC-2 model is intended for calculating water-surface profiles for steady and

gradually varied flow in natural or man-made channels. Both subcritical and supercritical flow profiles can be calculated.

The following channels, the locations of which are shown on Exhibit 10-5, were simulated with HEC-2: A, D, H, I, M, N, O, P and Exhibit 10-5 presents longitudinal profiles for these channels. Exhibits 10-6 through 10-8 present profile location details at a larger scale. Cross sections included in the simulated channels include: HCT-1, HCT-2, HCT-3, HC4-1, HC4-2, HC4-3, HC4-5, HC4-6, HC4-9, HC4-10, HC4-11, HC4-12, HC4-13, HC5-1, HC5-2, HC5-3, HC5-4, HC5-5, HC5-6, HC5-7, HC5-8, HC5-9, HC5-10, HC5-11, HC5-12 and HC5-14. Channel C and other off-tailings channels were not simulated with HEC-2. During the PMF, some erosion in these channels would be possible. However, because they are located off of the tailings, any potential erosion would not threaten exposure of the tailings. Other channels, including B and G, have a relatively uniform slope, and Manning's equation calculations and HEC-2 results are essentially the same. Therefore, simulation was not considered necessary for these channels.

In some of the simulated channels, some cross sections shown on Exhibit 10-5 were not included in the HEC-2 simulations. These include, for example, HC4-7 and HC4-8. The channel reaches in and near these locations are very flat, they are generally upstream from channel control structures, and they will therefore be subject to backwater conditions at low flows, as well as during the PMF. These cross sections are shown on Tables 5-4 and Table 5-5 as being ponded.

Cross-section HCT-1, located on Channel H, is a V-notch control section with ponding upstream of the section and is used to verify the stage-discharge rating table used in the HEC-1 simulation of the tailings area. The following table shows the comparison between the HEC-2 generated stage-discharge relationship at HCT-1 and that generated using Manning's equation.

DISCHARGE	HEC-2 STAGE $(n = 0.035)$	HEC-2 STAGE (n=0.05)	MANNING'S EQN STAGE (n =0.05)
(cfs)	(ft)	(ft)	(ft)
100	3.9	4.2	4.2
200	5.0	5.3	5.4
300	5.8	6.2	6.3
500	7.1	7.4	7.6
800	8.4	8.8	9.1
1000	9.2	9.5	9.9
1500	10.7	11.1	11.5

Based on the above comparison, it is concluded that use of Manning's equation is adequate in determining stage-discharge relationships at channel controls.

The HEC-2 channel analysis was conducted with a constant Manning's n of 0.035. This was done for the sake of simplicity and to avoid iteration between rock sizing, adjustment of Manning's n, and re-analysis with HEC-2. The rock sizing for the HEC-2 analysis was done with the Stephenson method and a constant Manning's n. This was also done to simplify the HEC-2 design procedure. Varying both the rock sizing method and the Manning's n requires numerous simulations with successive adjustments in channel geometry and slope conditions. In general, the HEC-2 simulations indicated that the transition areas downstream of the control structures were critical areas due to steeper energy gradients in the gradually varied flow profile.

The results of the HEC-2 analyses are presented in Table 5-5, which is divided into two parts. The first part gives PMF channel conveyance characteristics at channel cross-section locations based on Manning's equation. The second part gives channel conveyance characteristics based on HEC-2 simulations. In the HEC-2 analysis, many intermediate cross sections were simulated, in addition to the "HC" cross sections. These intermediate cross sections are designated with a channel descriptor letter prefix, and the channel

station. For example, A-200 is located in Channel A at station 2+00. The channel stations are shown on Exhibits 10-5, 10-6, and 10-7, and are given in feet. Therefore, A-200 is located at station 2+00 and is 200 ft upstream from the downstream end of Channel A.

The channel geometry, riprap sizing, and channel transition design are shown on Exhibit 10-2. The HEC-2 simulations revealed the need for more gradual transitions and larger riprap immediately downstream of the control sections. Changes in channel geometry in the transition areas are gradual. For example, the channel geometry gradually changes from a V-notch with 2H:1V side slopes at station H-1470 to a trapezoidal channel with 40 ft. base width and 4H:1V side slopes at station H-1280 (Table 5-5). The energy gradient was used in the sizing of the riprap for these areas. In all cases, the energy gradient was significantly greater than the channel bottom slopes immediately downstream of the control. The use of energy gradient to size riprap in these areas should provide a very conservative design.

In general, channel conveyance characteristics determined using HEC-2 and Manning's equation compare very well in relatively uniform stream reaches, because the energy gradient closely approximates the channel bottom slope. In areas where the stream profile is changing rapidly because of changing bottom slope or channel constrictions due to control structures, HEC-2 is better able to account for those changes.

HEC-2 input files used to simulate Pathfinder's tailings reclamation channels and the resulting output files were furnished to the NRC on a disk due to the volume of tabulated data. The input file listings are also included in Appendix F.

5.3 Rock Design and Placement

Rock erosion protection is used in channel sections and on the tailings surface as a rock mulch. The source of rock is a granite outcrop. Rock properties are discussed in Section 3.3.

5.3.1 Rock Mulch Design

Overland flow paths discussed in a previous section were used in determining discharge for rock mulch design and sizing. segmentation of the flow paths into uniform slope segments will allow the use of more than one mulch size on each flow path (see Table 5-2). However, for construction purposes, the use of one mulch in a specific area was the preferred solution. Where this was the case, the mulch was sized or designed for the extreme Thus, the rock mulch is conservatively designed for much of the tailings surface. The Safety Factors method was used for rock design for slopes of less than 10%, and the Stephenson (1979) method was used for slopes of 10% and greater. (1987) method was used to determine Manning's n for all rock mulch areas. The parameters that were used in the rock mulch design include:

> Safety Factor (SF method) = 1.05 Rock Porosity = 0.45 Specific Weight of Rock = 165 lb/ft³ Rock Shape = Angular

According to NUREG/CR-4620 (Nelson et al. 1986), a safety factor slightly greater than 1 is sufficient for PMF applications. Rock porosity is dependent on percentage of fines and placement procedures. A study by Abt et al. (1987) used small D_{50} riprap with porosities ranging from 0.44 to 0.46. The riprap use by Abt et al. (1987) was fairly uniform and the granite is expected to yield similar surface porosities for rock mulch. Specific gravities

(SSD) for the granite ranged from 2.64 to 2.65, giving a specific weight of 165 lb/ft^3 .

There are very few areas where overland flow from a non-tailings area flows onto a tailings protection area. Flow lengths for these areas are limited to 450 feet, and the flow from these areas will not compromise the final tailings cover. In the overland flow analysis, these non-tailings segments were included in determination of peak flow.

The granite will be mined from a quarry area located approximately 15 miles northeast of the mill. The final gradation of the mined material is unknown and is expected to vary with depth and required processing techniques. The majority of the rock is expected to require blasting and crushing to produce the desired product. Gradation of the processed granite will be monitored during construction, and processing or design adjustments will be made if necessary.

The eventual rock mulch configuration will be dictated by crushing and processing limitations and construction constraints. The minimum placement thickness for a layer of rock mulch or filter material is 3.6 inches. The areas where rock mulch will be used are shown on Exhibit 5-1 and Exhibit 10-2. The rock mulch will be placed to a thickness of 2.0 times the D_{50} , or the D_{100} , whichever is greater. The D_{100} thickness requirement may be waived if the rock can be placed to achieve a smooth uniform surface. At the base of the reclaimed No. 5 dam outslope, the rock mulch will extend through a drainage channel at the base of the slope or a rock toe will be installed. The design of the rock toe is detailed in subsequent sections.

The rock mulch size presented on Exhibit 10-2 in Volume II is a minimum D_{50} . If placement or rock crushing constraints require a larger rock, larger rock may be substituted for smaller rock with appropriate adjustments in thickness and filter requirements.

The filter layer may consist of a dual filter design or a single filter design. Where a dual filter is employed, the lower filter layer will consist of a sandy material from the dump piles. Gradation requirements for the filter are discussed in a later section. The upper filter will consist of the crushed granite which has been subjected to additional crushing and processing or screenings from the granite crushing operation. Where a single filter design is employed, the filter will consist of a crushed and processed granite.

5.3.2 Rock Riprap Design

The riprap design was done with the same rock sizing methods used for rock mulch design with the exception of the HEC-2 and non-uniform analyses, where the Stephenson method was used exclusively. The Abt (1987) method was used to determine Manning's n for slopes of less than 10%, and the Army Corp. of Engineers (ACOE) method was used to determine Manning's n for slopes greater than or equal to 10%.

For all areas discharging from or through the tailings drainage, the peak PMF discharge was used in sizing channel riprap. Tables 5-4 and 5-5 list channel conveyance characteristics for the PMF. Rock riprap designs for off-tailings drainage areas are not included in the tables because these areas are not subject to PMF design criteria. Four design rock sizes (D_{50} 's of 0.150 ft., 0.400 ft., 0.600 ft, and 1.200 ft.) are presented in Table 5-4 for channel cross-section locations. The rock sizes are designated rock mulch, small riprap, intermediate riprap, and large riprap.

End protection structures are included just downstream of cross-sections HCT-1, HC5-5 and HC5-14 and at the Mine Creek outlet of subbasin 5-4. Channels and riprap upstream of these cross-sections are designed according to peak PMF runoff.

5.3.3 Non-uniform Flow Channel Rock Sizing

Determination of appropriate rock sizes in non-uniform flow is problematic in that all rock sizing methods were developed for uniform flow conditions or indirectly incorporate the assumption of uniform flow. The Stephenson method, which was used for rock sizing in channels where HEC-2 water surface profiles were developed, is expressed as:

$$D_{50} = \left[\frac{q (\tan \theta)^{7/6} n_p^{1/6}}{C g^{1/2} [(1 - n_p) (G_s - 1) \cos \theta (\tan \phi - \tan \theta)]^{5/3}}\right]^{2/3}$$

where:

 D_{50} = required rock diameter in feet, (50%

of the rock must be larger than this),

q = maximum flow rate per unit width,

 θ = angle of channel bottom from horizontal,

 ρ = angle of friction for the rock,

 $n_p = rockfill porosity,$

 G_s = specific gravity of the rock,

g = the acceleration of gravity, and

C = empirical factor which varies from 0.22
 for gravel to 0.27 for crushed granite.

The assumption of uniform flow is implicit in the use of γ , the channel bottom angle. Since $\sin(\gamma)$ and $\tan(\gamma)$ are approximately equal to channel bottom slope expressed in rise/run, the channel bottom slope is often substituted for $\sin(\gamma)$ and $\tan(\gamma)$.

Under non-uniform flow conditions, intuition suggests that shear stress can increase, particularly when the energy gradient is greater than the bottom slope. To compensate for this, it has been suggested that the larger of the channel bottom slope or the energy gradient be used for rock sizing. However, this substitution can be inappropriate when the actual increase in shear stress is calculated.

The equation describing shear stress on a channel base was developed from the momentum equation for open channel flow. Barfield et al. (1981) presents a functional form of this equation on a per unit width basis as:

$$\frac{Yy_1^2}{2} - \frac{Yy_2^2}{2} + W\sin\theta - R_f = \rho q(V_2 - V_1)$$

where:

 γ = unit weight of water,

 y_i = flow depth at point i (i = 1 is upstream station

and i = 2 is downstream station),

W = weight of water in the control volume between

station 1 and 2,

 θ = angle of the channel bottom from horizontal,

 R_f = resistance force exerted on the flow by

the channel base,

 ρ = the density of water, and

 V_i = velocity at station i.

For the entire channel cross-section, resisting force (R_f) can be approximated as the product of shear stress (τ) , wetted perimeter (P) and control volume channel length (L). This assumes that shear stress at the free water surface is negligible, which adds some degree of conservatism. Weight of the water in the control volume (W) is the product of the cross-sectional area of flow (A), the length of the control volume (L), and the specific weight of water (γ) . These relationships are expressed below, along with a conversion from density to specific weight.

$$R_f = \tau P L$$

$$W = A L \gamma$$

$$\rho = \frac{\gamma}{g}$$

When these relationships are substituted into the momentum equation with a conversion from a unit width basis to the cross-sectional flow area (multiplication by average flow width, b_a) and expressed in terms of shear stress, the result is:

$$\tau = \frac{1}{PL} \left[\frac{\gamma}{2} (y_1^2 - y_2^2) + A L \gamma \sin \theta - \frac{\gamma q}{g} (V_2 - V_1) \right]$$

Uniform flow conditions result when $y_1 = y_2$ and $V_1 = V_2$. Incorporation of these expressions and the substitution of S for $\sin\theta$ and hydraulic radius (R) for A/P results in:

$$\tau = \frac{\gamma A L \sin \theta}{P L} = \gamma \frac{A}{P} \sin \theta = \gamma R S$$

For wide trapezoidal channels with shallow flow depths, the flow depth (y) is often substituted for the hydraulic radius (R). This is not appropriate for channels where the ratio of channel base width (b) to flow depth (y) is less than 10. Use of the flow depth rather than the hydraulic radius grossly overestimates shear stress for V-notch channels and channels with a b/y ratio less than 5.

The Stephenson rock sizing method presented earlier does not use shear stress directly. Rather, the method uses the channel bottom slope and a unit discharge (q) which indirectly represent the shear stress. For non-uniform flow conditions, the most appropriate means of adjusting rock size for increased shear stress is to determine an equivalent slope for the non-uniform flow conditions. Although the use of energy gradient is appealing because of its simplicity, the following example indicates that use of energy gradient is not appropriate.

An example channel section from channel H was chosen to illustrate the adjustment of slope for non-uniform flow conditions. The increased shear stress was determined at station 1357, a transition area where the channel base width is expanding. The bottom slope of the channel is approximately 0.002 ft/ft, and the energy gradient from the HEC-2 model was 0.034 ft/ft. stress calculated by the uniform flow shear stress equation was 0.212 lb/ft² using the bottom slope and 3.61 lb/ft² for the energy gradient. The shear stress calculated by the non-uniform flow shear stress equation was 3.21 lb/ft2. This illustrates that there is an increase in shear stress when the energy gradient is much greater than the bottom slope, but that using the energy gradient in the definition of shear stress for uniform flow typically overpredicts that increase in shear stress. The determination of the equivalent slope for the increased shear stress is accomplished by rearranging the uniform flow shear stress equation as:

$$S = \frac{\tau}{\gamma R}$$

Using the increased shear stress of $3.21 \, \mathrm{lb/ft^2}$, the equivalent slope for station 1357 is $0.0302 \, \mathrm{ft/ft}$. This slope can be used in rock sizing methods to compensate for increased shear in non-uniform flow.

For V-notch channels or transition areas where the depth to base width ratio for the channel is fairly large, the shear stress distribution varies dramatically across the channel. To compensate for this, both an average unit flowrate and a maximum unit flow rate will be used in the Stephenson rock sizing method. Maximum unit flow rate will be calculated as the product of average velocity and maximum flow depth. For V-notch channels, maximum unit discharge can be more than three times the average unit discharge.

There is an increase in shear stress and required rock size in channel bends. This increased shear stress can be estimated using a method presented by USACOE, (1970). This method is based on Plate 34 of USACOE (1970), which is a figure relating the ratio of increased shear in bends to the ratio of channel bend radius divided by water surface width. The equation for this ratio of shear stress for smooth channels is given as:

$$\frac{\tau_b}{\tau} = 2.65 \left(\frac{r}{w}\right)^{0.5}$$

where:

 τ_b = maximum boundary shear in bend,

τ_o = average boundary shear,

r = center-line radius of channel bend, and

w = upstream water surface width of bend.

Unfortunately, this equation does not produce results that correspond with the figure in Plate 34 of USACOE (1970) and the correct form of the equation should be:

$$\frac{\tau_b}{\tau} = 2.65 \left(\frac{w}{r}\right)^{0.5}$$

For rough channels, the plotted data indicates that the constant 3.1 should be substituted for the constant 2.65 in the preceding equation. However, there are only two data points for a very small r/w ratio (less than 1.6) to support the rough channel constant, and these values were determined from a two foot wide flume. Very little confidence can be placed in the increased constant (3.1) and results derived with this form of the equation should only be used in a qualitative manner. Using this equation with the alternate constant, the minimum w/r ratio is 0.104. Below this value there is no increase in shear stress due to the channel bend.

All of the shear stress and rock sizing adjustments discussed above are included in the channel riprap design (Table 5-6). The Stephenson method was used to determine required rock size for each section on channels analyzed with the HEC-2 program. Manning's n was set at 0.035 to avoid iteration between flow characteristics and rock sizing. The parameters used in the rock sizing were a granite specific weight of 165 lb/ft^3 and rock layer porosity of 32.5%. The specific weight values were from measured properties and the porosity estimate was taken from Stephenson (1979), who gives porosity values ranging from 15% to 40%. Table 5-5 also includes the target rock D_{50} to show compliance with rock size requirements at each station in each channel.

5.3.4 Hydraulic Jump Analysis

The potential effect of hydraulic jumps in the channel was evaluated. Potential locations for hydraulic jumps were determined by noting all slope transitions from mild to steep slopes while moving upstream in the channels. These locations include: Channel D at Stations 0 to 200, Channel H at Stations 0 to 300, Channel H at Stations 1700 to 1900, Channel M at Stations 100 to 350, Channel Q at Stations 130 to 250 and Channel Q at Stations 1350 to 1500. The potential for a hydraulic jump in Channel H downstream of the control structure between Stations 1050 and 1300 was also evaluated. It was found that formation of a jump at this location is unlikely (see Table 5-7), and that the rock at this section is already grossly oversized. With the exception of Channel H at Stations 0 to 300 and Channel Q at Stations 130 to 250, the potential hydraulic jump locations occur in a ponded or backwater area. The depth of ponding is such that a jump will be completely submerged.

Table 5-7 presents the results of the hydraulic jump analysis for the sections listed above. The flow characteristics for the

channel section were determined assuming uniform flow at three different flowrates. These flowrates were: peak PMF flow, 1/2 peak PMF flow and 1/10 peak PMF flow. This was done to identify potential migration of the jump under various flow conditions and to determine threshold jump formation conditions. The downstream backwater condition for the very flat plain discharge area for channel H was not considered.

The location of the hydraulic jump is indicated in Table 5-7 by the bold, underlined values in the Froude number column. These values bracket the transition through critical flow (Froude number equal to 1). The station values corresponding to these noted Froude numbers therefore bracket the location of the jump. In some cases, Froude numbers approaching one were designated with underlining to indicate that a jump could occur at that location with only a slight change in flow conditions.

For each jump location, a turbulent flow rock size was determined by multiplying average unit flowrate by 1.5 and sizing rock for that unit flowrate using the Stephenson method. The channel bottom slope was used in the rock sizing, along with rock characteristics discussed in earlier sections. The target rock size and design rock size (for maximum unit flowrate) were taken from the analysis summarized in Table 5-5 and included in the columns 10 and 11 of Table 5-7.

The last column in Table 5-7 lists the ratio of target rock size to design rock size. The turbulent design rock D_{50} was used for jump locations. This ratio is essentially an adequacy ratio of the rock for the more severe turbulent condition. The ratios for the Channel H and Channel Q jumps indicate that the rock is more than adequate to withstand additional shear stress occurring near the jump.

5.3.5 Standing Wave and Confluence Analysis

The potential for development of wave pileup or standing waves at channel confluences was evaluated by examining all major confluences for characteristics that could result in occurrence of significant wave action. The PMF hydrographs presented in Appendix E were examined to determine whether the duration of severe hydraulic conditions for key sites warranted quantitative analysis.

As an example PMF duration condition, the hydrograph for channel cross section HC4-9 was examined (see Figure E-1). confluence of Channels A and D is located just downstream of this cross section. The peak PMF flow of approximately 180 cfs occurs approximately 45 minutes after the start of the storm. ten minutes before and after the peak flow, the approximate flows are 48 and 88 cfs, respectively. This gives a indication of the extreme brevity of the peak flows. Preliminary calculations indicate that the 1000-year return period storm will produce peak flows that are 10% to 25% of the PMF peak flows. Given the very low probability of occurrence for the PMF and the extremely short duration of peak PMF flows, a quantitative analysis for this condition does not seem worthwhile. Rather, a qualitative assessment of the vulnerability of critical confluences should be sufficient.

The locations of major confluences evaluated were as follows: confluence of Channels A and D, confluence of Channels P and O, confluence of Channels D and I, and confluence of Channels M and I. For the confluences listed, only the confluence of Channels A and D occurs under non-ponded conditions. Channel A is considered a minor channel and the flow depth is approximately 1 foot at the confluence. It is unlikely that wave action resulting from this limited flow depth could be significant.

5.3.6 Scour Analysis

The potential for scour downstream of the rock aprons or end protection structures was evaluated with several methods. End protection structures are used on all applicable channel sections and a rock toe is used where there is a transition from rock mulch to non-rock areas. The scour potential was evaluated at these areas by up to six methods. Unfortunately, most scour evaluation techniques were developed for continuous flows under hydraulic conditions that are dramatically different than those that occur in the tailings basin. Only one method (United States Department of Transportation or USDOT, (FHA, 1983)) appears to be of some use in evaluating scour for these conditions.

The use of surge ponds and control sections in the drainage design results in only four locations where tailings area drainage is released from a riprap channel to a non-protected sloping channel. These are the downstream ends of channels H, D, Q and the end drainage from subbasin 5-4. This downstream grade control will produce a tailwater condition which limits scour depth.

An attempt was made to evaluate scour potential for channels using three traditional methods that were intended for continuous flows with a large flow depth, such as a dam spillway. These methods are the Schoklitsch, Eggenberger, and Jager formulas (Barfield et al., 1983) which are presented below in the same order as listed.

$$d_{a} = S + h_{d} = 4.75 \frac{H^{0.2} q^{0.5}}{D_{90}^{0.32}}$$

$$d_{a} = S + h_{d} = C \frac{H^{0.5} q^{0.6}}{D_{90}^{0.4}}$$

$$d_{a} = S + h_{d} = 6 H^{0.25} q^{0.5} \left[\frac{h_{d}}{D_{90}}\right]^{1/3}$$

where:

d_s = distance between downstream water level and bottom of scour hole (meter),

 h_d = downstream water depth (meter),

H = vertical distance between the upstream and downstream energy grade line (meter),

 $q = unit discharge (meters^2/sec),$

 D_{90} = the particle size for which 90 percent of the

material is finer (mm), and

C = constant which is 22.8 for existing conditions.

Unfortunately, the head difference, H, was very sensitive to hydraulic conditions for the PMF condition and it was not possible to get the methods to yield results without highly artificial manipulation of the hydraulic conditions. This is primarily a result of the small flow depths and the limited elevation differences. With this manipulation, it was determined that the methods were not appropriate and that the results were of no value.

The use of the continuous flow scour analysis technique for the highly infrequent and extremely short duration flows for the design condition is a gross misapplication of methodology. The extreme sensitivity of the method to flow conditions also indicates that the methods are not appropriate. By the same reasoning, flow regime methods such as Lacey's stable channel design procedures should not be used for highly ephemeral channels. In addition to the difficulty of relating scour depth to a deviance from the stable channel slope, the use of a very short duration PMF design peak flow is not consistent with the intent of producing a stable channel design under steady flow.

Alternative methods for calculation of the scour include the USDOT method and an earlier method which will be designated the Federal Highway Administration (FHA) methods. Both methods where intended for protection of culvert outlet areas and should therefore produce conservative designs for riprap to earthen transitions where there is no distinct change in channel geometry.

In fact, both methods are very similar and it appears that the USDOT method is an update of the FHA method. The USDOT method includes both a peak flow duration and a base flow duration and should be most applicable for these highly ephemeral flow conditions. Two versions of the USDOT equation are provided for cohesive and non-cohesive base materials.

The USDOT cohesive, USDOT non-cohesive, and FHA formulations are as follows:

$$D = \frac{A}{2} \alpha \left[\frac{\rho V}{\tau} \right], \left(\frac{t}{t} \right)$$

$$D = \frac{A}{2}^{st} \alpha \left[\frac{Q}{\left(g^{st} \left(\frac{A}{2}\right)^{st}\right)^{st}} \right]^{t} \left(\frac{t}{t}\right)^{s}$$

$$D = \frac{A}{2}^{\omega} \alpha \gamma \left[\frac{Q}{\left(\left(\frac{A}{2} \right) \right)^{\omega}} \right]' (t_{1})'$$

where:

D = Scour depth (feet),

A = Flow area (square feet),

g = acceleration of gravity,

 t_1 = time of peak flow (minute),

 t_0 = base time of flow (minute),

Q = flowrate (cfs),

V = flow velocity (fps),

 $\tau_c = \text{critical shear stress } (lb/ft^2)$

 ρ = fluid density,

 $\alpha, \beta, \theta, \gamma$ = scour dimension coefficients that are specific to the method.

The base soil condition is expected to range from a very cohesive shale and clay material to a graded sand. The critical shear stress, τ_c was estimated for a cohesive clay as 0.12 lb/ft².

Scour depths in Table 5-8 were calculated for the cohesive clay and shale using α , β , and θ coefficients of 1.37, 0.18 and 0.10. Additional scour depths were calculated for a graded sand with a D₅₀ of 2.0 mm. using α , β , and θ coefficients of 0.75, 0.85 and 0.07. For the FHA method, the α , β , θ , and γ coefficients of 0.76, 0.375, 0.10 and 1.0, respectively, where taken from Table 7.8 of Barfield et al. (1983). The results of calculations for both methods are presented in Table 5-8.

Peak flow and base flow durations in Table 5-8 were taken directly from hydrographs for each channel cross section. The duration of the peak was defined as the time during which the flow exceeded 90% of the peak flowrate. The duration of the peak and base flow were arbitrarily set at 10 and 60 minutes for the overland flow paths. The overland flow paths were used to determine required depth of the toe at the edge of the rock mulch.

Uniform flow conditions were assumed for the scour analysis. The Manning's n for rock areas was set at 0.035 and the n value for earthen areas was set at 0.015. The calculated scour depths for these design conditions are presented in Table 5-8. The required depth of the end protection structures for channels H, N and Q is 4.5, 4.5 and 7 feet, respectively, as indicated by the USDOT. The channel N end protection structure was extended to a depth of 15 feed for conservatism. The natural channel downstream of the end protection structure for the outlet of subbasin D5-4 is at a very mild slope, and a end protection depth of 4.5 feet should be more than adequate. The two feet deep rock toe designated for overland rock to non-rock transitions is more than adequate as indicated by Table 5-8. Lateral flow along the rock toe is not expected to be a problem because upland drainage area has been deliberately limited.

5.3.7 Sedimentation Analysis

Potential sedimentation in channels and surge ponds in the tailings area is nearly eliminated by exclusion of non-tailings

sediment, the potential for accumulation of detrimental volumes of sediment in tailings drainage channels is virtually nil. A qualitative discussion of sediment accumulation impacts for key drainage basins follows.

There is potential for accumulation of sediment in the surge pond located in subbasin D4-4. Subbasin D4-4 is not within the tailings area. Sediment may accumulate in the surge pond at the base of the subbasin, but this storage volume was not considered in flood analysis and this accumulation will not adversely affect flood analysis based on level-pool flood routing. Depth of sediment in the surge pond will have to approach 12 feet before the sediment will completely fill the surge pond.

There is a small portion of subbasin D3-1 that is not within the tailings protection area. The configuration of the confluence of channels G and I will result in the discharge of the majority of the sediment down channel H. Likewise, there is a small portion of the drainage area for channel Q that is not within the tailings protection area. Again, the channel configuration will allow for the transport or flushing of the small quantities of sediment delivered to the channel.

The majority of the south area discharges through the Industrial Pond and associated surge pond area. The permanent pool area in the reclaimed Industrial Pond was assumed to be full in the flood analysis. Therefore, this volume is not necessary to preserve PMF capacity. Approximately 15 feet of sediment accumulation in the Industrial pond, followed by 4 feet of accumulation in the breech drainage channel would be necessary to reduce the channel to a 1 foot flow depth. Any sediment which does accumulate in the channel will be subject to flushing during more severe events.

The north drainage area has a series of four staged surge ponds, and all of the drainage area passes through at least one of the surge ponds. The use of this volume of temporary storage should nearly eliminate sediment delivery to the lowest surge pond in subbasin N-13. The surge ponds in subbasins N-6, N-7 and N-8 have sufficient capacity to absorb a tremendous volume of sediment with no significant detrimental effects. The general land slope of contributing areas to these ponds is very mild, and the sediment delivery to the upland ponds will be very small.

5.3.8 End Protection and Rock Toe Design

The scour depths discussed in Section 5.3.6 were used to determine the geometry and rock sizing in end protection structures. The length and slope of the end protection structures were designed to provide a stable channel bottom under PMF conditions if excessive scour does occur.

Under assumed uniform flow conditions, the Stephenson method was used to determine the stable slope of the end protection structure (see Table 5-9 and Figure 10-4). Manning's n was held constant at 0.035. The stable slope and specified scour depth then dictated the length of the end protection structure. It should be noted that the end protection structures are designated on relevant exhibits (e.g. Exhibit 10-2) by location only. The majority of the end protection structure will be covered by general fill and, therefore, scale of these structures on the exhibits is of little value. The end protection structure for the outlet of subbasin 5-4 will use the intermediate rock and will have dimensions as indicated in Table 5-9.

The design of rock toe protection at the base of rock mulch protected slopes is very similar to the end protection design. The rock mulch will be used and the toe protection will extend to a

depth of 2 ft. (see Figure 10-6). Table 5-9 indicates a slope of 0.27 ft/ft will be a stable slope for the rock toe.

5.3.9 Rock Layer Thickness Oversizing

Abt (1988) presents a rock size adjustment based on placement thickness (Figure 4.8) for rock with a D_{50} less than six inches. This figure indicates that no rock sizing adjustment is required for a thickness of 3 times the D_{50} . At a thickness of 2 times the D_{50} , the figure indicates that the rock should be oversized by a factor of 1.2. The rock sizes in Table 5-2 were evaluated to determine if this adjustment was necessary. Of the 49 rock sizes in Table 5-2, only six required rock D_{50} 's exceed 83% (1 divided by the oversizing factor of 1.2) of the target rock D_{50} . considers that the rock sizes were determined for the flow at the end of a flow path segment, the portion of rock mulch area that does not already incorporate the oversizing factor is probably in the neighborhood of 1%. With a PMF design condition, further oversizing or overthickening is not necessary, particularly when the affected area is so small and all other design considerations are grossly conservative.

5.3.10 Non-Tailings Channel Design

The drainage system design for the areas adjacent to the tailings was done by methods similar to those used on the tailings. Table 5-10 presents the channel design for the channel cross-sections on north drainage and south drainage areas. For the sections shown, the rock riprap size and channel capacity is more than adequate for the 1000-year storm design flow. Just downstream of control section HCN-1, (a 2:1 side slope V-notch section) there is a transition to small riprap on a mildly sloping low-water crossing. Immediately downstream of this section, the design riprap is the large riprap on a 0.25 ft/ft slope. This riprap is

slightly undersized for a 1000-year design storm, but is grossly oversized for a 100-year design storm.

For the south drainage, the channel capacity and rock size for Channel C is more than adequate for a 1000-year storm, and the channel capacity is adequate for a PMF. Likewise, the channel capacity of the Industrial Pond breech channel is large enough to accommodate the PMF with freeboard. The channel rock for the section that conveys the south drainage to the Area 2/8 reservoir will be sized to withstand the 100-year storm.

5.4 Consolidation Analysis

Substantial consolidation of the tailings will occur as the tailings dewatering progresses. There is currently a large volume of water in the tailings facility. Current efforts are directed to eliminating tailings solution. Dewatering wells are in place on ponds 4 and 5 (TW4- and TW5- wells) and are operational. Tailings consolidation to date is expected to be limited to that occurring in sandier areas above the water table, which represents only a small portion of expected settlement.

5.4.1 Settlement Monument Analysis

Settlement monuments have recently been placed on the tailings. The location of the 13 monuments is presented in Exhibit 5-3. Fifteen additional monuments will be installed before January 2001 to provide additional coverage and to replace destroyed monuments. A construction detail for the monuments is shown in Figure 11-1. Progression of consolidation or settlement will be interpreted primarily by monitoring of settlement monuments. Some predictive analysis was conducted and the results of this are discussed later in this section. However, the heterogenous and stratified nature of the tailings, in conjunction with complex

drainage conditions, makes predictive consolidation modeling a very uncertain process. For this reason, the actual settlement measurements are considered more valuable.

Settlement monitoring will be performed on a monthly basis until water levels in both ponds are approaching the base of the tailings or until the t_{90} has been reached. The t_{90} will be evaluated by plotting the settlement vs. square root of time or by plotting settlement vs. log of time.

5.4.2 Subsidence Calculations

Consolidation tests were conducted on five samples of tailings materials. One of the samples (TW4-1B, 25'-27') was a mixed sand and clay; two of the samples (TW4-2C, 40'-42' and TW4-3C, 42'-43') were fat clays; and the remaining samples (TW4-5B, 5.5'-8' and TW4-1C, 3'-5') were sands. The results of laboratory testing are presented in Appendix C. All samples were taken with a Shelby tube sampler to minimize disturbance. Sample were subjected to various loading conditions ranging from 0.5 to 8.0 ksf.

Results of the laboratory tests show a large variability in the consolidation characteristics. Initial void ratios in the samples ranged from 0.759 to 2.007 (see Table 5-11). Coefficients of compression ranged from 0.039 to 0.673, with averages of 0.065 for the sand tailings and 0.248 for the mixed and clay tailings.

An analysis of predicted settlement was conducted to develop estimates of required fill and to determine the potential magnitude of differential settlement. Differential settlement will have to be corrected, particularly in non-rock protected areas, but this is not expected to require large volumes of fill.

A very simple model of consolidation was used to predict total settlement for Pond 4 and Pond 5. The results of this modeling are presented in Table 5-12. The model of consolidation is based on

Terzaghi's consolidation theory. Total settlement due to consolidation can be estimated using the relationships:

$$\Delta H = \frac{HC_c}{1 + e_1} \log \frac{\overline{P}_j}{\overline{P}_j}$$

and

$$\overline{P} = P - U_{\omega}$$

where:

 ΔH = settlement, ft

H = layer thickness, ft

 $C_c = compression index$

e = void ratio

P = effective stress, Ksf

i,j = subscripts indicating initial and final conditions

P = total stress, Ksf

 U_w = pore water pressure, Ksf

Depth of tailings for each of the well locations was determined from well logs and a predicted settlement was computed for each location. A composite compression index for the tailings was based on estimates of sand/clay fractions from well logs.

The cover surcharge was assumed to be 3 ft of material at a density of 110 lb/ft^3 . A void ratio of 1.0 was used for both materials. Calculations were made on 1 ft increments for simplicity in programming. The maximum predicted settlement on Pond 4 is 1.4 ft, and the maximum predicted settlement on Pond 5 is 1.48 ft.

The magnitude of settlement in many areas produces a greater strain than a clay radon/infiltration barrier can withstand without losing effectiveness. Placement of the final clay barrier will not be started until settlement monitoring indicates that the rate of

consolidation has dropped and the t_{90} has been reached. After this point, settlement is not expected to affect the integrity of the clay cap. If any clay radon/infiltration barrier material is placed as an interim cover, it will be tilled and recompacted to eliminate cracks when final cover is placed.

5.5 Water Infiltration

Infiltration rates were estimated using the partially saturated flow model developed by McWhorter and Nelson (1979). The calculated infiltration rates for the maximum time of ponding of water on the tailings during the PMF runoff events were used to estimate the depth of infiltration. Equation 18 of McWhorter and Nelson (1979) was used for these calculations. This equation is equivalent to the Green and Ampt equation. The equation used to calculate the penetration rate of the infiltration was as follows:

$$q = \frac{K_f(H_f + L - H_c)}{I_c}$$

where:

q = seepage rate, cm/sec

 K_f = vertical permeability of material, cm/sec

 H_f = depth of ponding, cm

L = penetration distance, cm

 H_c = effective capillary drive, cm

Several vertical permeability tests were conducted at a compaction of 95 percent standard proctor on the cover material. This data is presented in Appendix C. A typical permeability from these tests is 2.6E-8 cm/sec and this value was used in these calculations. Table 5-13 presents estimates of the depths of infiltration during the PMF. In order to dampen peak flows from the PMF runoff event, channel control structures will be constructed at three locations that retard runoff on the tailings ponds (see Section 5.2.3). These sites are designated HCT-1 (No.

4 tailings), and HC5-3 and HC5-11 (No. 5 tailings) on Exhibit 10-2. Based on HEC-1 simulations of the PMP runoff event, maximum time that water will be ponded at these sites is presented in Table 5-The maximum depth of ponding varies for the different areas of the tailings and are presented in this table. The maximum time of ponding of water over these tailings for the PMF was also used. The maximum time of ponding from the PMF was obtained from the hydrographs in Appendix E for channel control sites. The maximum ponding depths were obtained from the depth of flow at the cross section which is given in the eleventh column in Table 5-4. depths at the control cross sections are the maximum depth of flow due to ponding. The rating curves for the different control outlets are given in SQ-SE rows in Tables E-1 and E-2 of Appendix The infiltration depth was obtained from the penetration rate equation on page 5-44 and the following infiltration depth equation:

L=q(t)/n

where: the parameters are the same as on the previous page, plus:

t = time of ponding, seconds

n = effective porosity

An effective capillary drive head of -310 cm was estimated to be representative of the compacted clay. This estimate was obtained from Table 1-3 of Harr (1962). The wetting front depth was varied in the calculations until the seepage rate resulted in that particular depth of penetration for the time of ponding. An effective porosity of 0.05 was used to calculate the penetration depth based on the infiltration rate and time of ponding. The estimate of effective porosity (specific yield) for the compacted clay was obtained from Figure 4.2 of Bear (1987) and Table 5.2 of Driscoll (1986). For example, the seepage rate equation for site

HCT-1 produces a rate of 3.1 E-6 cm/sec for an infiltration depth of 3.92 cm, a maximum ponding of 152 cm, and an effective capillary drive of -310 cm. Table 5-13 shows that the penetration rates into the clay confining layer should be relatively small at less than 5 cm. during the PMF. Water will be removed by evapotranspiration after this infiltration occurs. The removal of the drainable portion of water is important relative to continual movement of water downward. An average annual potential evapotranspiration rate for the Shirley Basin is estimated to be 0.057 inches/day from Figure 12.3 of Martner (1986). A rate of one-half of this value was used to estimate the removal rate of water because the compacted clay will be covered by rock mulch or topsoil. seepage model provided an estimate of the depth of penetration of the wetting front from the PMF. The evapotranspiration rate indicates the number of days for this water to be removed by evapotranspiration. The effective removal time is presented in Table 5-13, based on the evapotranspiration rate of 0.0285 inches of water per day and the removal rate of wetting front of 0.57 inches/day (0.0285/0.05). Therefore, evapotranspiration should be able to remove the water that penetrates the clay layer in a few days after the PMF event.

5.6 WIND EROSION

There will be no potential for wind erosion in the tailings disposal and mill areas where rock mulch will be used as the ground cover. However, in locations where a vegetated cover is planned, wind erosion potential must be addressed. In those areas, vegetative density and variety is expected to be comparable to that in the undisturbed areas after 20 or so years.

SCS has developed a wind erosion equation (WEQ) (SCS, 1982) which was used to predict the amount of soil loss due to wind over

a 1,000 year time period. This WEQ is expressed as a function of the following parameters:

I = soil erodibility by wind,
K = soil surface roughness,

C = climatic factor,

L = field width

and V =vegetative cover.

E, soil loss in tons per acre per year, is then determined from a series of charts based on values assigned to the above parameters.

In 1986, SCS personnel from their Casper, Wyoming office estimated the WEQ parameters for Pathfinder's Shirley Basin mine, mill, and tailings disposal site. The parameters, which were given as an example in SCS (1982), were determined by field analysis and are considered by SCS personnel to still be valid (verbal communication, L. Young, September, 1993). The parameters are as follows:

I = 56, K = 1.0,

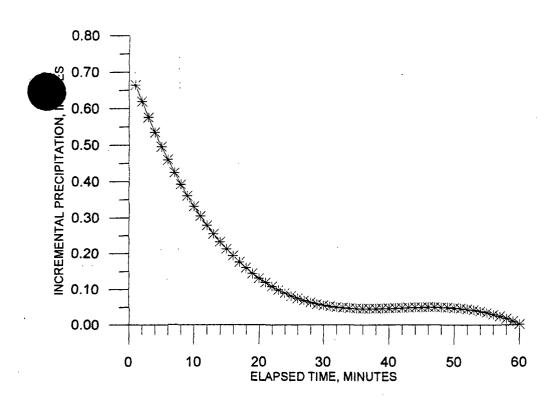
C = 100,

and V = 3700 lb/ac flat small grain residue.

The field width factor, L, is the width, in feet, of the unsheltered distance across a strip parallel to the direction of the prevailing erosive wind. In the Shirley Basin, the prevailing erosive wind is from the south southwest. The maximum width of the vegetative cover on Pathfinder's tailings after reclamation, measured in the south southwest direction is approximately 2,800 ft. This value was used in the WEQ.

In the tables given in SCS (1982), the maximum value of V for the other WEQ parameters given above is 2000 lb/ac. E, annual soil loss, based on 2000 lb/ac, is estimated from SCS' tables to be 0.9 tons/ac. Assuming the unit weight of soil is 105 lb/ft 3 , annual soil loss is 0.00039 ft/yr. Over 1,000 years then, 0.39 ft (4.7)

in) of cover material is predicted to be lost to wind erosion. This estimate is considered to be conservative because SCS has estimated a vegetative cover factor of 3700 lb/ac and the above estimate is based on 2000 lb/ac.



MIN	PRECIP.	PRECIP.
0	0.0000	0.0000
1	0.6647	0.6647
2	0.6192	1.2839
3	0.5759	1.8597
4	0.5348	2.3945
5	0.4958	2.8903
6	0.4590	3.3493
7	0.4242	3.7735
8	0.3914	4.1649
9	0.3605	4.1049
10	0.3315	4.8569
11	0.3043	5.1611
12	0.2788	5.4399
13	0.2551	5.6950
14	0.2330	5.9280
15	0.2124	6.1404
16	0.1934	6.3338
17	0.1759	6.5097
18	0.1598	6.6695
19	0.1450	6.8145
20	0.1315	6.9460
21	0.1193	7.0653
22	0.1083	7.1736
23	0.0940	7.2720
24	0.0895	7.3615
25	0.0817	7.4432
26	0.0748	7.5180
	0.0689	7.5869
27		
28	0.0637	7.6506
29	0.0594	7.7100
30	0.0558	7.7658
31	0.0528	7.8186
32	0.0505	7.8691
33	0.0487	7.9178
34	0.0474	7.9652
35	0.0466	8.0118
36	0.0461	8.0579
37	0.0460	8.1039
38	0.0462	8.1501
39	0.0465	8.1966
40	0.0470	8.2436
41	0.0477	8.2913
42	0.0483	8.3396
43	0.0490	8.3886
44	0.0496	8.4382
45	0.0500	8.4882
46	0.0503	8.5384
47	0.0503	8.5887
	· -	
48	0.0500	8.6387
49	0.0494	8.6881
50	0.0483	8.7364
51	0.0468	8.7832
52	0.0448	8.8280
53	0.0421	8.8701
54	0.0388	8.9089
55	0.0349	8.9438
56	0.0301	8.9739
57	0.0246	8.9985
58	0.0182	9.0167
59	0.0108	9.0275
60	0.0025	9.0300
00	0.0020	3.0300

TIME

MIN

INCREM.

PRECIP.

CUM.

PRECIP.

CUMULATIVE PRECIPITATION INCHES	1 — 1	
	0	
	0 10	20 30 40 50 60 ELAPSED TIME, MINUTES

FIGURE 5-1. INCREMENTAL AND CUMULATIVE 1-HR, 1-MI 2 PMP PRECIPITATION DISTRIBUTIONS FOR OVERLAND FLOW ANALYSIS.

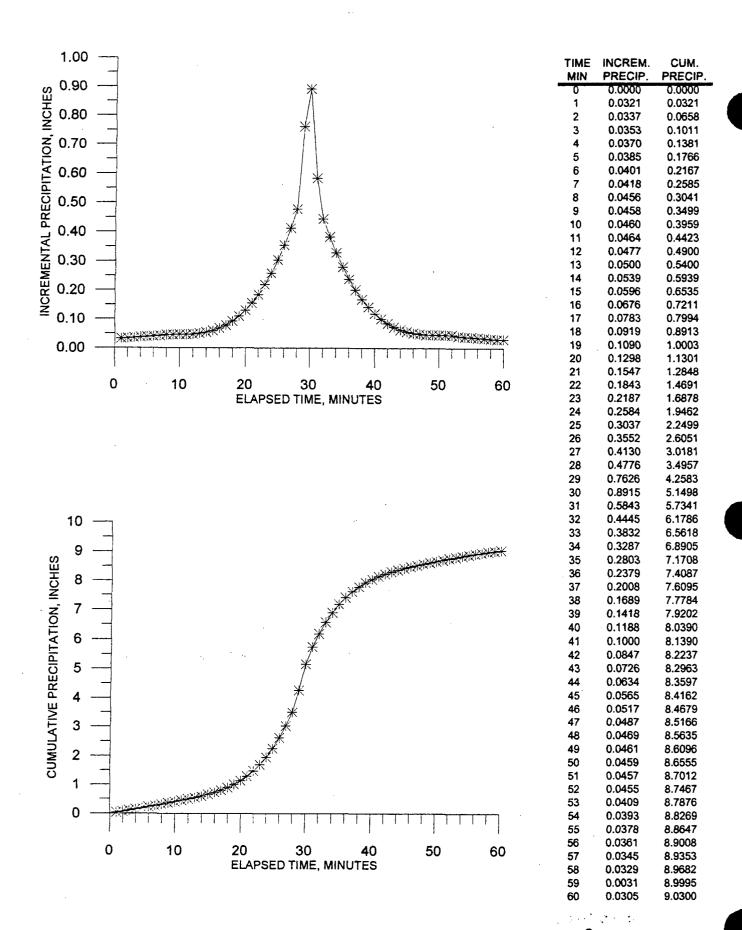


FIGURE 5-2. INCREMENTAL AND CUMULATIVE 1-HR, 1-MI² PMP PRECIPITATION DISTRIBUTIONS FOR HEC-1 ANALYSIS.

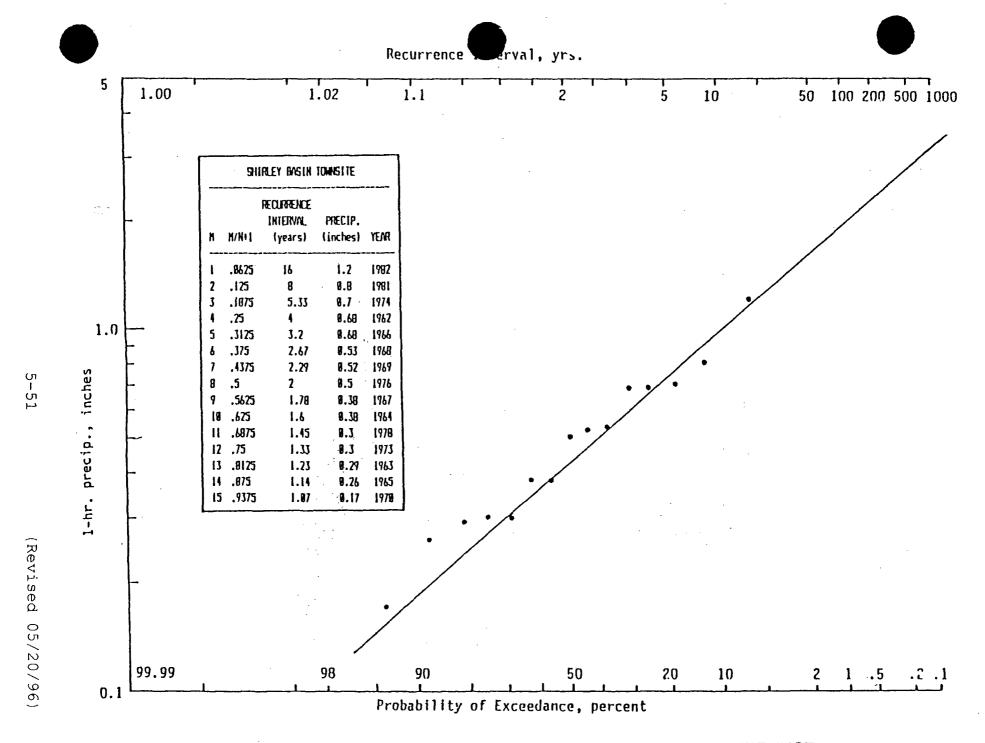
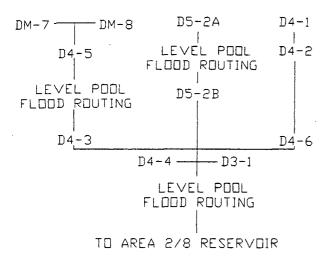
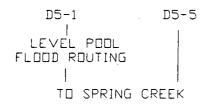


FIGURE 5-3. ONE-HOUR DURATION PRECIPITATION VERSUS RECURRENCE INTERVAL FOR SHIRLEY BASIN TOWNSITE.

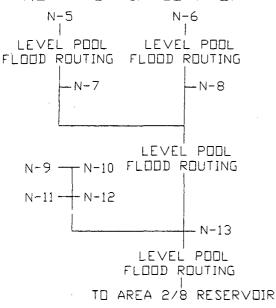
WEST TAILINGS DRAINAGE AREA



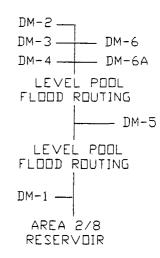
EAST TAILINGS DRAINAGE AREA



NORTH DRAINAGE AREA



SOUTH DRAINAGE AREA



SOUTHEAST TAILINGS DRAINAGE AREA

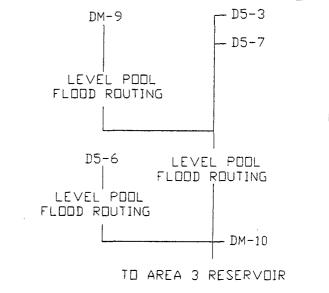


TABLE 5-1. BASIN CHARACTERISTICS FOR TAILINGS AREA SUBBASINS.

BASIN	Area (acre)	Area (mi^2)	Hydrologic High Elev. (ft-msl)	Hydrologic High Elev. (ft-msl)	Basin Relief (ft)	Basin Length (ft)	Basin Slope (ft/ft)	Kirpich's tc (hour)	Kirpich's tL (hour)	SCS Curve Number
DM-1	10.76	0.017	7165	7110	55	800	0.069	0.063	0.0376	91
DM-2	27.40	0.043	7182	7128	54	3090	0.017	0.301	0.1805	91
DM-3	18.54	0.029	7182	7127	55	2250	0.024	0.207	0.1243	91
DM-4	21.86	0.034	7180	7120	60	1950	0.031	0.170	0.1019	91
DM-5	22.64	0.035	7133	7110	23	1120	0.021	0.179	0.0777	91
DM-6	49.72	0.033	7162	7115	47	1440	0.033	0.123	0.0788	91
DM-6A	6.67	0.070	7137	7113 7119	18	980	0.033	0.131	0.0731	91
DM-7	7.67	0.012	7138	7119	13	480	0.013	0.122		91
DM-7(Overla		0.012					0.027		0.0364	91
		0.007	7140	7138	2	650		0.177	0.1061	•
DM-8	4.44	0.007	7139	7125	14	850	0.016	0.114	0.0684	91
DM-8(Overla	•		7140	7139	1	320	0.003	0.102	0.0611	
DM-9	29.13	0.046	7140	7110	30	1270	0.024	0.135	0.0811	91
DM-10	9.47	0.015	7148	7093	55	220	0.250	0.014	0.0085	91
DM-10(Low s	Slape)		7093	7065	28	1000	0.028	0.105	0.0632	91
D3-1	46.41	0.073	7105	7083	22	1020	0.022	0.118	0.0709	89
D3-1(overlan	nd)		7121	7105	16	800	0.020	0.101	0.0605	89
D4-1	24.83	0.039	7115	7108	7	1780	0.004	0.349	0.2096	91
D4-1(Overlar	nd)		7116	7115	1	350	0.003	0.113	0.0678	
D4-2	24.83	0.039	7108	7083	25	1460	0.017	0.170	0.1021	85
D4-3(Path 1)	29.05	0.045	7114	7083	31	1300	0.024	0.137	0.0822	86
D4-3(Overlar	nd)		7120	7114	6	500	0.012	0.086	0.0513	
D4-3(Path 2)	29.05	0.045	7115	7083	32	1300	0.025	0.135	0.0812	86
D4-3(Overlar	nd)		7116	7115	1	550	0.002	0.190	0.1142	
D4-4	27.98	0.044	7124	7075	49	980	0.050	0.083	0.0497	91
D4-5	24.31	0.038	7125	7114	11	1350	0.008	0.213	0.1280	87
D4-6	32.34	0.051	7114	7083	31	920	0.034	0.092	0.0552	91
D4-6(Overlan	nd)		7116	7114	2	280	0.007	0.067	0.0401	• ,
D5-1	87.30	0.136	7110	7093	17	3480	0.005	0.539	0.323	85
D5-2A	16.42	0.026	7109	7100	9	1000	0.009	0.163	0.098	91
D5-2B	4.87	0.008	7100	7083	17	770	0.022	0.094	0.057	87
D5-3	28.20	0.044	7110.5	7100	10.5	2240	0.005	0.390	0.234	91
D5-4	33.22	0.052	7110.5	7050	60.5	3900	0.016	0.377	0.226	87
D5-5	6.70	0.010	7108.5	7080	28.5	890	0.032	0.091	0.055	87
D5-6	7.19	0.011	7105	7065	40	1080	0.037	0.100	0.060	87
D5-7	6.55	0.010	7111	7105	6	810	0.007	0.149	0.090	87
N-5	23.19	0.036	7290	7280	10	2120	0.005	0.373	0.2236	91
N-6	29.85	0.047	7285	7250	35	1680	0.021	0.373	0.1055	91
N-7	13.21	0.021	7290	7268	22	1520	0.021	0.170	0.1033	
N-8	38.36	0.060	7250	7235	15		0.014			91
N-9	23.91	0.037	7230 7170	7235 7140		2920		0.461	0.2769	91
N-10	1.64	0.037	7170 7278	7140 7234	30 44	1530	0.020	0.168	0.1005	91
N-10 N-11			_			540 570	0.081	0.043	0.0260	91
N-11 N-12	8.57	0.013	7137	7114	23	570	0.040	0.059	0.0356	91
	11.60	0.018	7155	7114	41	2030	0.020	0.206	0.1235	91
N-13	8.05	0.013	700-		0				0.0000	100
N-14	24.55	0.038	7236	7110	126	2840	0.044	0.197	0.1182	91

TABLE 5-2. OVERLAND FLOW PATH CHARACTERISTICS AND ROCK MULCH DESIGN.

Path Name	Length (feet)	Relief (feet)	Slope (ft/ft)	tc (min)	Discharge (cfs/ft)	Rock D50 (in)
O3-1A	110	21.5	0.195	0.546	0.076	0.708
*O3-2A	390	5	0.013	4.132	0.256	0.252
Q3-2B	170	6	0.035	1.169	0.389	0.804
O3-2C	280	9	0.032	1.653	0.584	0.972
O3-2D	90	5	0.056	0.409	0.646	1.752
O3-2E	.90	3	0.033	0.391	0:709	1.128
*O3-3A	470	12	0.026	3.659	0.314	
O3-3B	140	4	0.029	0.809	0.424	0.720
O3-3C	190	10	0.053	0.955	0.556	1.500
O3-3D	100	3	0.030	0.436	0.626	0.960
**O4-3A	400	1.1	0.003	7.621	0.233	
O4-3B	100	5.9	0.059	0.754	0.348	1.224
O4-3C	120	5	0.042	0.756	0.431	1.020
04-3D	260	15	0.058	1.351	0.612	1.740
O4-3E	90	5	0.056	0.372	0.674	1.800
04-3F	150	1	0.007	0.663	0.779	0.300
04-4A	270	4	0.015	2.945	0.185	0.228
		5	0.013	0.684	0.165	0.852
O4-4B	100					
04-4C	150	5	0.033	0.922	0.361	0.720
04-4D	90	5	0.056	0.454	0.424	1.320
04-4E	90	5	0.050	0.469	0.494	1.320
04-4F	120	1.5	0.013	0.566	0.577	0.420
*O4-5A	520	17	0.033	3.596	0.348	0.708
*O4-5B	70	9	0.129	0.301	0.410	1.392
*04-7A	120	7	0.058	0.930	0.083	
*O4-7B	150	1	0.007	1.144	0.188	
04-7C	200	3	0.015	1.410	0.327	0.336
04-7D	230	3	0.013	1.511	0.487	0.384
04-70	200		0.010	1.011	0.401	0.004
O4-8A	240	1	0.004	4.383	0.156	@
O4-8B	200	5	0.025	1.781	0.306	0.504
O4-8C	240	6	0.025	1.684	0.473	0.672
**05-1A	90	1	0.011	1.412	0.063	
O5-1B	110	24	0.218	0.515	0.139	1.356
O5-1C	90	15	0.167	0.341	0.202	1.200
**05-2A	170	1.1	0.006	2.837	0.117	
O5-2B	310	58	0.187	1.225	0.334	1.776
00 £D	310					
*O5-3A	360	1.3	0.004	6.328	0.219	
O5-3B	50	0.2	0.004	0.613	0.285	@

TABLE 5-2. OVERLAND FLOW PATH CHARACTERISTICS AND ROCK MULCH DESIGN (continued).

					:	
Path	Length	Relief	Slope	tc	Discharge	Rock D50
Name	(feet)	(feet)	(ft/ft)	(min)	(cfs/ft)	(in)
**O5-5A	180	1	0.006	3.144	0.122	
O5-5B	150	11	0.073	0.978	0.229	1.140
00 02	,00	, ,	0,070	0.070	0,220	1.140
**O5-6A	90	0.5	0.006	1.843	0.063	
O5-6B	140	11	0.079	0.918	0.160	0.972
**O5-7A	290	0.5	0.002	7.121	0.172	
O5-7B	220	15	0.068	1.380	0.355	1.416
O5-7C	100 -	0.5	0.005	0.548	0.424	0.192
						÷
**O5-8A	90	1	0.011	1.412	0.063	
O5-8B	80	15	0.188	0.424	0.118	0.900
O5-8C	80	5	0.063	0.382	0.174	0.828
**O5-9A	60	1.5	0.025	0.756	0.040	
05-9A 05-9B	120	4.5	0.025 0.038	0.756 0.987	0.042	0.408
05-9C	260	10	0.038	1.653	0.125	
O5-9D	190	3			0.306	0.744
O3-9D	190	3	0.016	1.077	0.438	0.420
O5-10A	420	13	0.031	3.116	0.286	0.588
**O5-11A	50	0.7	0.014	0.821	0.035	
**05-11B	300	1.1	0.004	4.523	0.227	
O5-11C	30	3	0.100	0.217	0.264	0.816
O5-11D	140	5	0.036	0.977	0.361	0.792
O5-11E	360	5	0.014	2.481	0.612	0.468
**05-12A	110	0.5	0.005	2.324	0.076	
O5-12B	40	· 3	0.075	0.342	0.104	0.696
O5-12C	190	5	0.026	1.468	0.236	0.444
O5-12D	550	5	0.009	4.193	0.583	0.312
O5-14A	300	67	0.223	1.124	0.200	4 506
05-147	300	07	0.223	1.124	0.209	1.596
**OM-1A	450	1.4	0.003	7.958	0.259	
OM-1B	130	0.5	0.004	1.686	0.403	0.180
OM-1C	160	9	0.056	1.094	0.514	1.500
*OM-1D	70	3	0.043	0.383	0.563	1.000
J 15	, •	J	0.040	0,000	0.005	
*OM-2A	100	0.4	0.004	2.269	0.070	
OM-2B	270	14	0.052	1.798	0.257	0.888
*OM-2C	240	15	0.063	1.192	0.424	- -

^{*} FLOW PATH SEGMENT ON NON-TAILINGS AREA

^{**} NON-ROCK AREA

[@] LOW SLOPE - ROCK SIZING PROGRAM WILL NOT CONVERGE (SPECIFIED ROCK MULCH IS OVERSIZED FOR THIS SEGMENT)

TABLE 5-3. OVERLAND FLOW PATH CHARACTERISTICS AND ALLOWABLE SLOPE LENGTH

Path Name	Length (feet)	Relief (feet)	Slope (ft/ft)	tc (min)	Discharge (cfs/ft)	Max. Slope Length (ft)
*O3-2A	390	5	0.013	4.132	0.288	81
*O3-3A	470	12	0.026	3.659	0.353	36
O4-1A	500	1.5	0.003	8.752	0.314	516
O4-2A	360	1.1	0.003	6.748	0.243	471
O4-3A	400	1.1	0.003	7.621	0.262	549
*O4-5A	520	17	0.033	3.596	0.391	27
*O4-5B	70	. 9	0.129	0.301	0.461	1
*O4-7A	120	7	0.058	0.930	0.094	13
O5-1A	90	1	0.011	1.412	0.070	90
O5-2A	170	1.1	0.006	2.837	0.131	171
O5-3A	360	1.3	0.004	6.328	0.246	382
O5-4A	550	1.5	0.003	9.771	0.334	597
O5-5A	180	1	0.006	3.144	0.138	207
O5-6A	90	0.5	0.006	1.843	0.070	202
O5-7A	290	0.5	0.002	7.121	0.193	930
O5-8A	90	1	0.011	1.412	0.070	90
O5-11A	50	0.7	0.014	0.821	0.039	69
O5-11B	300	1.1	0.004	4.523	0.255	302
O5-12A	110	0.5	0.005	2.324	0.086	255
O5-13A	150	1	0.007	2.547	0.117	164
OM-1A	440	1.4	0.003	7.958	0.291	481
OM-2A	100	0.4	0.004	2.269	0.078	296

^{*} FLOW PATH ON NON-TAILINGS AREA

ABLE 5-4. CHANNEL CONVEYANCE AND INITIAL ROCK SIZING FOR PMF.

Cross Section	Bottom Elevation (Ft+MSL)	Bottom Width (ft)	Side Slope (H : V)	Energy Gradient (ft/ft)	Bottom Slope (ft/ft)	Discharge (cfs)	Velocity (fps)	Flow Area (sq. ft.)	Manning's n	Depth (ft)	Riprap D50 (ft)	Target Rock D50 (ft)
	Manning's	Equation	Channel C	Conveyanc	e							
HC4-1	7108.6	20	4:1	0.008		399	7.4	53.8	0.024	1.94	0.162	0.4
HC4-2	7106.7	40	4:1	0.014		399	7.1	56.4	0.027	1.25	0.214	0.4
HC4-3	7089	50	4:1	0.033		581	8.4	69.5	0.036	1.26	0.529	0.6
HC4-4	7103	20	4:1	0.009		270	6.8	39.6	0.024	1.52	0.148	0.15
HC4-5	7100.1	0	2:1	0.006		36	5.2	6.9	0.02	1.86	0.067	0.6
HC4-6	7084	50	6.4 : 1	0.035		170	5.7	29.9	0.032	0.56	0.258	0.6
HC4-7	7081	0	2:1					PONDED				0.15
HC4-8	7081	40	4:1					PONDED)			
HC4-9	7126.4	20	4:1	0.039	0.022	180	7.4	24.3	0.036	1.01	0.469	0.4
HC4-10	7127.8	20	4:1	0.02		110	5.9	18.6	0.028	8.0	0.192	0.4
HC4-11	7114.8	20	4:1	0		289		PONDED)			0.4
HC4-12	7108.8	36	4:1	0.005		797	7.7	103.9	0.021	2.3	0.127	0.4
HC4-13	7085.8	50	4:1	0.05		1300	11.1	116.8	0.044	2.01	1.246	1.2
HCT-1	7081	0	2:1	0.006		620	9.3	66.7	0.023	5.78	0.209	1.2
HCT-2	7079.4	40	4:1	0.007		620	7.3	84.4	0.023	1.79	0.146	1.2
HCT-3	6971.4	40	4:1	0.131		620	11.6	53.6	0.049	1.2	0.988	1.2
HC5-1	7097	60	10:1	0.001		1149		PONDED)		,	0.15
HC5-2	7094.5	40	4:1	0.004		1149		PONDED)		`	0.15
HC5-3	7093	0	2:1	0.006		180	7.2	24.9	0.022	3.53	0.128	1.2
HC5-4	7088.9	25	4:1	0.105		180	8.7	20.6	0.042	0.74	0.479	0.6
HC5-5	7077.5	30	4:1	0.026		229	6.9	33	0.031	0.97	0.312	0.6
HC5-6	7101.5	25	4:1	0.003		501	6.7	75	0.018	2.21	0.07	0.4
HC5-7	7107	20	4:1	0.007		173	5.9	29.5	0.021	1.19	0.094	0.4
HC5-8	7102.5	20	4:1	0.003		173	5.2	33	0.016	1.31	0.043	0.4
HC5-9	7097	20	4:1	0.013		583	8.7	67.1	0.028	2.3	0.304	0.4
HC5-10	7110.1	1	2:1	0.016		243	8.9	27.4	0.03	3.46	0.363	1.2
HC5-11	7095	Ó	2:1	0.013	0.002	706	10.7	66.3	0.03	5.76	0.455	0.4
HC5-12	7071.3	30	4:1	0.025		751	9.6	78.2	0.034	2.05	0.575	0.6
HC5-13	7065	0	2:1	0.002		121	2.3	52.3	0.05	5.11	0.062	0.6
HC5-14	7062.5	30	4:1	0.012		843	9	93.8	0.028	2.38	0.308	0.6

TABLE 5-5. CHANNEL CONVEYANCE CHARACTERISTICS FOR PMF WITH STEPHENSON ROCK SIZING.

Cross Section	Bottom Elevation (Ft+MSL)	Bottom Width (ft)	Side Slope (H : V)	Energy Gradient (ft/ft)	Discharge (cfs)	Velocity (fps)		Manning's n	Depth (ft)	Riprap D50 (ft)	Target Rock D50 (ft)	Alternate (por = 0.45) Riprap D50 (ft)
	Manning's	Equation (Channel C	onveyance	•							
HC4-1	7108.6	20	4:1	0.008	399	7.4	53.8	0.024	1.94	0.092	0.4	0.12
HC4-2	7106.7	40	4:1	0.014	399	7.1	56.4	0.027	1.25	0.104	0.4	0.14
HC4-3	7089	50	4:1	0.033	581	8.4	69.5	0.036	1.26	0.233	0.6	0.30
HC4-4	7103	20	4:1	0.009	270	6.8	39.6	0.024	1.52	0.081	0.15	0.11
HC4-5	7100.1	0	2:1	0.006	36	5.2	6.9	0.02	1.86	0.056	0.6	0.07
HC4-6	7084	50	6.4 : 1	0.035	170	5.7	29.9	0.032	0.56	0.110	0.6	0.14
HC4-7	7081	0	2:1				PONDE				0.15	
HC4-8	7081	40	4 : 1				PONDE	D				
HC4-9	7126.4	20	4:1	0.039	180	7.4	24.3	0.036	1.01	0.212	0.4	0.28
HC4-10	7127.8	20	4 : 1	0.02	110	5.9	18.6	0.028	0.8	0.091	0.4	0.12
HC4-11	7114.8	20	4:1	0	289		PONDE	D			0.4	
HC4-12	7108.8	36	4:1	0.005	797	7.7	103.9	0.021	2.3	0.073	0.4	0.10
HC4-13	7085.8	50	4:1	0.05	1300	11.1	116.8	0.044	2.01	0.542	0.6	0.71
HCT-1	7081	0	2:1	0.006	620	9.3	66.7	0.023	5.78	0.177	1.2	0.23
HCT-2	7079.4	40	4:1	0.007	620	7.3	84.4	0.023	1.79	0.078	1.2	0.10
HCT-3	6971.4	40	4:1	0.131	620	11.6	53.6	0.049	1.2	0.943	1.2	1.23
HC5-1	7097	60	10 : 1	0.001	1149		PONDE	.D			0.15	
HC5-2	7094.5	40	4:1	0.004	1149		PONDE	D			0.15	
HC5-3	7093	0	2:1	0.006	180	7.2	24.9	0.022	3.53	0.107	1.2	0.14
HC5-4	7088.9	25	4:1	0.105	180	8.5	21.2	0.044	0.76	0.479	0.6	0.60
HC5-5	7077.5	30	4:1	0.026	229	7.2	31.8	0.034	0.94	0.143	0.6	0.19
HC5-6	7101.5	25	4:1	0.003	501	6.7	75	0.018	2.21	0.044	0.4	0.06
HC5-7	7107	20	4:1	0.007	173	5.9	29.5	0.021	1.19	0.051	0.4	0.07
HC5-8	7102.5	20	4:1	0.003	173	5.2	33	0.016	1.31	0.026	0.4	0.03
HC5-9	7097	20	4:1	0.013	583	8.7	67.1	0.028	2.3	0.169	0.4	0.22
HC5-10	7110.1	1	2:1	0.016	243	8.9	27.4	0.03	3.46	0.265	1.2	0.35
HC5-11	7095	0	2:1	0.013	706	10.7	66.3	0.03	5.76	0.357	0.4	0.46
HC5-12	7071.3	30	4 ; 1	0.025	751	9.6	78.2	0.034	2.05	0.281	0.6	0.37
HC5-13	7065	0	2:1	0.002	121	2.3	52.3	0.05	5.11	0.027	0.6	0.04
HC5-14	7062.5	30	4:1	0.012	843	9	93.8	0.028	2.38	0.166	0.6	0.22

ABLE 5-5. CHANNEL CONVEYANCE CHARACTERISTICS FOR PMF WITH STEPHENSON ROCK SIZING (continued). Alternate Target (por = 0.45)Cross **Bottom** Flow Manning's Riprap **Bottom** Side Energy Rock Riprap Depth 050 Section Elevation Width Gradient Discharge Velocity Area Slope n **D50** D50 (Ft+MSL (ft) (H:V) (ft/ft) (cfs) (ft) (ft) (ft) (ft) (fps) (sq. ft.) Flow Based on HEC-2 Channel Conveyance Channel A HC4-9(90) 7126 4 20 0.039 0.035 0.277 4 : 1 180 7.5 24 1 0.213 0.4 A- 200 7128.8 20 4 : 1 0.031 170 6.8 25 0.035 1.03 0.168 0.4 0.219 7129.4 A- 220 : 1 20 0.021 165 6 27.6 0.035 1.13 0 120 0.4 0.156A- 240 7130 20 : 1 0.058 160 8.3 19.4 0.035 0.83 0.281 0.4 0.366 A- 260 7131.2 20 4 : 1 0.052 153 79 19.5 0.84 0.035 0.250 n 4 0.325 A- 280 7132.4 20 4 : 1 0.08 147 8.9 16.5 0.035 0.72 0.355 0.4 0.462 A- 400 7135.2 20 : 1 0.012 120 4.4 27.4 0.035 1.12 0.062 0.081 0.4 A- 500 7135.9 4:1 20 0.005 110 3.2 34.5 0.035 1.36 0.029 0.4 0.037 A- 600 7136.5 20 4 : 1 0.006 100 3.3 30.6 0.035 1.23 0.032 0.4 0.041 A-700 7137.1 20 : 1 0.005 4 90 3 29.7 0.035 1.2 0.025 0.033 04 A-800 7137.7 20 4 : 1 0.005 80 2.9 27.6 0.035 1.13 0.024 0.4 0.031 Channel D 7083.5 D- 150 0.025 50 4 : 1 1325 10.3 128.6 0.035 2.19 0.308 1.2 0.401 D- 200 7085 50 4 : 1 0.052 1300 13 100.2 0.035 1.76 0.570 0.742 12 HC4-13(215) 7085.8 50 : 1 4 0.05 1300 12.8 101.3 0.035 1.77 0.548 1.2 0.713 D- 220 7086.1 50 : 1 0.05 1297 12.8 1013 0.035 1.77 0.548 1.2 0.713 D-240 7087.1 50 4 : 1 0.05 100.8 1285 12.8 0.035 1.77 0.5481.2 0.713 D- 260 7088.1 50 4 : 1 0.048 1274 12.6 101.1 0.035 1.77 0.524 1.2 0.682 D- 280 7089.2 50 : 1 0.045 1262 103 0.035 0.643 12.3 1.8 0.494 1.2 D- 300 7090.2 50 4 : 1 0.038 1250 11.6 108 0.035 1.88 0.425 1.2 0.553 D- 320 7091 50 4 0.038 1240 107.7 0.035 1.87 • 1 11.5 0.421 1.2 0.548 D- 340 7091.8 50 4 : 1 0.037 1230 11.4 107.5 0.035 1.87 0.409 1.2 0.533D- 360 7092.5 50 : 1 0.037 107.1 1220 11.4 0.408 0.035 1.86 0.531 0.6 D- 380 7093.3 50 4 :1 0.035 1210 11.2 108.4 0.035 1.88 0.388 0.6 0.504 D- 400 7094.1 50 4:1 0.031 1200 10.7 0.035 1:95 112.5 0.3490.6 0.454 D- 420 7094 7 50 4 : 1 0.031 1190 10.7 111.6 0.035 1.93 0.346 0.6 0.451 D- 440 7095.4 50 0.031 1180 10.6 111.2 0.035 1.93 0.344 0.6 0.448 D- 460 7096 50 4 0.031 : 1 1170 106 110 4 0.035 1.91 0.342 0.6 0.445 D- 480 7096.7 50 4 : 1 0.031 1160 10.5 110.1 0.035 1.91 0.340 0.6 0.442 D- 500 7097.3 50 : 1 0.031 1150 109.4 10.5 0.035 1.9 0.3380.6 0 441 D- 520 7097.9 50 4 : 1 0.031 1140 10.5 108.5 0.035 0.337 0.439 1.89 0.6 D- 540 7098.6 50 4 : 1 0.031 1130 10.4 108.4 0.035 1.88 0.334 0.6 0.435 D- 560 7099.2 50 • 1 4 0.031 1120 10.4 107.7 0.035 1.87 0.333 0.6 0.433 D- 580 7099.9 50 4 : 1 0.03 1110 10.3 108 0.035 1.88 0.323 0.6 0.420 D-600 7100.5 50 4:1 0.028 1100 10 110.1 0.035 1.91 0.303 0.6 0.394D- 700 7102 9 50 4 : 1 1000 0 LOW SLOPE 0.6 D- 800 7103.5 50 4:1 0.001 950 LOW SLOPE-0.4 D- 900 50 7104 2 4 : 1 0.001 900 LOW SLOPE 0.4 D- 1000 7105.1 50 : 1 0.001 875 LOW SLOPE 04 D- 1100 7106.1 50 : 1 4 0.001 850 LOW SLOPE 0.4 D- 1200 7107 50 4 : 1 0.001 797 LOW SLOPE 0.4 D- 1220 7107.2 48 4:1 0.001 797 LOW SLOPE 0.4 D- 1240 7107.3 46 277.2 4:1 0.001 797 2.9 0.017 0.035 4.37 0.4 0.022 D- 1260 7107.5 44 : 1 0.001 797 3.1 256.8 0.035 4.22 0.017 0.4 0.022 D- 1280 7107.6 42 : 1 0.001 797 3.3 240.7 0.035 4.12 0.017 0.023 0.4D- 1300 7107.8 40 4:1 0.002 797 3.7 217.7 0.035 3.91 0.031 0.041 0.4 D- 1320 7108 39 : 1 0.002 797 4 198.3 0.035 3.69 0.032 0.4 0.041 D-1340 7108.3 38 4:1 0.003 797 4 4 181.7 0.035 3 49 0.045 0.4 0.058 D- 1360 7108 5 37 4:1 0.004 797 4.8 164.3 0.035 3.28 0.057 0.4 0.074 HC4-12(1380) 7108.8 36 41:1 0.005 797 5.4 147 4 0.035 3.06 0.070 0.4 0.091 D- 1400 7109 35 4:1 0.007 797 6 132.6 0.035 2.86 0.093 0.4 0.121 D- 1420 7109.3 34 4:1 0.01 797 6.9 2.62 1156 0.035 0.1280.4 0.167 D- 1440 7109.5 32 : 1 0.013 797 7.7 103.2 0.035 2.45 0,162 0.4 0.211 D- 1480 7110.1 30 : 1 0.018 797 8.6 92.1 0.035 2.35 0.4 0.286 0.220 D- 1500 7110.4 29 4:1 0.016 797 8.4 94.5 0.035 2.46 0.203 0.4 0.264 D- 1520 7110.6 27 4 : 1 0.015 797 8.3 96.1 0.035 2.57 0.257 0 197 0.4 D- 1540 7110.9 26 : 1 0.018 797 8.8 90.1 0.035 2.51 0.233 0.4 0.304 D- 1560 7111.2 25 4 : 1 0.015 797 8.4 95 0.035 2.69 0.205 0.4 0.267 D- 1580 7111.4 23 : 1 0.018 797 88.4 9 0.035 2.62 0.2440.4 0.317 D- 1620 7111.9 : 1 22 0.011 797 7.5 105.8 0.035 3.11 0.164 0.4 0.213 D- 1640 7112.1 21 4 : 1 0.011 797 7.6 104.5 0.035 0.165 3:11 04 0.215

7.6

7.7

104.3

104.2

0.035

0.035

3.13

3.15

0.166

0.168

797

797

D- 1660

D- 1680

7112.3

7112.5

21

20

4:1

4:1

0.011

0.011

0.4

04

0.216

0.219

TABLE 5-5. CHANNEL CONVEYANCE CHARACTERISTICS FOR PMF WITH STEPHENSON ROCK SIZING (continued).

TABLE 0-0. OI	ANNEL CON	TETATOL (. •		TENO	on Noon c) OF1121C		· /·	Alternate
Cross Section	Bottom Elevation	Bottom Width	Side Slope	Energy Gradient	Discharge	Velocity	Area	Manning's	s Depth	Riprap D50	Rock D50	(por = 0.45) Riprap D50
	(Ft+MSL)	(ft)	(H:V)	(ft/ft)	(cfs)	(fps)	(sq. ft.		(ft)	(ft)	(ft)	(ft)
D- 1700	7112.7	20	4:1	0.011	797	7.7	103.3	0.035	3.16	0.168	0.4	0.040
D- 1800		20	4:1	0.009	797	7.7	114.1	0.035	3.16	0.168	0.4	0.219 0.184
D- 1850		20 :		0.008	797	6.9	116.1	0.035	3.44	0.129	0.4	0.167
D- 1900		20	4:1	0.004	819	5.1	160.4	0.035	4.31	0.071	0.4	0.092
D- 2000		20	4:1	0.002	863	4.2	208.2	0.035	5.14	0.041	0.4	0.053
D- 2100		20	4:1	0.001	720	3.2	222.6	0.035	5.37	0.020	0.4	0.027
D- 2200	7114.5	20	4:1	0.001	576			-LOW SLC	PE		0.4	
D- 2300		20	4:1	0.001	433			-LOW SLC	PE		0.4	
HC4-11(2400)	7114.8	- 20	4:1	0.001	289			-LOW SLO			0.4	
D- 2500		20	4:1	0.001	289			-LOW SLO			0.4	
D- 2600 D- 2700		20 20	4:1	0.001	289	2.5		-LOW SLO			0.4	0.040
D- 2900		20	4:1	0.003 0.014	289 289	3.5 6.2	82.1 46.5	0.035 0.035	2.68 1.73	0.032	0.4	0.042
D- 3000		20	4:1	0.014	289	6.5	44.6	0.035	1.73	0.118 0.132	0.4 0.4	0.154 0.172
D- 3100		20	4:1	0.026	289	7.7	37.7	0.035	1.46	0.200	0.4	0.172
D- 3110		20	4:1	0.026	277	7.6	36.7	0.035	1.43	0.196	0.4	0.255
D- 3130		20	4:1	0.025	253	7.2	35	0.035	1.37	0.178	0.4	0.231
D- 3150		20	4:1	0.025	229	7.1	32.5	0.035	1.29	0.169	0.4	0.220
D- 3170		20	4:1	0.026	206	6.8	30.1	0.035	1.21	0.162	0.4	0.211
D- 3190		20	4:1	0.024	182	6.4	28.2	0.035	1.15	0.141	0.4	0.184
D- 3210		20	4:1	0.024	158	6.1	25.8	0.035	1.06	0.130	0.4	0.169
D- 3230		20	4:1	0.025	134	5.9	22.8	0.035	0.96	0.123	0.4	0.160
HC4-10(3250)	7127.8	20	4:1	0.02	110	5.1	21.4	0.035	0.91	0.090	0.4	0.117
D- 3400		20	4:1	0.035	110	6.1	17.9	0.035	0.78	0.144	0.4	0.187
D- 3420 D- 3440		20 20	4 : 1 4 : 1	0.035 0.034	110	6.2	17.9	0.035	0.77	0.144	0.4	0.188
D- 3460		20	4:1	0.034	110 110	6.1 6.2	18.1 17.8	0.035 0.035	0.78 0.77	0.140 0.147	0.4 0.4	0.183 0.192
D- 3480		20	4:1	0.034	110	6.1	18	0.035	0.78	0.140	0.4	0.183
D- 3500		20	4:1	0.036	110	6.2	17.7	0.035	0.77	0.147	0.4	0.192
D- 3520		20	4:1	0.032	110	6	18.4	0.035	0.8	0.134	0:4	0.175
D- 3540	7135.7	20	4:1	0.038	110	6.3	17.4	0.035	0.76	0.155	0.4	0.201
D- 3560		20	4:1	0.029	110	5.8	19.2	0.035	0.82	0.123	0.4	0.160
D- 3580	7137.1	20	4:1	0.046	110	6.7	16.4	0.035	0.72	0.182	0.4	0.237
					05111							
H- 50	6963.7	40	4 . 4	0.01	Channel H	6.2	99.8	0.005	2.07	0.400		0.422
H- 100	6963.8	40	4 : 1 4 : 1	0.005	620 620	6.2 4.8	129.1	0.035 0.035	2.07 2.57	0.102 0.057	0 1.2	0.133 0.075
H- 157	6964.1	40	4 : 1	0.005	620	4.9	126.4	0.035	2.52	0.058	1.2	0.075
H- 165	6965.1	40	4:1	0.131	620	14.3	43.3	0.035	0.99	0.954	1.2	1.241
H- 175	6966.4	40	4 : 1	0.133	620	14.4	43	0.035	0.98	0.966	1.2	1.257
H- 183	6967.5	40	4 : 1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 193	6968.8	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 203	6970.1	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
HCT-3(213)	6971.4	40	4 : 1	0.131	620	14.4	43.2	0.035	0.98	0.951	1.2	1.239
H- 223	6972.7	40	4:1	0.132	620	14.4	43.1	0.035	0.98	0.959	1.2	1.248
H- 233	6974	40	4:1	0.142	620	14.7	42.1	0.035	0.96	1.031	1.2	1.342
H- 240	6975	40	4:1	0.141	620	14.7	42.2	0.035	0.96	1.024	1.2	1.333
H- 260	6977.8	40 40	4:1	0.142	620	14.7	42.2	0.035	0.96	1.031	1.2	1.342
H- 280	6980.7 6983.5	40 40	4:1	0.139 0.13	620 620	14.6	42.4	0.035	0.97	1.012	1.2	1.317
H- 300 H- 320	6985.5 6986.1	40 40	4 : 1 4 : 1	0.13	620 620	14.3 14.3	43.3 43.3	0.035 0.035	0.99 0.99	0.946 0.946	1.2 1.2	1.232 1.232
H- 340	6988.7	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 360	6991.3	40	4 : 1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 380	6993.9	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 400	6996.5	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 420	6999.1	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 440	7001.7	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 460	7004.3	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 480	7006.9	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 500	7009.5	40	4:1	0.132	620	14.4	43.1	0.035	0.98	0.959	1.2	1.248
H- 520	7012.1	40	4:1	0.133	620	14.4	43	0.035	0.98	0.966	1.2	1.257
H- 540	7014.8	40	4:1	0.132	620	14.4	43.1	0.035	0.98	0.959	1.2	1.248
H- 560	7017.5	40	4 : 1	0.132	620	14.4	43.1	0.035	0.98	0.959	1.2	1.248
H- 580	7020.1	40 40	4:1	0.133	620 620	14.4	43 43 1	0.035	0.98	0.966	1.2	1.257 1.248
H- 600 H- 620	7022.8 7025.4	40 40	4 : 1 4 : 1	0.132 0.132	620 620	14.4 14.4	43.1 43.1	0.035 0.035	0.98 0.98	0.959 0.959	1.2 1.2	1.248 1.248
FI- UZU	1020.4	70	₹ . I	J. 132	020	(T) T	70.1	0.038	J. 50	J. 3J3	1.2	1.470

ABLE 5-5. CHANNEL CONVEYANCE CHARACTERISTICS FOR PMF WITH STEPHENSON ROCK SIZING (continued). Alternate Target (por = 0.45) Riprap Cross **Bottom Bottom** Side Energy Flow Manning's Riprap Rock Width Gradient Discharge Velocity Area Depth D50 Section Elevation Slope D50 D50 (Ft+MSL) (ft) (H:V) (ft/ft) (cfs) (fps) (sq. ft.) (ft) (ft) (ft) (ft) H- 640 7028 0.133 620 1.257 40 4:1 14.4 43.1 0.035 0.98 0.966 1.2 H- 660 7030.7 40 4:1 0.132 620 14.4 43.1 0.035 0.98 0.959 1.2 1.248 4:1 H- 680 40 1.248 7033.4 0.132 620 14.4 43.1 0.035 98.0 0.959 1.2 4:1 0.13 14.3 1.2 1.232 H- 700 7036 40 620 43.3 0.035 0.99 0.946

H- 720	7038.6	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 740	7041.2	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 760	7043.8	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 780	7046.4	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 800	7049	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 820	7051.6	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 840	7054.2	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 860	7056.8	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0:946	1.2	1.232
H- 880	7059.4	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 900	7062	40	4:1	0.135	620	14.5	42.8	0.035	0.98	0.985	1.2	1.282
H- 920	7064.7	40	4:1	0.135	620	14.5	42.8	0.035	0.98	0.985	1.2	1.282
H- 940	7067.4	40	4:1	0.135	620	14.5	42.8	0.035	0.98	0.985	1.2	1.282
H- 960	7070.1	40	4:1	0.134	620	14.4	42.9	0.035	0.98	0.973	1.2	1.267
H- 980	7072.8	40	4:1	0.13	620	14.3	43.3	0.035	0.99	0.946	1.2	1.232
H- 1000	7075.5	40	4:1	0.109	620	13.5	45.9	0.035	1.04	0.794	1.2	1.034
H- 1010	7077	40	4:1	0.069	620	11.7	53.2	0.035	1.19	0.522	1.2	0.680
H- 1030	7078.5	40	4:1	0.026	620	8.5	72.9	0.035	1.57	0.224	1.2	0.292
HCT-2(1100)	7079.4	40	4:1	0.007	620	5.5	112.9	0.035	2.3	0.076	1.2	0.099
H- 1200	7079.6	40	4:1	0.004	620	4.4	141.4	0.035	2.77	0.048	1.2	0.062
H- 1220	7079.7	39	4:1	0.004	620	4.6	135.6	0.035	2.73	0.049	1.2	0.064
H- 1240	7079.8	38	4:1	0.004	620	4.8	130.5	0.035	2.7	0.050	1.2	0.065
H- 1260	7079.9	36	4:1	0.005	620	5	125.1	0.035	2.66	0.060	1.2	0.079
H- 1280	7080	35	4:1	0.005	620	5.1	121	0.035	2.65	0.061	1.2	0.080
H- 1282	7080	35	3.9 : 1	0.006	620	5.3	115.9	0.035	2.6	0.071	1.2	0.093
H- 1285	7080.1	34	3.9 : 1	0.007	620	5.7	109.5	0.035	2.51	0.083	1.2	0.108
H- 1288	7080.2	33	3.8 : 1	0.008	620	6.1	102.4	0.035	2.43	0.094	1.2	0.122
H- 1291	7080.2	32	3.7 : 1	0.01	620	6.5	95.7	0.035	2.34	0.114	1.2	0.148
H- 1294	7080.3	32	3.7 : 1	0.023	620	8.7	71.3	0.035	1.86	0.231	1.2	0.301
H- 1297	7080.3	31	3.6 : 1	0.021	620	8.4	73.3	0.035	1.95	0.216	1.2	0.282
H- 1300	7080.4	30	3.5 . 1	0.0195	620	8.25	73.3	0.035	2.03	0.207	1.2	0.269
H- 1303	7080.4	29	3.5 1	0.018	620	8.1	76.2	0.035	2.08	0.195	1.2	0.254
H- 1306	7080.4	29	3.4 : 1	0.018	620	8.2	75.7	0.035	2.12	0.199	1.2	0.259
H- 1309	7080.4	28	3.4 : 1	0.019	620	8.5	73.1	0.035	2.1	0.211	1.2	0.275
H- 1312	7080.5	27	3.3 : 1	0.019	620	8.5	72.8	0.035	2.14	0.214	1.2	0.279
H- 1315	7080.5	26	3.3 : 1	0.019	620	8.5	72.9	0.035	2.18	0.217	1.2	0.282
H- 1318	7080.5	26	3.3 : 1	0.016	620	8	77.2	0.035	2.33	0.190	1.2	0.247
H- 1321	7080.5	25	3.2 : 1	0.015	620	. 8	77.2	0.035	2.38	0.183	1.2	0.238
H- 1324	7080.5	24	3.2 : 1	0.016	620	8.3	74.6	0.035	2.36	0.196	1.2	0.255
H- 1327	7080.5	23	3.2 : 1	0.017	620	8.5	72.9	0.035	2:36	0.209	1.2	0.272
H- 1330	7080.5	23	3.1 : 1	0.019	620	8.8	70.7	0.035	2.37	0.235	1.2	0.305
H- 1333	7080.5	22	3.1 : 1	0.016	620	8.4	73.8	0.035	2.5	0.205	1.2	0.267
H- 1336	7080.5	21	3.1 : 1	0.025	620	9.8	63.3	0.035	2.26	0.305	1.2	0.396
H- 1339	7080.6	20	3:1	0.026	620	10	61.8	0.035	2.28	0.321	1.2	0.417
H- 1342	7080.6	20	3:1	0.028	620	10.4	59.8	0.035	2.27	0.348	1.2	0.454
H- 1345	7080.6	19	2.9 : 1	0.031	620	10.8	57.3	0.035	2.26	0.387	1.2	0.504
H- 1348	7080.6	18	2.9 : 1	0.031	620	10.9	56.8	0.035	2.3	0.394	1.2	0.513
H- 1351	7080.6	17	2.9 ; 1	0.034	620	11.3	54.7	0.035	2.29	0.434	1.2	0.565
H- 1354	7080.6	17	2.8 : 1	0.034	620	11.6	53.7	0.035	2.33	0.447	1.2	0.582
H- 1357	7080.6	16	2.8 : 1	0.036	620	11.8	52.6	0.035	2.35	0.477	1.2	0.620
H- 1360	7080.6	15	2.8 : 1	0.039	620	12.2	51	0.035	2.36	0.522	1.2	0.680
H- 1363	7080.7	14	2.7 : 1	0.039	620	12.3	50.4	0.035	2.42	0.534	1.2	0.695
H- 1366	7080.7	14	2.7 : 1	0.041	620	12.7	48.9	0.035	2.43	0.570	1.2	0.742
H- 1369	7080.7	13	2.6 : 1	0.042	620	12.9	48.1	0.035	2.5	0.599	1.2	0.780
H- 1372	7080.7	12	2.6 : 1	0.043	620	13.1	47.2	0.035	2.54	0.624	1.2	0.730
H- 1375	7080.7	11	2.6 : 1	0.044	620	13.3	46.6	0.035	2.59	0.651	1.2	0.812
H- 1378	7080.7	11	2.5 : 1	0.045	620	13.5	45.9	0.035	2.67	0.683	1.2	0.847
H- 1381	7080.7	10	2.5 : 1	0.045	620	13.6	45.5	0.035	2.74	0.699		
H- 1384	7080.8	99.	2.5 : 1	0.045	620	13.7		0.035	2:74	0.716	1.2	0.909
H- 1387	7080.8	8	2.4 : 1	0.045	620	13.7	44.8	0.035	2.92	0.710	1.2 1.2	0.932 0.963
H- 1390	7080.8	8	2.4 : 1	0.045	620	13.9	44.6	0.035	3.02	0.756	1.2	0.985
H- 1393	7080.8	7	2.3 : 1	0.043	620	13.9	44.5	0.035	3.16	0.750	1.2	0.965
H- 1396	7080.8	6	2.3 : 1	0.043	620	13.9	44.7	0.035	3.29	0.750	1.2	1.004
		-				0	,	0.000	0.20	V.111	1.2	1.004

TABLE 5-5. CHANNEL CONVEYANCE CHARACTERISTICS FOR PMF WITH STEPHENSON ROCK SIZING (continued).

Cross Section	Bottom Elevation (Ft+MSL)	Bottom Width (ft)	Side Slope (H : V)	Energy Gradient (ft/ft)	Discharge (cfs)	Velocity (fps)	Flow Area (sq. ft.)	Manning n	's Depth (ft)	Riprap D50 (ft)	Target Rock D50 (ft)	Afternate (por = 0.45) Riprap D50 (ft)
H- 1399	7080.8	5	2.3 : 1	0.041	620	13.7	45.1	0.035	3.43	0.754	1.2	0.982
H- 1402	7080.8	5	2.2 : 1	0.039	620	13.7	45.4	0.035	3.63	0.752	1.2	0.978
H- 1405	7080.8	4	2.2 : 1	0.037	620	13.4	46.2	0.035	3.8	0.731	1.2	0.952
H- 1408	7080.9	3	2.2 : 1	0.035	620	13.1	47.4	0.035	4.01	0.713	1.2	0.928
H- 1411 H- 1414	7080.9 7080.9	2 2	2.1 : 1 2.1 : 1	0.032 0.028	620 620	12.8	48.5 50.7	0.035	4.29	0.682	1.2	0.888
H- 1417	7080.9	1	2.1 . 1	0.028	620	12.2 11.5	50.7 54	0.035 0.035	4.57 5	0.618 0.538	1.2 1.2	0.804 0.700
HCT-1(1470)	7080.9	ò	2:1	0.023	620	6.9	89.7	0.035	6.7	0.336	1.2	0.208
H- 1490	7081	Ö	2:1	0.005	620	6.3	98.2	0.035	7.01	0.135	1.2	0.205
H- 1500	7081	5	2:1	0.002	620	4.2	147.9	0.035	7.44	0.052	1.2	0.068
H- 1501	7081	5	2:1	0.002	620	4.1	151.5	0.035	7.46	0.051	1.2	0.067
H- 1504	7081	6	2.1 : 1	0.001	620	3.7	166.2	0.035	7.5	0.028	1.2	0.037
H- 1507	7081	7	2.1 : 1	0.001	620	3.5	178.1	0.035	7.61	0.027	1.2	0.036
H- 1510	7081	9	2.2 : 1	0.001	620	3.3	187	0.035	7.49	0.026	1.2	0.034
H- 1513	7081	10	2.3 : 1	0.001	620				PONDED		1.2	
H- 1516	7081	11	2.3 : 1	0.001	620				PONDED		1.2	
H- 1519	7081	12	2.4 : 1	0.001	620				PONDED		1.2	
H- 1522	7081	13	2.4 : 1	0.001	620				PONDED		1.2	
H- 1525 H- 1528	7081 7081	14 15	2.5 : 1	0.001	620				PONDED		1.2	
H- 1531	7081	16	2.6 : 1 2.6 : 1	0.001 0.001	620 620				PONDED		1.2 1.2	
H- 1534	7081	17	2.7 : 1	0.001	620				PONDED		1.2	
H- 1537	7081	18	2.7 : 1	0.001	620				PONDED		1.2	
H- 1540	7081	19	2.8 : 1	0.001	620				PONDED		1.2	
H- 1543	7081	20	2.9 : 1	0.001	620				PONDED		1.2	
H- 1546	7081	21	2.9 : 1	0.001	620				PONDED		1.2	
H- 1549	7081	22	3:1	0.001	620				PONDED	t	1.2	
H- 1552	7081	23	3:1	0.001	620				PONDED	1	1.2	
H- 1555	7081.1	24	3.1 : 1	0.001	620				PONDED		1.2	
H- 1558	7081.1	25	3.2 : 1	0.001	620				PONDED		1.2	
H- 1561	7081.1	26	3.2 : 1	0.001	620				PONDED		1.2	
H- 1564	7081.1	27	3.3 : 1	0.001	620				PONDED		1.2	
H- 1567	7081.1	28	3.3 : 1	0.001	620				PONDED		1.2	
H- 1570 H- 1573	7081.1 7081.1	30 31	3.4 : 1 3.5 : 1	0.001 0.001	620 620				PONDED		1.2 1.2	
H- 1576	7081.1	32	3.5 : 1	0.001	620				PONDED		1.2	
H- 1579	7081.1	33	3.6 : 1	0.001	620				PONDED		1.2	
H- 1582	7081.1	34	3.6 : 1	0.001	620				PONDED			
H- 1585	7081.1	35	3.7 : 1	0.001	620				PONDED			
H- 1588	7081.1	36	3.8 : 1	0.001	620				PONDED			
H- 1591	7081.1	37	3.8 : 1	0.001	620				PONDED			
H- 1594	7081.1	38	3.9 : 1	0.001	620 ·				PONDED			
H- 1597	7081.1	39	3.9 : 1	0.001	620			٠.	PONDED			
H- 1600	7081.1	40	4:1	0.001	620				PONDED		,	
H- 1700	7081.2	40	4 : 1	0.001	620				PONDED			
					Channel I					1.		
1- 1450	7081	50	4:1	0.007	581	5.1	113.2	0.035	1.96	0.000	0,15	0.085
1- 1490	7081.3	50	4:1	0.007	581	5.2	112.2	0.035	1.94	0.065	0.15	0.085
I- 1590	7082.1	50	4:1	0.008	581	5.3	108.9	0.035	1.89	0.065	0.15	0.094
1- 1630	7082.4	50	4:1	0.008	581	5.3	109.3	0.035	1.9	0.072	0.15	0.095
i- 1730	7083.2	50	4:1	0.008	581	5.3	109.8	0.035	1.9	0.073	0.15	0.095
I- 1770	7083.8	50	4:1	0.015	581	6.6	87.8	0.035	1.56	0.073	0.15	0.158
I- 1870	7085.3	50	4:1	0.033	581 504	8.5	68.6	0.035	1.25	0.121	0.6	0.305
i- 1890	7086 7086 6	50 50	4:1	0.033	581 581	8.4	68.8	0.035	1.25	0.234	0.6	0.302
1- 1910 1 1930	7086.6	50 60	4:1	0.032	581	8.4	69.2	0.035	1.26	0.232	0.6	0.296
J- 1930	7087.3	50 50	4:1	0.034	581 581	8.6	68 68 2	0.035	1.24	0.228	0.6	0.313
I- 1940 I- 1960	7087.6 7088.3	50 50	4 : 1 4 : 1	0.034 0.034	581 581	8.5 8.5	68.2 68.3	0.035 0.035	1.24 1.24	0.240 0.239	0.6 0.6	0.311 0.311
1- 1960 HC4-3(1980)	7088.3	50 50	4:1	0.034	581 581	8.5 8.4	68.8	0.035	1.24 1.25	0.239	0.6 0.6	0.311
1- 1995	7089.5	50 50	4:1	0.033	581	8.4	69.2	0.035	1.25	0.232	0.6	0.302
I- 2015	7099.3	50	4 : 1	0.032	581	8.3	70.2	0.035	1.27	0.228	0.6	0.288
1- 2015	7090.1	50	4:1	0.033	581	8.5	68.6	0.035	1.25	0.221	0.6	0.305
I- 2055	7090.5	50	4:1	0.033	581	8.3	70.1	0.035	1.27	0.234	0.6	0.288
I- 2075	7092.1	50	4:1	0.034	581	8.5	68	0.035	1.24	0.221	0.6	0.311
											0.6	0.302
i- 2075 i- 2090	7092.1 7092.6	50 50	4:1	0.034	581 581	8.5 8.4	68.9	0.035	1.24 1.25	0.221		

Cross Section	Bottom Elevation (Ft+MSL)	Bottom Width (ft)	Side Siope (H : V)	Energy Gradient (ft/ft)	Discharge (cfs)	Velocity (fps)		Manning's n	s Depth (ft)	Riprap D50 (ft)	Target Rock D50 (ft)	Alternate (por = 0.45 Riprap D50 (ft)
		<u> </u>				<u></u>				<u>-</u>		
I- 211		50	4:1	0.034	581	8.5	68.2	0.035	1.24	0.232	0.6	0.311
I- 213		50	4:1	0.033	581	8.4	69	0.035	1.26	0.239	0.6	0.304
I- 215		50 50	4:1	0.035	581	8.6	67.B	0.035	1.23	0.233	0.6	0.319
I- 217 I- 227		50 50	4 : 1 4 : 1	0.02 0.036	581 541	7.2 8.5	80.3	0.035	1.44	0.245	0.6	0.200
1- 239		49	4:1	0.030	496	5.6	63.7 88.8	0.035 0.035	1.17 1.6	0.153 0.241	0.6 0.4	0.313 0.112
I- 241		48	4:1	0.012	491	5.8	85.3	0.035	1.57	0.086	0.4	0.112
I- 243		47	4:1	0.012	486	5.9	83	0.035	1.55	0.094	0.4	0.122
I- 245		46	4:1	0.012	482	5.9	81.9	0.035	1.56	0.094	0.4	0.123
1- 247		46	4:1	0.012	477	5.8	81.6	0.035	1.58	0.094	0.4	0.122
I- 249		45	4:1	0.012	473	5.9	80.7	0.035	1.59	0.094	0.4	0.124
I- 251		44	4:1	0.012	468	5.9	79.7	0.035	1.6	0.095	0.4	0.125
I- 253		43	4:1	0.012	464	5.9	78.2	0.035	1,59	0.096	0.4	0.124
I- 255	0 7102.8	42	4:1	0.013	459	6	76.8	0.035	1.6	0.095	0.4	0.134
1- 257	7103.1	41	4:1	0.012	455	6	76.3	0.035	1.61	0.103	0.4	0.127
1- 259		40	4:1	0.012	450	5.9	75.6	0.035	1.63	0.097	0.4	0.126
1- 261	4 7103.7	40	4:1	0.014	445	6.2	71.9	0.035	1.55	0.097	0.4	0.143
I- 271		40	4:1	0.015	422	6.2	67.8	0.035	1.48	0.110	0.4	0.146
HC4-2(2810)	7106.7	40	4:1	0.014	399	5.9	67.1	0.035	1.46	0.112	0.4	0.133
I- 281		40	4 . 1	0.012	399	5.7	70.6	0.035	1.53	0.102	0.4	0.118
i- 283		40	4 ; 1	0.007	399	4.8	82.9	0.035	1.76	0.091	0.4	0.076
I- 293		40	4:1	0.005	399	4.1	96.7	0.035	2.01	0.058	0.4	0.057
1- 303		40	4:1	0.004	399	4	98.9	0.035	2.05	0.044	0.4	0.048
I- 304		39	4:1	0.004	399	4.1	97.7	0.035	2.06	0.037	0.4	0.049
1- 306		38	4:1	0.005	399	4.2	94.7	0.035	2.07	0.037	0.4	0.059
I- 308		36	4:1	0.005	399	4.4	91.3	0.035	2.06	0.046	0.4	0.061
I- 310		34	4:1	0.005	399	4.5	88.8	0.035	2.08	0.047	0.4	0.062
I- 312		33	4:1	0.006	399	4.6	86.4	0.035	2.1	0.048	0.4	0.073
I- 314 I- 316		31 30	4:1	0.006	399	4.7	84.6	0.035	2.13	0.056	0.4	0.075
I- 318		28	4:1	0.006	399	4.8	82.3	0.035	2.15	0.058	0.4	0.077
I- 320		26 26	4 : 1 4 : 1	0.006 0.007	399 399	4.9	80.7	0.035	2.19	0.059	0.4	0.079
1- 322		25	4 : 1	0.007	399	5,1 5.2	78.7 77.1	0.035 0.035	2.23 2.28	0.060 0.071	0.4 0.4	0.092 0.095
1- 324		23	. 4 1	0.007	399	5.3	76	0.035	2.26	0.071	0.4	0.098
I- 326		22	4:1	0.007	399	5.4	74.2	0.035	2.38	0.075	0.4	0.090
HC4-1(3280)	7108.6	. 20	4:1	0.008	399	5.5	72.1	0.035	2.43	0.077	0.4	0.114
Ì- 329		21	4:1	0.016	399	7.1	56.5	0.035	1.94	0.088	0.4	0.202
I- 331		24	4:1	0.016	399	7	57.1	0.035	1.81	0.155	0.4	0.191
1- 333(7109.7	27	4:1	0.017	399	6.9	57.6	0.035	1.7	0.147	0.4	0.190
1- 336	7110.3	29	4:1	0.01	399	5.7	69.8	0.035	1.89	0.146	0.4	0.118
I- 338	7110.4	28	4:1	0.009	399	5.5	72.3	0.035	2.01	0.091	0.4	0.110
1- 3400		27	4:1	0.009	399	5.5	72.6	0.035	2.07	0.085	0.4	0.113
I- 3420		25	4:1	0.009	399	5.6	71.7	0.035	2.12	0.087	0.4	0.116
1- 3440		24	4:1	0.009	399	5.6	71.1	0.035	2.17	0.089	0.4	0.118
I- 3460		23	4:1	0.009	399	5.8	69.4	0.035	2.2	0.090	0.4	0.122
1- 3480		21	4:1	0.009	399	5.8	68.6	0.035	2.26	0.093	0.4	0.124
I- 3500	7111.1	20	4:1	0.009	399	5.9	67.7	0.035	2.32	0.095	0.4	0.127
					Channel M							
M- 200	7082.5	50	8:1	0.005	170	2.8	59.6	0.035	1.02	0.000	0.6	0.028
M- 270	7083	50	8 : 1	0.007	170	3.2	53.3	0.035	0.93	0.022	0.6	0.038
IC4-6(295)	7084	50	6.4 : 1	0.035	170	5.4	31.8	0.035	0.59	0.029	0.6	0.143
M- 395	7088	50	8:1	0.046	170	5.8	29.5	0.035	0.54	0.110	0.6	0.178
M- 495	7094.1	50	8:1	0.09	150	6.8	22	0.035	0.41	0.137	0.6	0.295
M- 605	7099.9	50	8:1	0.001	36	1	36	0.035	0.65	0.226	0.6	0.003
M- 612	7099.9	43	7.1 : 1	0.002	36	1.2	31	0.035	0.65	0.002	0.6	0.006
M- 618	7099.9	36	5.1 : 1	0.002	36	1.4	25.5	0.035	0.65	0.004	0.6	0.006
M- 625	7100	30	7.4 : 1	0.003	36	1.6	22	0.035	0.64	0.005	0.6	0.010
M- 633	7100	20	4:1	0.008	36	2.6	13.9	0.035	0.62	0.007	0.6	0.028
M- 641	7100	10	4:1	0.079	36	6.5	5.5	0.035	0.47	0.021	0.6	0.279
M- 645	7100 7100	5 0	4:1	0.083	36	7.4	4.8 7.4	0.035	0.64	0.214	0.6	0.390
	/3/10	. []	4:1	0.023	36	4.9	:7	0.035	1.36	0.300	0.6	0.166
M- 650 M- 670	7100	Õ	2.5 : 1	0.023	36	5.3	6.8	0.035	1.64	0.128	0.6	0.199

0.006

0.005

C4-5(705) M- 730

7100.1 7100.2

0

0.6

0.6

0.6

0.199

0.063

0.054

0.128 0.153

0.146

0.048

0.035

0.035

0.035

1.85

2.33

2.43

6.8 6.9

10.8

11.8

5.2

3.3

3.1

TABLE 5-5. CHANNEL CONVEYANCE CHARACTERISTICS FOR PMF WITH STEPHENSON ROCK SIZING (continued).

Cross Section	Bottom Elevation (Ft+MSL)	Bottom Width (ft)	Side Slope (H : V)	Energy Gradient (ft/ft)	Discharge (cfs)		Flow Area (sq. ft.)	Manning's n	Depth	Riprap D50 (ft)	Target Rock D50 (ft)	(por = 0.45 Riprap D50 (ft)
M- 755	7100.2	25	4:1	0.001	36			SURGE			- 0.6	
M- 199	7 100.2	23	7 . 1	0.001					FOIND -	•	- 0.0	
N 400	7070.0	20	4 . 4	0.040	Channel N	7.6	20.2	0.035	0.9	0.225		0.277
N- 100	7076.2 7076.6	30 30	4 : 1 4 : 1	0.042 0.048	229 229	7.6 7.9	30.2 28.9	0.035	0.86	0.252	· <u>-</u>	0.309
N- 110 IC5-5(130)	7070.6	30	4:1	0.046	229	6.5	35.5	0.035	1.04	0.232	0.6	0.309
N- 140	7078.3	30	4:1	0.024	229	6.3	36.2	0.035	1.06	0.141	0.6	0.172
N- 190	7079	30	4:1	0.025	229	6.4	35.7	0.035	1.04	0.146	0.6	0.178
N- 200	7079.3	30	4 : 1	0.019	213	5.7	37.4	0.035	1.09	0.11	0.6	0.136
N- 220	7080	30	4:1	0.055	180	7.6	23.7	0.035	0.72	0.242	0.6	0.300
N- 230	7080.5	28	4:1	0.055	180	7.7	23.4	0.035	0.75	0.248	0.6	0.311
N- 240	7081	27	4:1	0.054	180	7.8	23.1	0.035	0.78	0.254	0.6	0.317
N- 250	7081.5	25	4:1	0.045	180	7.4	24.2	0.035	0.85	0.225	0.6	0.278
N- 270	7082.4	25	4:1	0.046	180	7.5	24	0.035	0.85	0.229	-0.6	0.285
N- 290	7083.3	25	4:1	0.044	180	7.4	24.3	0.035	0.86	0.221	0.6	0.275
N- 310	7084.2	25	4:1	0.046	180	7.5	23.9	0.035	0.84	0.231	0.6	0.283
N- 330	7085.1	25 25	4:1	0.038	180	7.1	25.5	0.035	0.89	0.196	0.6	0.242
N- 350	7086	25 25	4:1	0.083	180	9.1	19.8	0.035	0.71	0.385	0.6	0.480
N- 365	7087.2	25 25	4:1	0.077	180	8.9	20.3	0.035	0.73	0.361	0.6	0.451
C5-4(385) N- 395	7088.9 7090	25 25	4:1	0.105 0.092	180 180	9.9 9.4	18.3 19.1	0.035 0.035	0.66 0.69	0.483 0.426	0.6 0.6	0.600 0.528
	7090.2	25 20	4 : 1 4 : 1	0.092	180	9.4 10.6	19.1	0.035	0.69	0.426	0.6	0.526
N- 400 N- 402	7090.2	19	3.9 : 1	0.109	180	10.8	16.7	0.035	0.74	0.594	1.2	0.729
N- 402 N- 404	7090.4	18	3.8 : 1	0.109	180	10.9	16.7	0.035	0.79	0.613	1.2	0.747
N- 406	7090.7	17	3.7 : 1	0.103	180	11	16.3	0.035	0.82	0.623	1.2	0.757
N- 408	7090.8	16	3.6 : 1	0.105	180	11,1	16.2	0.035	0.85	0.633	1.2	0.766
N- 410	7091	15	3.5 : 1	0.099	180	11.1	16.2	0.035	0.9	0.622	1.2	0.753
N- 412	7091.1	14	3.4 : 1	0.102	180	11.4	15.8	0.035	0.92	0.668	1.2	0.800
N- 414	7091.2	13	3.3 : 1	0.104	180	11.7	15.4	0.035	0.95	0.715	1.2	0.847
N- 416	7091.3	12	3.2 : 1	0.106	180	12	15	0.035	0.99	0.761	1.2	0.901
N- 418	7091.4	11	3.1 : 1	0.107	180	12.3	14.7	0.035	1.03	0.803	1.2	0.949
N- 420	7091.5	10	3:1	0.105	180	12.4	14.5	0.035	1.09	0.837	1.2	0.974
N- 430	7092	5 .	3:1	0.092	180	12.7	14.1	0.035	1.49	1.003	1.2	1.078
N- 440	7092.5	0	3:1	0.065	180	11.4	15.7	0.035	2.29	1.051	1.2	0.981
N- 445	7092.8	0	2.5 : 1	0.047	180	10.5	17.2	0.035	2.62	0.826	1.2	0.770
C5-3(475)	7093	0	2:1	0.006	180	5.2	34.9	0.035	4.18	0.145	1.2	0.126
N- 500	7093.1	0	2:1	0:005	180	4.8	37.4	0.035	4.32	0.122	1.2	0.106
N- 503	7093.1	3	2.2 : 1	0.002	180	3	59.4	0.035	4.58	0.034	1.2	0.039
N- 508	7093.1	8	2.6 : 1	0.001	180	•		W SLOPE W SLOPE			1.2 1.2	
N- 513	7093.1 7093.1	13 17	3 : 1 3.2 : 1	0.001 0.001	180 180	•	LC	W SLOPE			0.15	
N- 515	7093.1	29	3.6 : 1	0.001	180			W SLOPE			0.15	
N- 520 N- 525	7093.1	40	4:1	0.001	180			W SLOPE			0.15	
N- 600	7093.2	40	4:1	0.001	450			W SLOPE			0.15	
N- 680	7094.1	40	4:1	0.001	650	3.2	202.1	0.035	3.69	0.023	0.15	0.021
N- 780	7094.3	40	4:1	0.003	899	4.6	194.4	0.035	3.58	0.052	0.15	0.061
C5-2(880)	7094.5	40	4:1	0.005	1149	5.8	198.6	0.035	3.64	0.086	0.15	0.107
N- 885	7094.5	40	4.1 : 1	0.004	1149	5.7	202.6	0.035	3.66	0.083	0.15	0.089
N- 890	7094.5	41	4.2 : 1	0.004	1149	5.5	208.7	0.035	3.71	0.077	0.15	0.088
N- 895	7094.5	41	4.3 : 1	0.004	1149	5.4	211.4	0.035	3.72	0.075	0.15	0.087
N- 900	7094.6	41	4.4 : 1	0.004	1149	5.3	216.3	0.035	3.74	0.072	0.15	0.086
N- 920	7094.6	43	4.8 : 1	0.003	1149	5	231.2	0.035	3.81	0.062	0.15	0.067
N- 940	7094.7	44	5.1 : 1	0.003	1149	4.7	245.3	0.035	3.86	0.055	0.15	0.065
N- 960	7094.8	45	5.5 : 1	0.003	1149	4.4	258.9	0.035	3.9	0.049	0.15	0.063
N- 980	7094.8	46	5.9 : 1	0.002	1149	4.3	270.4	0.035	3.9	0.045	0.15	0.045
N- 1000	7094.9	48	6.3 : 1	0.002	1149	4.1	282.9	0.035	3.92	0.041	0.15	0.044
N- 1020	7094.9	49	6.6 : 1	0.002	1149	4	290.8	0.035	3.9	0.039	0.15	0.043
N- 1040	7095	50	7:1	0.002	1149	3.8	305.3	0.035	3.94	0.035	0.15	0.042 0.041
N- 1060	7095.1	51 52	7.4 1	0.002	1149	3.7	315.1	0.035	3.93	0.033 0.032	0.15 0.15	0.041
N- 1080	7095.1	53 54	7.8 : 1	0.002	1149	3.6	319	0.035 0.035	3.86	0.032	0.15 0.15	0.040
N- 1100	7095.2	54 55	8.1 : 1	0.002	1149	3.5 3.4	328.5 339.5	0.035	3.87 3.87	0.03	0.15	0.039
N- 1120	7095.3 7095.3	55 56	8.5 : 1 80 · 1	0.002	1149 1149	3.4 3.3	343.2	0.035	· 3.81	0.028	0.15	0.037
N- 1140	7095.3	56 58	8.9 : 1	0.002	1149 1149	3.3	343.2 354.8	0.035	3.82	0.026	0.15	0.037
N- 1160 N- 1180	7095.4 7095.4	58 59	9.3 : 1 9.6 : 1	0.002 0.002	1149	3.2 3.2	357.6	0.035	3.77	0.026	0.15	0.036

TABLE 5-5. CHANNEL CONVEYANCE CHARACTERISTICS FOR PMF WITH STEPHENSON ROCK SIZING (continued).

Cross Section	Bottom Elevation (Ft+MSL)	Bottom Width (ft)	Side Slope (H : V)	Energy Gradient (ft/ft)	Discharge (cfs)	Velocity (fps)	Flow Area (sq. ft.)	Manning's n	Depth (ft)	Riprap D50 (ft)	Target Rock D50 (ft)	Alternate (por = 0.45) Riprap D50 (ft)
N- 1300	7095.7	60	10 : 1	0.002	1149	3.2	361.2	0.035	3.72	0.025	0.15	0.036
N- 1400	7095.8	60	10 : 1	0.002	1149	3.2	361.2	0.035	3.72	0.025	0.15	0.036
N- 1500	7096	60	10 : 1	0.002	1149	3.2	361.2	0.035	3.72	0.025	0.15	0.036
N- 1600	7096.2	60	10 : 1	0.002	1149	3.3	352.9	0.035	3.66	0.027	0.15	0.036
N- 1950	7096.7	60	10 : 1	0.002	1149	3.2	361.2	0.035	3.72	0.025	0.15	0.036
N- 2150	7096.9	60	10 : 1	0.001	1149	3.1	370.1	0.035	3.78	0.024	0.15	0.021
HC5-1(2250)	7097	60	10 : 1	0.001	1149	3	380.1	0.035	3.86	0.022	0.15	0.020
N- 2300 N- 2400	7097.1	60	10 : 1	0.001	1149	3	380.1	0.035	3.86	0.022	0.15	0.020
N- 2500	7097.2 7097.3	60 60	10 : 1 10 : 1	0.001 0.001	1149 1149	3 3	380.1	0.035	3.86	0.022	0.15	0.020
N- 2600	7097.5	60	10 : 1	0.001	1149	3	380.1 380.1	0.035 0.035	3.86 3.86	0.022 0.022	0.15	0.020
N- 2700	7097.6	60	10 : 1	0.001	1149	,3	380.1	0.035	3.86	0.022	0.15 0.15	0.020 0.020
N- 2800	7097.7	60	10 : 1	0.001	1149	,3 3	380.1	0.035	3.86	0.022	0.15	0.020
N- 2900	7097.9	60	10 : 1	0.001	1149	3	380.1	0.035	3.86	0.022	0.15	0.020
N- 3000	7098	60	10 : 1	0.001	1149	3	380.1	0.035	3.86	0.022	0.15	0.020
,				•	Channel O			•				
HC5-9(55)	7097	20	4:1	0.013	583	7.3	79.5	0.035	2.61	0.000	0.4	0.212
O- 150	7098.7	20	4:1	0.012	583	7.3	79.9	0.035	2.62	0.163	0.4	0.200
O- 250	7100	20	4 : 1	0.014	583	7.6	76.5	0.035	2.54	0.153	0.4	0.227
0-310	7100.4	20	4:1	0.006	583	5.7	102.1	0.035	3.14	0.133	0.4	0.221
0-410	7101	20	4:1	0.006	583	5.7 5.7	102.1	0.035	3.14	0.175	0.4	0.111
0-411	7101	21	5.3 : 1	0.004	575	4.4	131.2	0.035	3.41	0.085	0.4	0.072
0- 414	7101	22	5.3 : 1	0.003	550	4.4	138.5	0.035	3.44	0.055	0.4	0.072
O- 417	7101	24	5.2 : 1	0.003	526	3.6	146.3	0.035	3.51	0.033	0.4	0.037
O- 420	7101	25	3.9 : 1	0.002	501	3.7	134.9	0.035	3.49	0.029	0.4	0.037
O- 460	7101.3	25	3.9 : 1	0.002	501	3.9	127.1	0.035	3.34	0.029	0.4	0.052
O- 465	7101.3	25	3.9 : 1	0.003	501	3.9	127.1	0.035	3.34	0.029	0.4	0.052
0- 470	7101.3	25	3.9 : 1	0.003	501	4	125.4	0.035	3.31	0.040	0.4	0.052
0- 475	7101.3	25	3.9 : 1	0.003	501	4	125.4	0.035	3.31	0.040	0.4	0.053
O- 480	7101.4	25	3.9 1	0.003	501	4.1	123.8	0.035	3.28	0.040	0.4	0.053
O- 485	7101.4	25	3.9 : 1	0.003	501	4.1	123.8	0.035	3.28	0.041	0.4	0.053
O- 490	7101.4	25	4:1	0.003	501	4.1	122.8	0.035	3.24	0.041	0.4	0.053
O- 495	7101.4	25	4:1	0.003	501	4.1	122.8	0.035	3.24	0.040	0.4	0.053
O- 500	7101.5	25	4:1	0.003	501	4.1	121.3	0.035	3.21	0.040	0.4	0.052
O- 505	7101.5	25	4:1	0.003	501	4.1	121.3	0.035	3.21	0.040	0.4	0.052
HC5-6(510)	7101.5	25	4 1	0.003	501	4.1	121.3	0.035	3.21	0.040	0.4	0.052
0-610	7102	25	4:1	0.004	501	4.6	109.6	0.035	2.97	0.040	0.4	0.067
0- 620	7102	25	4 1	0.004	501	4.6	109.6	0.035	2.97	0.052	0.4	0.067
0- 720	7102.5	25	4 : 1	0.004	501	4.7	105.9	0.035	2.9	0.052	0.4	0.067
O- 820	7103	25	4:1	0.005	501	4.8	104.3	0.035	2.86	0.052	0.4	0.080
O- 830	7103	25	4:1	0.005	501	4.8	104.3	0.035	2.86	0.062	0.4	0.080
O- 930	7103.5	25	4:1	0.005	501	4.9	102.8		2.83	0.062	0.4	0.081
O- 1030	7104	25	4:1	0.005	501	4.9	102.8		2.83	0.062	0.4	0.081
O- 1130 O- 1230	7104.5 7105	25 25	4 : 1 4 : 1	0.005 0.005	501 501	4.9 4.9	102.8 102		2.83 2.81	0.062 0.062	0.4 0.4	0.081 0.081
0 1200	. ,00	20	7 . '		Channel P	4,0	102	0.000	2.01	0.002	0.4	0.001
P- 10	7101.3	20	4:1	0.005	446	4.9	90.4	D. 035	2.87	0.000	0.4	0.082
P- 30	7102	20	4:1	0.001	173	2.2	79.5		2.61	0.063	0.4	0.002
HC5-8(50)	7102.5	20	4:1	0.003	173	3	58.7		2.07	0.010	0.4	0.032
P- 70	7103	20	4 . 1	0.007	173	4.2	41.2		1.57	0.024	0.4	0.064
P- 110	7104	20	4:1	0.022	173	6.1	28.4		1.15	0.049	0.4	0.166
P- 200	7105.5	20	4:1	0.01	173	4.7	36.7		1.43	0.128	0.4	0.086
P- 240	7106	20	4:1	0.013	173	5.1	33.8		1.33	0.066	0.4	0.107
P- 300	7106.5	20	4:1	0.007	173	4.3	40.7		1.55	0.082	0.4	0.065
IC5-7(370)	7107	20	4:1	0.007	173	4.2	41.4		1.58	0.050	0.4	0.064
P- 450	7107.5	20	4:1	0.006	173	4			1.63	0.050	D.4	0.056
P- 540	7108	20	4:1	0.006	173	3.8	45		1.68	0.043	0.15	0.056
P- 600	7108.5	20	4:1	0.008	173	4.3	40.3		1.54	0.043	0.15	0.072
P- 670	7109	20		0.008	173	4.2			1:57	0.055	0.15	0.064
	7109.5	20	4:1	0.006	173	3.8	45.3		1.69	0.049	0.15	0.056
P- 760	/1DM 2								1.09	U.U48	0.13	0.000

TABLE 5-5. CHANNEL CONVEYANCE CHARACTERISTICS FOR PMF WITH STEPHENSON ROCK SIZING (continued).

Cross Section	s	Bottom Elevation (Ft+MSL)	Bottom Width (ft)	Side Slope (H : V)	Energy	Discharge	Velocity	Flow	Manning's	·	Riprap D50 (ft)	•	Alternate (por = 0.45) Riprap D50 (ft)
						06							
Q- 10	nn	7060.9	30	4:1	0.011	Channel Q 843	7.3	114.9	0.035	2.79	0.163	0.6	0.194
Q- 13		7061.3	30	4 : 1	0.012	843	7.6	111	0.035	2.72	0.103	0.6 0.6	0.194
Q- 22		7062.4	30	4:1	0.012	843	7.7	109.7	0.035	2.69	0.182	0.6	0.211
HC5-14(230)		7062.5	30	4:1	0.024	843	9.7	86.9	0.035	2.23	0.318	0.6	0.378
Q- 25	50	7063	30	4:1	0.021	843	9.3	91.2	0.035	2.32	0.283	0.6	0.338
Q- 26	30	7063.2	30	4:1	0.02	833	9.1	92	0.035	2.34	0.27	0.6	0.323
Q- 27		7063.5	30	4:1	0.015	823	8.2	100.2	0.035	2.5	0.214	0.6	0.250
Q- 28		7063.7	30	4:1	0.022	813	9.3	87.8	0.035	2.25	0.286	0.6	0.344
Q- 29		7064	30	4:1	0.021	803	9.1	88	0.035	2.26	0.276	0.6	0.328
Q- 30		7064.2	30	4 : 1	0.02	792	8.9	88.5	0.035	2.27	0.264	0.6	0.311
Q- 31	10	7064.5	30	4:1	0.015	782	8.1	96.7	0.035	2.43	0.207	0.6	0.243
Q- 32		7064.7	30	4:1	0.021	772	9.1	84.9	0.035	2.19	0.276	0.6	0.321
Q- 33		7065	30 30	4 : 1 4 : 1	0.029	761 751	10.1	75.5	0.035	1.99	0.355	0.6	0.419
Q- 34 Q- 36		7065.2 7066.1	30	4 : 1 4 : 1 .	0.046 0.045	751 751	11.7 11.6	64.2 64.5	0.035 0.035	1.74 1.74	0.517 0.511	0.6	0.619 0.604
Q- 38		7067	30	4:1	0.045	751 751	11.6	64.8	0.035	1.75	0.506	0.6 0.6	0.607
Q- 40		7068	30	4:1	0.043	751	11.4	65.7	0.035	1.77	0.488	0.6	0.582
Q- 42		7068.9	30	4:1	0.039	751	11.1	67.9	0.035	1.82	0.448	0.6	0.537
Q- 44		7069.8	30	4 : 1	0.025	751	9.5	78.8	0.035	2.06	0.311	0.6	0.365
HC5-12(500)		7071.3	30	4:1	0.025	751	9.5	79.2	0.035	2.07	0.307	0.6	0.366
`Q- 54	Ю	7072.3	30	4:1	0.025	751	9.6	78.4	0.035	2.05	0.314	0.6	0,366
Q- 59	90	7073.6	30	4:1	0.024	751	9.4	80	0.035	2.09	0.299	0.6	0.354
Q- 69		7076.1	30	4:1	0.026	751	9.7	77.6	0.035	2.04	0.322	0.6	0.380
Q- 79		7078.6	30	4:1	0.023	751	9.3	81.1	0.035	2.11	0.289	0.6	0.342
Q- 87		7080.6	30	4:1	0.027	751	9.8	76.5	0.035	2.01	0.334	0.6	0.390
Q- 97		7083.1	30	4:1	0.021	751	8.9	84.1	0.035	2.17	0.265	0.6	0.314
Q- 98		7083.4	30	4:1	0.016	747	8.2	91.3	0.035	2.32	0.215	0.6	0.250
Q- 10		7085.9 7087.3	30 30	4:1	0.093	706 706	14.5	48.7	0.035	1.37	0.93	1.2	1.125
Q- 10 Q- 11		7087.3 7089.2	30	4 : 1 4 : 1	0.093 0.092	706. 706	14.5 14.4	48.8 48.9	0.035 0.035	1.37 1.38	0.925 0.919	1.2 1.2	1.125 1.114
Q- 11 Q- 11		7091	30	4:1	0.032	706	14.3	49.5	0.035	1.39	0.89	1.2	1.081
Q- 11	55	7092.9	30	4:1	0.079	706	13.8	51.3	0.035	1.44	0.803	1.2	0,971
Q- 11		7094.8	30	4 1	0.053	706	12	58.8	0.035	1.61	0.559	1.2	0.674
Q- 11	80	7094.8	30	4:1	0.009	706	6.4	109.7	0.035	2.69	0.122	1.2	0.148
Q- 11	95	7094.9	10	4:1	0.046	706	13	54.3	0.035	2.64	0.813	1.2	0.877
Q- 12		7094.9	. 0	4:1	0.033	706	11.8	59.7	0.035	3.86	0.864	1.2	0.804
Q- 12		7094.9	0	3.7 : 1	0.03	706	11.6	60.8	0.035	4.05	0.818	1.2	0.759
Q- 12		7095	0	3 : 1	0.015	706	9.4	75.5	0.035	5.02	0.47	1.2	0.436
Q- 12		7095	0	2.8 : 1	0.02	706	10.4	67.6	0.035	4.91	0.616	1.2	0.578
Q- 12		7095	0	2.5 : 1	0.018	706	10.3	68.7	0.035	5.24	0.59	1.2	0.551
Q- 121		7095	0	2.4 : 1	0.022	706	11.2	63.2	0.035	5.13	0.726	1.2	0.675
Q- 12	60	7095 7095	0	2.2 : 1	0.02 0.012	706	11.1 9.3	63.8	0.035	5.39	0.711	1.2	0.642
HC5-11(1295) Q- 132		7095	Ö	2:1	0.012	706 706	9.3 7.4	75.8 95.5	0.035 0.035	6.16 6.91	0.469 0.267	1.2 1.2	0.415 0.252
Q- 13		7095	6	2.4 : 1	0.001	706		187.4	0.035	7.68	0.044	0.4	0.232
Q- 13		7095	12	2.8 : 1	0.001	706	0.0		ONDING		0.044	0.4	0.000
Q- 133		7095	18	3.2 : 1	0.001	706	_		ONDING			0.4	
Q- 134		7095	24	3.6 : 1	0.001	706	_		ONDING -			0.4	
Q- 134	45	7095	30	4:1	0.001	706	_	Р	ONDING			0.4	
Q- 134	48	7095.3	30	4:1	0.001	700	-		ONDING			0.4	
Q- 139	52	7095.8	30	4:1	0.001	650	_	——-P(ONDING			0.4	
Q- 13	56	7096.3	30	4:1	0.001	600	-	P	ONDING			0.4	
Q- 136	60	7096.7	30	4:1	0.001	550	-	P	ONDING			0.4	
Q- 136		7097.2	30	4:1	0.001	500	-	——P(ONDING			0.4	
Q- 136		7097.6	30	4:1	0.001	450	-		ONDING			0.4	
Q- 137		7098.1	30	4:1	0.001	400	-		ONDING			0.4	
Q- 137		7098.5	30	4:1	0.08	480	12.1	39.6		1.15	0.635	1.2	0.774
Q- 138		7099	30 30	4:1	0.078	460	11.8	39		1.13	0.597	1.2	0.736
Q- 138		7099.5	30 30	4:1	0.074	440 430	11.4	38.4		1.12	0.558	1.2	0.682
Q- 138 Q- 139		7099.9 7100.4	30 30	4 : 1 4 : 1	0.07 0.063	420 400	11 10.5	38.1		1,11 1,11	0.511 0.452	1.2 1.2	0.631 0.558
Q- 139		7100.4	30	4 : 1	0.003	300		38.2 27.1		0.82	0.452	1.2	0.721
Q- 140		7100.3	30	4:1	0.101	290	11	26.4	0.035	0.8	0.574	1.2	0.718
Q- 140		7101.8	30	4:1	0.106	280	10.9	25.6		0.77	0.575	1.2	0.715
Q- 140		7102.2	30	4:1	0.112	270	11	24.6		0.75	0.591	1.2	0.744

TABLE 5-5. CHANNEL CONVEYANCE CHARACTERISTICS FOR PMF WITH STEPHENSON ROCK SIZING (continued).

Cross Section	Bottom Elevation (Ft+MSL)	Bottom Width (ft)	Side Slope (H : V)	Energy Gradient (ft/ft)	Discharge (cfs)	Velocity (fps)		Manning's n	Depth (ft)	Riprap D50 (ft)	Target Rock D50 (ft)	Alternate (por = 0.45) Riprap D50 (ft)
		······			· · · · · · · · · · · · · · · · · · ·		· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·		·····		
Q- 1412	7102.7	30	4:1	0.122	260	11.2	23.3	0.035	0.71	0.63	1.2	0.788
Q- 1416	7103.1	30	4:1	0.146	250	11.6	21.5	0.035	0.66	0.732	1.2	0.918
Q- 1420	7103.6	30	4:1	0.2	243	12.7	19.1	0.035	0.59	1.008	1.2	1.267
Q- 1427	7105	30	4:1	0.19	243	12.5	19.4	0.035	0.6	0.953	1.2	1.197
Q- 1434	7106.5	30	4:1	0.159	243	11.9	20.5	0.035	0.63	0.789	1.2	0.988
Q- 1444	7108.1	30	4:1	0.144	243	11.5	21.2	0.035	0.65	0.709	1.2	0.891
Q- 1454	7109.8	30	4:1	0.047	243	8	30.3	0.035	0.9	0.255	1.2	0.315
Q- 1455	7109.8	29	4:1	0.053	243	8.4	28.9	0.035	0.88	0.29	1.2	0.355
Q- 1475	7109.9	10	4:1	0.072	243	11.4	21.3	0.035	1.38	0.676	1.2	0.766
Q- 1485	7110	0	4:1	0.03	243	8.7	27.9	0.035	2.64	0.502	1.2	0.471
Q- 1490	7110	0	3.6 : 1	0.027	243	8.6	28.2	0.035	2.8	0.483	1.2	0.446
Q- 1499	7110	0	3:1	0.017	243	7.5	32.2	0.035	3.28	0.342	1.2	0.312
Q- 1500	7110	0	3:1	0.016	243	7.4	33	0.035	3.32	0.322	1.2	0.297
Q- 1510	7110	0	2.7 : 1	0.019	243	7.9	30.7	0.035	3.37	0.383	1.2	0.359
Q- 1530	7110.1	0	2.2 : 1	0.022	243	8.7	27.9	0.035	3.56	0.491	1.2	0.447
HC5-10(1543)	7110.1	1	2.2 : 1	0.004	243	4.7	51.8	0.035	4.59	0.101	1.2	0.091
Q- 1548	7110.1	3	2.4 : 1	0.002	243	3.4	70.9	0.035	4.81	0.044	0.4	0.044
Q- 1553	7110.1	5	2.7 : 1	0.001	243	2.7	89.1	0.035	4.88	0.024	0.4	0.022
Q- 1558	7110.1	7	2.9 : 1	0.001	243			ONDING —			0.4	
Q- 1563	7110.1	9	3.2 : 1	0.001	243		——Р	ONDING —			0.4	
Q- 1568	7110.1	11	3.5 : 1	0.001	243		P	ONDING -			0.4	
Q- 1573	7110.1	13	3.7 : 1	0.001	243		P	ONDING				
Q- 1578	7110.1	15	4:1	0.001	243		<u></u> Р	ONDING				
Q- 1590	7110.2	30	4:1	0.001	243		——Р	ONDING				

	Channet Station	Elev.	Base	Channel			Discharge (cfs)	HEC-2 Velocity (fps)	Area (ft*2)		Top Width (ft)		Hydraulic Radius (ft)	Calc- ulated Average Sheer (Ib/II^2)	Equivalent Slope For Average Shear (ff/ft)			Maximum Shear (ib/ft^2)	Calc- ulated Slope For Rock Sizing (ff/ft)	Unit	Unit	Average Unit Discharge Stephenson Rock D50 (f1)	Maximum Unit Discharge Stephenson Rock D50 (ft)	Rock Type+	Target Riprap 050
												•	HANNEL A	A											
HC4-9	90 200 220 240 260 290 400 500 600 700 800	7126.4 7128.8 7129.4 7130 7131.2 7132.4 7135.2 7135.9 7136.5 7137.1 7137.7	20 20 20 20 20 20 20 20 20 20 20	4 4 4 4 4 4	0.022 0.026 0.030 0.045 0.060 0.042 0.015 0.006 0.006 0.006	0.039 0.031 0.021 0.058 0.052 0.080 0.012 0.005 0.006 0.005	180 170 165 160 153 147 120 110 100 90	7.5 6.8 6 8.3 7.9 8.9 4.4 3.2 3.3 3	24 25 27.6 19.4 19.5 16.5 27.4 34.5 30.6 29.7 27.6	1 1.03 1.13 0.83 0.84 0.72 1.12 1.36 1.23 1.13	28 28.24 29.04 26.64 26.72 25.76 28.96 30.88 29.84 29.6 29.04	28.25 28.49 29.32 26.84 26.93 25.94 29.24 31.21 30.14 29.90 29.32	0.85 0.87 0.95 0.72 0.73 0.64 7 0.94 1.11 1.02 1.00 0.95	1.104 1.359 1.838 2.151 2.731 1.502 0.877 0.453 0.323 0.331 0.306	0.0208 0.0250 0.0312 0.0478 0.0601 0.0379 0.0150 0.0065 0.0051 0.0053	NONE NONE NONE NONE NONE NONE NONE NONE	1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000	1.157 1.410 1.769 2.151 2.728 1.651 0.888 0.450 0.381 0.373 0.354	0.022 0.026 0.030 0.048 0.060 0.042 0.015 0.006 0.006 0.006	6.43 6.02 5.68 6.01 5.73 5.71 4.14 3.56 3.35 3.04 2.75	7.50 7.00 6.78 6.89 6.64 6.41 4.93 4.35 4.06 3.60 3.28	0.12 0.13 0.14 0.22 0.26 0.19 0.07 0.03 0.03 0.03	0.13 0.15 0.16 0.24 0.28 0.20 0.07 0.04 0.03 0.03	SML SML SML SML SML SML SML SML SML SML	0.40 0.40 0.40 0.40 0.40 0.40 0.40 0.40
												•	CHANNEL	D											
HC4-13	150 200 215 220 240 260 320 340 380 400 420 440 460 500 550 560 560 700 800 900 1100	7083.5 7085 6 7085 6 7085.1 7087.1 7088.1 7088.1 7099.2 7091.8 7092.5 7093.3 7094.1 7095.4 7095.4 7096.7 7097.3 7097.3 7097.3 7097.3 7099.2 7099.2 7102.9 7102.9 7102.5 7102.1	555555555555555555555555555555555555555	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0.030 0.042 0.055 0.055 0.053 0.053 0.053 0.040 0.038 0.040 0.035 0.032 0.032 0.033 0.032 0.033 0.032 0.030	0.025 0.052 0.050 0.050 0.048 0.045 0.038 0.037 0.037 0.035 0.031 0.031 0.031 0.031 0.031 0.031 0.031 0.031 0.031	1325 1300 1297 1285 1274 1262 1250 1240 1230 1210 1210 1210 1190 1190 1150 1110 1110 1110 1100 1000 900 875 850		- LOW : - LOW : - LOW :	SLOPE- SLOPE- SLOPE-	ROCK S ROCK S ROCK S	IZING MET IZING MET IZING MET	1.89 1.56 1.56 1.56 1.56 1.59 1.64 1.64 1.65 1.71 1.69 1.67 1.67 1.67 1.67 1.65 1.65 1.65 1.65 1.65 1.65 1.65 1.65		0.0368 0.0485 0.0537 0.0551 0.0489 0.0468 0.0468 0.0468 0.0362 0.0370 0.0325 0.0313 0.0312 0.0313 0.0315 0.0295 0.0313 0.0318 0.0306 0.0308	NONE	1.000 1.000	3.540 5.049 5.631 5.368 4.860 5.124 5.202 4.636 4.101 3.825 4.121 3.727 3.427 3.347 3.348 3.334 3.334 3.334 3.332 3.348 3.332 3.447	0.030 0.055 0.055 0.055 0.053 0.053 0.045 0.040 0.038 0.040 0.032 0.032 0.032 0.031 0.033 0.032 0.032 0.031	19 62 20 29 20 22 20 22 20 21 19 66 19 60 19 89 18 99 18 99 18 99 18 90 18 90 18 77 77 17 64 17 77 17 64 17 77 17 64 17 77 17 64 17 77 17 64 17 77 17 64 17 77 17 64	22.56 22.86 22.66 22.56 22.30 22.14 21.51 21.51 21.32 21.20 21.06 20.87 20.87 20.66 19.95 19.85 19.85 19.10	0.33 0.57 0.55 0.50 0.52 0.52 0.45 0.41 0.38 0.40 0.33 0.33 0.33 0.33 0.33 0.33 0.32 0.32 0.32 0.32	0.36 0.57 0.57 0.59 0.55 0.57 0.56 0.49 0.41 0.43 0.39 0.36 0.36 0.36 0.35 0.35 0.35	LRGG LRGG LRGG LRGG LRGG LRGG LRGG LRGG	1.20 1.20 1.20 1.20 1.20 1.20 1.20 1.20
HC4-12	1200 1220 1240 1260 1280 1300 1320 1340 1360	7107 7107.2 7107.3 7107.5 7107.6 7108.6 7108.3 7108.3 7109.3 7109.5 7110.4 7110.4 7111.9 7111.2 7111.2 7112.1 7112.1 7112.3	50 48 44 42 40 39 36 37 36 32 29 27 26 21 20 20 20	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0.010 0.007 0.007 0.007 0.008 0.007 0.013 0.013 0.013 0.013 0.013 0.013 0.015 0.013 0.015 0.011 0.011 0.011 0.011 0.010 0.010 0.010	0.001 0.001 0.001 0.001 0.002 0.002 0.003 0.004 0.005 0.010 0.016 0.016 0.015 0.018 0.011 0.011 0.011	797 797 797 797 797 797 797 797 797 797	2.9 3.1 3.3 3.7 4 4.8 5.4 6 6.9 7.7 7.6 8.8 8.8 8.8 9.7.5 7.6 7.7 7.7 7.6 9.9	- LOW	SLOPE-	ROCK S	izing MET 12ing MET 82.04 78.80 75.97 72.24 69.43 66.78 64.05 55.61 52.20 48.19 48.19 44.61 47.65 46.65 46.65 46.65 46.04	HOOS FAIL 308 326 317 301 286 272 257 241 227 210 196 1.94 1.94 1.94 1.94 2.04 1.94 2.25 2.23 2.24 2.23 2.24		0.0911 0.0020 0.0010 0.0008 0.0037 0.0060 0.0071 0.0060 0.0071 0.0163 0.0163 0.0158 0.0150 0.0150 0.0110 0.0114 0.0113	500 500 500 500 500 500 500 500 500 500	1,000 1,000 1,247 1,223 1,200 1,170 1,148 1,126 1,102 1,078 1,055 1,000	1,974 1,865 1,781 2,199 2,584 2,207 2,027 1,865 1,680 1,593 1,861 1,938 1,861 1,938 1,533 1,533 1,533 1,533 1,533		9.84 10.25 10.63 11.63 12.09 12.60 13.18 13.77 14.50 15.45 16.33 16.37 17.30 17.30 17.31 17.63 17.63 17.63 17.63 17.68 16.89	12.67 13.08 13.60 14.76 15.36 15.74 16.52 17.16 18.08 18.87 20.21 20.66 21.33 22.09 22.60 23.58 23.33 23.64 23.74	0.08 0.08 0.08 0.01 0.13 0.13 0.13 0.13 0.14 0.19 0.17 0.20 0.17 0.20 0.13 0.13 0.14 0.19		SML SML SML SML SML SML SML SML SML SML	0.40 0.40 0.40 0.40 0.40 0.40 0.40 0.40

(Revised 05/20/96)

	Channel Station		Base Width	Channel Side	Slope		Discharge (cfs)	HEC-2 Velocity (fps)	Area (ff^2)				Hydraulic Radius IfO	Calc- ulated Average Shear (lb/Tl^2)	Equivalent Slope For Average Shear (R/R)				Calc- ulated Slope For Rock Sizing	Unit	Unit	Average Unit Discharge Stephenson Rock D50	Maximum Unit Discharge Stephenson Rock 050	Rock Type+	Target Riprap D50 (ft)
												c	HANNEL	,		**					1	•			
HC4-11	2000 2100 2200 2300 2400 2500 2600 2700 2900 3100 3110 3150 3150 3170 3190	7114.1 7114.3 7114.4 7114.5 7114.8 7114.9 7115.8 7116.6 7116.6 71121.2 7123 7123 7124 7124 7125.9	20 20 20 20 20 20 20 20 20 20 20 20 20 2	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0.002 0.001 0.001 0.002 0.001 0.005 0.009 0.012 0.015 0.016 0.024 0.032 0.033 0.030	0.004 0.002 0.001 0.001 0.001 0.001 0.001 0.003 0.014 0.016 0.026 0.025 0.025 0.025	819 863 720 576 433 289 289 289 289 289 289 289 277 253 229 206 182	3.5 6.2 6.5 7.7 6.7.2 7.1 6.8	LOW S	SLOPE-F SLOPE-F SLOPE-F	ROCK SIZ ROCK SIZ ROCK SIZ	ZING METH		2.048 1.216 -4.192 2.099 1.099 1.350 1.739 1.894 1.863 1.600 1.539	0.0114 0.0058 -0.0194 	NONE NONE NONE NONE NONE NONE NONE NONE	1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000	0.721 0.417 0.218 0.000 0.000 	0.004 0.002 0.001 	15.03 14.12 11.44 	21.98 21.59 17.18 	0.06 0.03 0.02 	0.07 0.04 0.02 - - - - 0.09 0.13 0.14 0.20 0.23 0.22 0.20 0.18	SML SML SML SML SML SML SML SML SML SML	0.40 0.40 0.40 0.40 0.40 0.40 0.40 0.40
HC4-10	3230 3250 3400 3420 3440 3460 3480 3500 3520 3540 3560	7126.5 7127.2 7127.8 7130.8 7131.5 7132.2 7132.9 7133.8 7134.3 7135 7136.7 7136.4 7137.1	20 20 20 20 20 20 20 20 20 20 20	4 4 4 4 4 4 4 4 4 4	0.033 0.032 0.025 0.028 0.035 0.035 0.035 0.035 0.035 0.035	0.024 0.025 0.025 0.035 0.035 0.034 0.036 0.034 0.036 0.038 0.032 0.038	158 134 110 110 110 110 110 110 110 110 110	6.1 5.9 5.1 6.2 6.1 6.2 6.1 6.2 6.3 5.8 6.7	25.8 22.8 21.4 17.9 17.9 18.1 17.8 18.1 17.7 18.4 17.4 19.2 16.4	1.06 0.96 0.91 0.78 0.77 0.78 0.77 0.78 0.77 0.8 0.76 0.82 0.72	28.48 27.68 27.28 26.24 26.16 26.16 26.24 26.16 26.4 26.4 26.08 28.56 25.76	28.74 27.92 27.50 26.43 26.35 26.43 28.35 26.43 26.35 26.60 26.27 26.76 25.94	0.89 0.82 0.78 0.68 0.67 0.68 0.67 0.67 0.70 0.67 0.71	1.442 1.261 1.171 1.171 1.483 1.495 1.474 1.486 1.470 1.517 1.431 1.593 1.504	0.0258 0.0246 0.0240 0.0275 0.0352 0.0351 0.0349 0.0349 0.0348 0.0344 0.0356 0.0379	250 250 270 300 350 400 500 600 600 600 600	1.046 1.032 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000	1.897 1.715 1.220 1.171 1.473 1.490 1.515 1.524 1.581 1.581 1.504	0.034 0.034 0.025 0.028 0.035 0.035 0.036 0.035 0.035 0.035 0.035	5.55 4.84 4.03 4.19 4.20 4.19 4.20 4.17 4.20 4.17 4.22 4.14 4.27	6.47 5.66 4.64 4.76 4.77 4.78 4.77 4.80 4.79 4.76 4.79 4.76 4.79	0.16 0.14 0.10 0.11 0.13 0.14 0.13 0.14 0.13 0.14 0.13	0.17 0.16 0.11 0.12 0.14 0.15 0.14 0.15 0.14 0.15 0.14	SML SML SML SML SML SML SML SML SML SML	0.40 0.40 0.40 0.40 0.40 0.40 0.40 0.40
			40										HANNEL												
нст-з	100 157 165 175 183 203 213 223 240 260 300 320 320 340 360 400 440 460 460 460 460 460 660 660	6963.7 6963.7 6964.1 6965.1 6966.5 6966.8 6970.1 6971.4 6972.7 6972.7 6972.7 6980.7 6980.7 6980.7 6980.7 6980.7 6993.9 6990.5 6993.9 6990.5 7001.7 7004.3 7009.5 7012.1 7022.8 7025.4 7025.4 7033.4 7038	40 40 40 40 40 40 40 40 40 40 40 40 40 4	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4	0.002 0.004 0.005 0.127 0.134 0.130 0.130 0.130 0.130 0.130 0.130 0.135 0.141 0.143 0.135 0.130	0.010 0.005 0.131 0.133 0.130 0.130 0.130 0.131 0.132 0.142 0.141 0.142 0.139 0.130	620 620 620 620 620 620 620 620 620 620	62 4.8 4.9 14.3 14.3 14.3 14.3 14.7 14.7 14.7 14.6 14.3 14.3 14.3 14.3 14.3 14.3 14.3 14.3	99.8 129.1 1	1.92 2.43 0.93 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92	55.38 59.44 47.36	55 83 60 04 47.67 47.59 47.59 47.59 47.59 47.59 47.59 47.42 47.42 47.42 47.42 47.59	1.64 2.01 0.85 0.84 0.84 0.84 0.84 0.83 0.83 0.83 0.83 0.83 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84	0.817 -0.638 12.417 19.442 7.475 7.398 7.320 7.436 7.419 7.5840 7.776 7.775 7.663 7.448 7.320	0.0080 -0.0051 -0.0051 0.2333 0.3590 0.1419 0.1404 0.13891 0.1518 0.1506 0.1506 0.1506 0.1506 0.1508 0.1389	25.55.55.55.55.55.55.55.55.55.55.55.55.5	1.000 1.000	0.817 0.456 0.457 6.903 7.048 6.850 6.850 6.903 6.956 7.303 7.358 7.1358 7.1358 6.850	0.008 0.005 0.131 0.134 0.134 0.130 0.130 0.131 0.142 0.141 0.143 0.135 0.130	11.20 10.43 13.07 13.09 13.09 13.09 13.09 13.09 13.14 13.14 13.14 13.14 13.14 13.19 13.09	11,90 11,56 13,16 13,16 13,16 13,16 13,16 13,25 13,23 13,23 13,23 13,16	0.08 0.04 0.48 0.91 0.92 0.92 0.90 0.91 0.91 0.98 0.99 0.99 0.90 0.90 0.90 0.90 0.90	0.08 0.04 0.24 0.91 0.93 0.93 0.90 0.91 0.99 0.99 0.99 0.99 0.90 0.90	LRG LRG LRG LRG LRG LRG LRG LRG LRG LRG	1.20 1.20 1.20 1.20 1.20 1.20 1.20 1.20

HEC.2

(II/II)

0.130

(cfs)

HFC-2

14.3

Side Slope Gradient Discharge Velocity Area Depth Width Perimeter Radius

(fps) (ft^2)

43.3

Flow

(ft)

0.92 47 36

Top

(81)

(ft)

47 50

Race Channel Bottom Energy

Slope

(17/11)

0.130

Calc-

ulated

Shear

(lb/ft*2)

7.320

Wetted Hydraulic Average

(11)

CHANNEL H

0.84

Equivalent

Average

Short

0.1380

(0/0)

Sione For Channel Channel

Bend

Radine

(fft)

NONE

Calc-

ulated

Sione

For Rock

Sizing

(17/11)

0.130

Unit

(cfs/ft)

13 09

Rand Maximum

Strage

Ratio

1.000

Sheer

(Ib/ft^2)

6.850

Average

11..........

Stanhanson

m

0.90

Average Maximum Discharge

Unit

Discharge Discharge Rock D50

(cfs/ft)

13 16

Maylmum

Unit

Discharge

Stephensor

(61)

0.90

Target

Riprap

D50

(0)

Bock

Type+

IRG

Channel

Cross Channel Base

Section Station Flav

720

Channel Channel

(fit+msi) (fit)

7038.6

Width



Channel Cross Section Name	Channel Station (ft)			Channel Side Slope		HEC-2 Energy Gradient D (fl/ft)	ischarge (cfs)	HEC-2 Velocity (fps)		Flow Depth (ft)	Top Width (ft)	Wetted Perimeter (ft)	Hydraulic Radius (ft)	ulated	Equivalent Slope For Average Shear (fl/fl)	Channel Bend Radius (ft)		Maximum Shear ((b/n^2)	Calc- ulated Slope For Rock Sizing (ft/ft)	Unit	Maximum Unit Discharge (cfs/fl)	Average Unit Discharge Stephenson Rock D50 (ft)	Maximum Unit Discharge Staphenson Rock 950 (ft)	Rock Type+	Target Riprap D50 (f1)
												•	CHANNEL I	4 ·		4				•	4 L	1			
НСТ-1	1470 1490 1501 1504 1507 1510 1513 1516 1519 1522 1525 1534 1534 1540 1543 1546 1552 1558 1561 1564 157 1573 1576 1573 1576 1573 1576 1573 1576 1573 1588 1591 1594 1594 1594 1594 1594 1594 1594	7081 7081 7081 7081 7081 7081 7081 7081	0 5 5 6 7 9 10 11 12 13 14 15 16 16 17 18 20 21 22 22 23 24 25 26 27 28 30 31 32 33 34 40 40 40 40 40 40 40 40 40 40 40 40 40	2 2 2 2 1 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2	0.001 0.000	0.006 0.005 0.002 0.002 0.001	620 620 620 620 620 620 620 620 620 620		LOW S	LOPE F. LOPE F	ROCK SI, ROCK SI, ROC	ZING METT	2.88 3.01 3.70 3.70 3.71 3.82 3.95 4.14 400S FAIL		0.0181 0.0121 0.0178 0.0024 0.0159 0.0236 -0.9394	22222222222222222222222222222222222222	1.000 1.000	1 078 0 938 0 463 0 463 0 239 0 247 0 000	0.006	24.07 23.07 18.59 18.56 17.23 16.50 	44.44 42.34 29.78 29.11 26.42 25.34 24.12	0.10 0.09 0.04 0.02 0.02 0.00 	0 16 0.13 0.05 0.03 0.03 0.00 	LEG GERGE GERG GERGE GERGE GERGE GERG GERGE GERGE GERGE GERGE GERGE GERGE GERGE GERGE GERGE GERG	1.20 1.20 1.20 1.20 1.20 1.20 1.20 1.20
	1450	7081	50	4	0.007	0.007	581	5.1	113.2	1.96	65.68	66.16	CHANNEL 1.71	0.785	0.0073	NONE	1.000	0.802	0.007	8.85	10.00	0,06	0.07	RM	0.15
HC4-3	1490 1590	7081.3 7082.1 7082.4 7083.2 7083.8 7086.7 7086.6 7087.3 7086.7 7087.3 7089.7 7089.1 7090.8 7090.1 7091.5 7092.1 7092.1 7092.3 7094.7 7094.7 7095.3 7098.7 7101.2 7101.2	50 50 50 50 50 50 50 50 50 50 50 50 50 5	444444444444444444444444444444444444444	0.008 0.008 0.008 0.011 0.025 0.032 0.033 0.035 0.034 0.032 0.035 0.032 0.035 0.033 0.035 0.033 0.035 0.035 0.031 0.035	0.007 0.008 0.008 0.008 0.013 0.033 0.033 0.034 0.034 0.034 0.031 0.031 0.031 0.033 0.032 0.033 0.033 0.033 0.033 0.034	581 581 581 581 581 581 581 581 581 581	5533565446555884625689	112.2 108.9 109.8 87.8 68.6 68.2 68.2 68.3 69.2 70.2 68.6 69.2 70.1 68 68.2 68.3 69.2 70.1 68 68.3 68.3 68.3 69.2 70.1 68.6 88.3 68.3 68.3 68.3 68.3 68.6 88.3 68.6 88.3 68.3 6	1.94 1.89 1.9 1.56 1.25 1.26 1.24 1.24 1.24 1.25 1.27 1.27 1.27 1.24 1.25 1.27 1.27 1.24 1.25 1.27 1.27 1.25 1.27 1.25 1.27 1.25 1.27 1.25 1.27 1.25 1.27 1.25 1.27 1.25 1.27 1.25 1.27 1.25 1.27 1.27 1.25 1.27 1.27 1.27 1.25 1.26 1.27 1.27 1.27 1.27 1.27 1.27 1.27 1.27	65.52 65.12 65.2 65.2 62.48 60 60.08 59.92 59.92 60.08 60.16 59.92 60.08 60.16 59.92 60.08 60.16 59.92 60.08 60.16 59.92 60.08 60.56 59.92 60.59 60.59	66.00 65.59 65.67 65.67 60.31 60.31 60.23 60.23 60.23 60.31 60.37 60.47 60.47 60.31 60.47 60.31 60.47 60.47 60.59 60.59 60.59 60.59	1.70 1.66 1.67 1.67 1.14 1.14 1.15 1.13 1.13 1.13 1.16 1.16 1.16 1.13 1.15 1.16 1.13 1.13 1.13	0.791 0.791 0.812 1.099 1.269 1.835 2.287 2.390 2.298 2.238 2.450 2.423 2.257 2.423 2.257 2.423 2.378 2.483 2.378 2.489 2.341 2.108 2.458 1.642 1.713 1.680 1.680	0.0075 0.0076 0.0076 0.0106 0.0106 0.0258 0.0324 0.0326 0.0317 0.0341 0.0316 0.0329 0.0329 0.0323 0.0327 0.0327 0.0327 0.0300 0.0323 0.0327 0.0325 0.0327	NONE NONE NONE NONE NONE 900 800 500 500 500 500 500 500 500 500 5	1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.000 1.074 1.075 1.074 1.075 1.073 1.074 1.075 1.073 1.075 1.073 1.075	0.821 0.828 0.832 1.196 1.305 2.347 2.329 2.401 2.401 2.471 2.610 2.523 2.574 2.523 2.574 2.523 2.574 2.523 2.574 2.523 2.674 2.523 1.829 1.740 1.829 1.740 1.829 1.740 1.829 1.940 1.	0.008 0.008 0.008 0.011 0.026 0.032 0.034 0.034 0.035 0.037 0.035 0.035 0.035 0.035 0.035 0.036 0.035 0.036 0.035	8.87 8.91 8.91 9.68 9.68 9.67 9.70 9.70 9.68 9.66 9.66 9.66 9.70 9.67 9.70 9.87 9.71 9.87 9.87 9.87 9.87 9.87 9.87 9.87 9.87	10.09 10.07 10.07 10.07 10.65 10.55 10.55 10.54 10.55 10.54 10.55	0.07 0.07 0.09 0.11 0.18 0.22 0.23 0.23 0.23 0.23 0.23 0.23 0.23	0.07 0.07 0.07 0.10 0.19 0.23 0.24 0.24 0.25 0.24 0.24 0.25 0.26 0.25 0.25 0.25 0.25 0.25 0.25 0.25	RMM RMM RMI STILL	0.15 0.15 0.15 0.15 0.60 0.60 0.60 0.60 0.60 0.60 0.60 0.6

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	Channel Base Station Elev. (fi) (fi+ms)	Width	Channel Side Slope			Discharge (cfs)	HEC-2 Velocity ([ps)		Flow Depth	Top Width (ft)		Hydraulic Radius (ft)	Calc- ulated Average Shear (lb/ft^2)	Equivalent Slope For Average Shear (ft/ft)	Channel Bend Radius (R)			Calc- ulated Slope For Rock Sizing (f/ft)	Unit	Unit	Average Unit Discharge Stephenson Rock D50 (ft)	Maximum Unit Discharge Stephenson Rock D50 (ft)	Rock Typ#+	Target Riprap D50 (ft)
											c	HANNEL	N						.=.					
	200 7079.3	30	4	0.032	0.019	213	5.7	37.4	1.09	38.72	38.99	0.96	1.742	0.0291	NONE	1.000	1.948	0.032	5.50	6.21	0.15	0.16	iNT	0.60
	220 7080 230 7080.5	30	4	0.043	0.055	180	7.6	23.7	0.72	35.76	35.94	0.66	1.812	0.0441	NONE	1.000	1.812	0.044	5.03	5.47	0.18	0.19	INT	0.60
	240 7080.5	28 27	4	0.050 0.050	0.055 0.054	180 180	7.7 7.8	23.4 23.1	0.75 0.78	34 33,24	34.18 33.43	0.68 0.70	2.353 2.233	0.0555 0.0509	NONE	1.000 1.000	2.334 2.368	0.055 0.054	5.29 5.42	5.78 6.08	0.23 0.23	0.24 0.24	INT	0.60 0.60
	250 7081.5	25	4	0.047	0.045	180	7.4	24.2	0.85	31.8	32.01	0.75	2.249	0.047B	NONE	1.000	2.235	0.047	5.66	6.29	0.21	0.22	INT	0.60
	270 7082.4 290 7083.3	25 25	4	0.045 0.045	0.046 0.044	180 180	7.5 7.4	24 24.3	0.85 0.86	31.8 31.88	32.01 32.09	0.75 0.76	2.115 2.112	0.0449 0.0444	NONE	1,000	2.165 2.140	0.046 0.045	5.66 5.65	6.38 6.36	0.20 0.20	0.22 0.22	INT	0.60 0.60
	310 7084.2	25	4	0.045	0.046	180	7.5	23.9	0.84	31.72	31.93	0.75	2.062	0.0443	NONE	1.000	2.142	0.046	5.67	6.30	0.20	0.22	INT	0.60
	330 7085.1 350 7086	25 25	4	0.045 0.063	0.038 0.083	180 180	7.1 9.1	25.5 19.8	0.89 0.71	32.12 30.68	32.34 30.85	0.79 0.64	2.446 2.830	0.0499 0.0708	NONE	1.000 1.000	2.207 3.318	0.045 0.083	5.60 5.87	6.32 6.46	0.20 0.35	0.21 0.37	INT	0.60 0.60
	365 7087.2	25	4	0.082	0.077	180	8.9	20.3	0.73	30.84	31.02	0.66	3.533	0.0862	NONE	1.000	3.383	0.082	5.84	6.50	0.34	0.37	INT	0.60
HC5-4	385 7088.9 395 7090	25 25	4	0.097 0.075	0.105 0.092	180 180	9.9 9.4	18.3 19.1	0.66 0.69	30.28 30.52	30.44 30.69	0.60 0.62	3.763 3.596	0.1006 0.0923	NONE	1.000 1.000	3.926 3.583	0.105 0.092	5.94 5.90	6.53 6.49	0.43 0.38	0.46 0.41	INT	0.60 0.60
	400 7090.2	20	4	0.070	0.109	180	10.6	17	0.74	25.92	26.10	0.65	5.607	0.1381	NONE	1.000	4.427	0.109	6.94	7.84	0.50	0.54	INT	0.60
	402 7090.4 404 7090.5	19 18	3.9 3.8	0.075 0.075	0.11 0.109	180 180	10.8 10.9	16.7 16.5	0.76 0.79	24.93 24	25.12 24.21	0.66 0.69	4.540 4.453	0.1095 0.1041	NONE	1.000 1.000	4.561 4.662	0.110 0.109	7.22 7.50	8.21 8.61	0.51 0.52	0.56 0.57	LRG LRG	1.20 1.20
	406 7090.7	17	3.7	0.075	0.107	180	11	16.3	0.82	23.07	23.29	0.71	4.578	0.1040	NONE	1.000	4.710	0.107	7.80	9.02	0.53	0.58	LRG	1.20
	408 7090.8 410 7091	16 15	3.6 3.5	0.075 0.075	0.105 0.099	180 180	11.1 11.1	16.2 16.2	0.85	22.12 21.3	22.35 21.55	0.72 0.76	4.629 5.363	0.1023 0.1134	NONE	1,000 1,000	4.749 4.682	0.105 0.099	8.14 8.45	9.44 9.99	0.53 0.52	0.59 0.58	LRG LRG	1.20 1.20
	412 7091.1	14	3.4	0.050	0.102	180	11.4	15.8	0.92	20.26	20.52	0.77	5.156	0.1076	NONE	1.000	4.887	0.102	8.89	10.49	0,55	0.61	LRG	1.20
	414 7091.2 416 7091.3	13 12	3.3 3.2	0.050 0.050	0.104 0.106	180 180	11.7 12	15.4 15	0.95 0.99	19.27 18.34	19.55 18.64	0.78 0.81	5,554 5,836	0.1135 0.1161	NONE	1.000 1.000	5.088 5.329	0.104 0.106	9.34 9.82	11.12 11.88	0.58 0.61	0.65 0.69	LRG LRG	1.20 1.20
	418 7091.4	11	3.1	0.050	0.107	180	12.3	14.7	1.03	17.39	17.71	0.83	5.488	0.1066	NONE	1.000	5.511	0.107	10.35	12.67	0.64	0.73	LRG	1.20
	420 7091.5 430 7092	10 5	3	0.050 0.050	0.105 0.092	180 180	12.4 12.7	14.5 14.1	1.09 1.49	16.54 13.94	16.89 14.42	0.86 0.98	5.434 6.133	0.1017 0.1005	NONE	1.000 1.000	5.610 5.616	0.105 0.092	10.88 12.91	13.52 18.92	0.65 0.64	0.75 0.83	LRG LRG	1.20 1.20
	440 7092.5	ŏ	3	0.055	0.065	180	11.4	15.7	2.29	13.74	14.48	1.09	6.793	0.1002	NONE	1.000	4.406	0.065	13.10	26.11	0.48	0.75	LRG	1.20
HC5-3	445 7092.8 475 7093	0	2.5 2	0.033 0.005	0.047 0.006	180 180	10.5 5.2	17.2 34.9	2.62 4.18	13,1 16.72	14.11 18.69	1.22 1.87	4.924 2.954	0.0649 0.0253	NONE	1.000 1.000	3.567 0.700	0.047 0.006	13.74 10.77	27.51 21.74	0.37 0.06	0.59 0.10	LRG LRG	1.20 1.20
1100-3	500 7093.1	ō	2	0.002	0.005	180	4.8	37.4	4.32	17.28	19.32	1.93	1.266	0.0105	NONE	1.000	0.603	0.005	10.42	20.74	0.05	0.08	LRG	1.20
	503 7093.1 508 7093.1	3 8	2.2 2.6	0.000	0.002 0.001	180 180	3	59.4		· 23.15	25.14	2.38 METHODS	-63.775	-0.4290	NONE	1.000	0.000	0.000	7.77	13.74	0.00	0.00	LRG LRG	1.20 1.20
	513 7093.1	13	3	0.000	0.001	180						METHODS		_	NONE	1.000	_	-	_	_	-	-	LRG	1.20
	515 7093.1 520 7093.1	17 29	3.2 3.6	0.000 0.010	0.001 0.001	180 180						METHODS METHODS			NONE	1.000 1.000	-	-	_	-	-	-	RM RM	0.15 0.15
	525 7093.2	40	4	0.015	0.001	180		L	OW SLO	PE - RO	CK SIZING	METHODS	FAIL	_	NONE	1.000	_	_	_	_	-		RM	0.15
	600 7094 680 7094.1	40 40	4	0.006	0.001 0.001	450 650		L	OW SLO		CK SIZING INDING	METHODS	FAIL		NONE	1.000 1.000		-	-	-		↔ .	RM RM	0.15 0.15
	780 7094.3	40	4	0.002	0.003	899	•				NDING				NONE	1.000	_	_	_	-	_	_	RM	0.15
HC5-2	880 7094.5 885 7094.5	40 40	4 4.1	0.001 0.000	0.005 0.004	1149 1149					nding Nding				NONE	1.000 1.000	_	-	-	-	-	-	RM RM	0.15 0.15
	890 7094.5	41	4.2	0.000	0.004	1149				PC	NDING				NONE	1.000	_	_	_	_	_		RM	0.15
	895 7094.5 900 7094.6	41 41	4.3 4.4	0.010 0.010	0.004 0.004	1149 1149		-			nding Nding	<u>-</u>			NONE	1.000 1.000	-	-		-	_	-	RM RM	0.15 0.15
	920 7094.6	43	4.8	0.003	0.003	1149	5	231.2	3.81	79.58	80.36	2.91	16.136	0.0890	NONE	1.000	0.544	0.003	14.44	19.05	0.04	0.05	RM	0.15
	940 7094.7 960 7094.8	44 45	5.1 5.5	0.005 0.002	0.003 0.003	1149 1149	4.7 . ′ 4.4	245.3 258.9	3.86 3.90	83.37 87.9	84.12 88.60	2.92 2.92	1.017 0.448	0.0056	NONE	1.000	0.912 0.456	0.005 0.002	13.78 13.07	18.14	0.06	0.07	RM RM	0.15
	980 7094.8	46	5.9	0.002	0.002	1149	4.3	270.4	3.90	92.02	92.68	2.90	0.411	0.0025 0.0023	NONE	1.000 1.000	0.453	0.002	12.49	17.16 16.77	0.03 0.03	0.04 0.04	RM.	0.15 0.15
	1000 7094.9 1020 7094.9	48 49	6.3 6.6	0.003	0.002 0.002	1149 1149	4.1	282.9 290.8	3.92 3.90	97.39 100.5	98.01 101.07	2.91 2.88	0.323 0.416	0.0018 0.0023	NONE	1.000	0.454 0.450	0.003 0.003	11.80 11.44	16.07 15.60	0.03 0.03	0.04 0.04	RM RM	0.15 0.15
	1040 7095	50	7	0.005	0.002	1149	3.8	305.3	3.94	105.2	105.72	2.89	0.919	0.0023	NONE	1.005	0.907	0.005	10.93	14.97	0.05	0.07	RM	0.15
	1060 7095.1 1080 7095.1	51 53	7.4 7.8	0.003	0.002 0.002	1149 1149	3.7 3.6	315.1 319	3.93 3.86	109.2 113.2	109.69 113.71	2.87 2.82	0.017 0.100	0.0001 0.0006	NONE	1.024 1.043	0.45 0 0.459	0.003 0.003	10.53 10.15	14.54 13.90	0.03 0.03	0.04 0.04	RM RM	0.15 0.15
	1100 7095.2	54	8.1	0.005	0.002	1149	3.5	328.5	3.87	116.7	117.17	2.82	0.849	0.0048	NONE	1.059	0.931	0.005	9.85	13.55	0.05	0.04	RM	0.15
	1120 7095.3 1140 7095.3	55 56	6.5 6.9	0.002	0.002	1149 1149	3.4	339.5 343.2	3.87	120.8	121.24	2.81 2.76	0.108 0.150	0.0006	NONE	1.077	0.472	0.003	9.51 9.28	13.16	0.03 0.03	0.04 0.04	RM RM	0.15 0.15
	1160 7095.4	58	9.3	0.003	0.002	1149	3.3 3.2	354.8	3.81 3.82	123.6 129.1	124.24 129.46	2.76	0.130	0.0009 0.0013	NONE	1.091 1.114	0.469 0.479	0.003 0.003	8.90	12.57 12.22	0.03	0.04	RM	0.15
	1180 7095.4	59	9.6	0.003	0.002	1149	3.2	357.6	3.77	131.4	131.78	2.72	0.222	0.0013	NONE	1.124	0.477	0.003	8.75	12.06	0.03	0.04	RM	0.15
	1200 7095.5 1300 7095.7	60 60	10 10	0.004 0.001	0.001 0.002	1149 1149	3.1 3.2	370.1 361.2	3.78 3.72	135.6 134.4	135,98 134,77	2.72 2.68	0.524 0.206	0.0031 0.0012	NONE	1.142 1.136	0.678 0.285	0.004 0.002	8.47 8.55	11.72 11.90	0.04 0.02	0.05 0.02	RM RM	0.15 0.15
	1400 7095.8	60	10 10	0.001	0.002	1149	3.2	361.2	3.72	134.4	134.77	2.68	0.251	0.0015	NONE	1.136	0.285	0.002	8.55	11.90	0.02	0.02	RM	0.15
	1500 7096 1600 7096.2	60 60	10	0.002 0.002	0.002 0.002	1149 1149	3.2 3.3	361.2 352.9	3.72 3.68	134.4 133.2	134.77 133.57	2.68 2.65	0.290 0.283	0.0017 0.0017	NONE	1,136 1,131	0.381 0.374	0.002 0.002	8.55 8.63	11.90 12.08	0.02 0.02	0.03 0.03	RM RM	0.15 0.15
	1950 7096.7	60	10	0.001	0.002	1149	3.2	361.2	3.72	134.4	134.77	2.68	0.235	0.0014	NONE	1.136	0.267	0.002	8.55	11.90	0.02	0.02	RM	0.15
HC5-1	2150 7096.9 2250 7097	60 60	10 10	0.001 0.002	0.001 0.001	1149 1149	3.1 3	370.1 380.1	3.78 3.86	135.6 137.2	135.98 137.59	2.72 2.77	0.241 0.342	0.0014	500 500	1.614 1.624	0.274 0.420	0.002 0.002	8.47 8.37	11.72 11.58	0.02 0.03	0.02 0.03	RM RM	0.15 0.15
	2300 7097.1	60 60	10 10	0.002	0.001	1149	3	380.1	3.86	137.2	137.59	2.77	0.259	0.0015	500	1.624	0.420	0.002	8.37	11.58	0.03	0.03	RM	0.15
	2400 7097.2 2500 7097.3		10	0.001 0.001	0.001 0.001	1149 1149	3 3	380.1 380.1	3.86 3.86	137.2 137.2	137.59 137.59	2.77 2.77	0.172 0.259	0.0010 0.0015	500 500	1.624 1.624	0.280 0.420	0.002 0.002	8.37 8.37	11.58 11.58	0.02 0.03	0.02 0.03	RM RM	0.15 0.15

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	Channel Station			Channel Side Siope	Bottom Slope (N/ft)	HEC-2 Energy Gradient (R/ft)	Discharge (cfs)	HEC-2 Velocity (fps)	Area (ft^2)	Flow Depth (ft)	Top Width (ft)	Wetted Perimeter		Calc- ulated Average Shear (ib/ft^2)	Equivalent Slope For Average Shear (f/fi)			Maximum Shear (ib/fi^2)	Calc- ulated Slope For Rock Sizing (ff/ft)	Unit	Unit	Unit Discharge Stephenson Rock D50 (ft)	Unit Discharge Stephenson Rock D50 (ft)	Rock Type+	Target Riprap D50 (ft)
												c	HANNEL	Q										_	
	290	7064	30	4	0.025	0.021	803	9.1	88	2.26	48.08	48.64	1.81	2.415	0.0213	NONE	1.000	2.830	0.025	16.70	20.57	0.25	0.29	INT	0.60
		7064.2 7064.5	30 30	4	0.025 0.025	0.02 0.015	792 782	8.9 8.1	88.5 96.7	2.27 2.43	48.16 49.44	48.72 50.04	1.82 1.93	2.536 2.795	0.0223 0.0232	NONE	1.000 1.000	2.841 3.009	0.025 0.025	16.45 15.82	20,20 19,68	0.25 0.24	0.29 0.28	INT	0.60 0.60
	320 330	7064.7 7065	30 30	4	0.025 0.025	0.021 0.029	772 761	9.1 10.1	84.9 75.5	2.19 1.99	47.52 45.92	48.06 46.41	1.77 1.63	2.858 3.681	0.0259 0.0362	NONE	1.000 1.000	2.755 2.945	0.025 0.029	16.25 16.57	19.93 20.10	0.25 0.28	0.28 0.32	INT	0.60 0.60
		7065.2 7066.1	30 30	4	0.033 0.045	0.046 0.045	751 751	11.7 11.6	64.2 64.5	1.74	43.92 43.92	44.35 44.35	1.45 1.45	3.534 4.033	0.0391 0.0446	NONE	1.000	4.162 4.072	0.046 0.045	17.10 17.10	20.36	0.42	0.48	INT	0.60
	380 400	7067 7068	30 30	4	0.047	0.045	751	11.6	64.8	1.75	44	44.43	1.46	4.249	0.0467	NONE	1.000	4.319	0.047	17.07	20.18 20.30	0.42 0.43	0.46 0.49	INT	0.60 0.60
	420	7068.9	30	4	0.047 0.045	0.043 0.039	751 751	11.4 11.1	65.7 67.9	1.77 1.82	44.16 44.56	44.60 45.01	1.47 1.51	4.181 3.647	0.0455 0.0388	NONE	1.000 1,000	4.362 4.233	0.047 0.045	17.01 16.85	20,18 20,20	0.43 0.41	0.49 0.46	INT INT	0.60 0.60
HC5-12	500	7069.8 7071.3	30 30	4	0.035 0.025	0.025 0.025	751 751	9.5 9.5	78.8 79.2	2.06 2.07	46.48 46.56	46.99 47.07	1.68 1.68	3.469 2.639	0.0332 0.0251	NONE	1.000 1.000	3.661 2.626	0.035 0.025	16.16 16.13	19.57 19.67	0.32 0.25	0.37 0.28	INT	0.60 0.60
		7072.3 7073.6	30 30	4	0.026 0.026	0.025 0.024	751 751	9.6 9.4	78.4 80	2.05 2.09	46.4 46.72	46.90 47.23	1.67 1.70	2.654 2.704	0.0255 0.0255	NONE	1.000 1.000	2.657 2.701	0.026 0.026	16.19 16.07	19.68 19.65	0.25 0.25	0.29 0.29	INT INT	0.60 0.60
	690	7076.1 7078.6	30 30	4	0.025 0.025	0.026	751 751	9.7	77.6	2.04	46.32	46.82	1.66	2.582	0.0249	NONE	1,000	2.697	0.026	16.21	19.79	0.26	0.29	INT	0.60
	870	7080.6	30	4	0.025	0.027	751	9.3 9.8	81.1 76.5	2:11 2:01	46.88 46.08	47,40 46.57	1.71 1.64	2.665 2.539	0.0250 0.0248	NONE	1.000 1.000	2.669 2.766	0.025 0.027	16.02 16.30	19.62 19.70	0.25 0.26	0.28 0.30	INT	0.60 0.60
	980	7083.1 7083.4	30 30	4	0.027 0.027	0.021 0.016	751 747	8.9 8.2	84,1 91.3	2.17 2.32	47.36 48.56	47.89 49.13	1.75 1.85	2.959 3.659	0.0271 0.0316	NONE	1.000 1.000	3.007 3.183	0.027 0.027	15.86 15.38	19.31 19.02	0.26 0.26	0.30 0.30	INT	0.60 0.60
		7085.9 7087.3	30 30	4	0.059 0.094	0.093	706 706	14.5 14.5	48.7 48.8	1.37 1.37	40.96 40.96	41.30 41.30	1.18 1.18	5.206 6.852	0.0709 0.0933	NONE	1.000 1.000	5 206 6 916	0.071 0.094	17.24 17.24	19.87 19.87	0.62 0.80	0.68 0.87	LRG LRG	1.20 1.20
	1115 1135	7089.2 7091	30 30	4	0.093	0.092 0.089	706 706	14.4 14.3	48.9 49.5	1:38 . 1:39	41.04 41.12	41.38 41.46	1.18 1.19	6.686 6.546	0.0905 0.0880	NONE	1.000	6.837 6.881	0.093	17.20 17.17	19.87 19.88	0.78 0.78	0.86 0.86	LRG	1.20
	1155	7092.9 7094.8	30	4	0.095	0 079	706	13.8	51,3	1.44	41.52	41.87	1 23	6.043	0.0787	NONE	1.000	7.290	0.095	17.00	19.87	0.79	0.88	LRG LRG	1 20 1.20
	1180	7094.8	30 30	4	0.047 0.003	0.053 0.009	706 706	12 6.4	58.9 109.7	1.61 2.69	42.88 51.52	43.28 52.18	1.36 2.10	0.904 7.518	0.0107 0.0573	NONE	1.000 1.000	4.018 1.180	0.047 0.009	16.46 13.70	19.32 17.22	0.42 0.10	0.47 0.11	LRG LRG	1.20 1.20
	1215	7094.9 7094.9	10 0	4	0.003	0.046 0.033	706 706	13 11.8	54.6 60.3	2.64 3.86	31.12 30.88	31.77 31.83	1.71 1.87	8.739 4.946	0.0820 0.0423	NONE	1.000 1.000	4.904 3.858	0.046 0.033	22.69 22.86	34.32 45.55	0.51 0.39	0.67 0.62	LRG	1.20 1.20
	1220 1230	7094.9 7095	0	3.7 3	0.005 0.005	0.03 0.015	706 706	11.6 9.4	61.5 64.6	4.05 5.02	29.97 30.12	31.05 31.75	1.95 2.38	6.980 5.388	0.0572 0.0363	NONE	1.000 1.000	3.659 2.229	0.030 0.015	23.56 23.44	46.98 47.19	0.37 0.21	0.58 0.33	LRG LRG	1.20 1.20
	1240 1270	7095 7095	0	2.8 2.5	0.000	0.02 0.018	706 706	10.4 10.3	77 71.4	4.91 5.24	27.5 26.2	29.20 28.22	2.31 2.43	1.338 1.488	0.0093 0.0098	NONE	1.000	1.338	0.009	25.68	51.06	0.15	0.24	LRG	1.20
	1275	7095	ŏ	2.4	0.000	0.022	706	11.2	75.4	5.13	24.62	26.68	2.37	2.872	0.0194	NONE	1.000	3.250	0.022	26.95 28.67	53.97 57.46	0.16 0.33	0.26 0.52	LRG LRG	1.20 1.20
HC5-11	1285 1295	7095 7095	0	2.2	0.000 0.000	0.02 0.012	706 706	11.1 9.3	69.3 99.7	5.39 6.16	23.72 24.64	26.05 27.55	2.45 2.75	5.576 4.834	0.0364 0.0281	NONE	1.000 1.000	3.062 2.063	0.020 0.012	29.77 28.65	59.83 57.29	0.31 0.20	0.49 0.32	LRG LRG	1.20 1.20
	1320 1325	7095 7095	0 6	2 2.4	0.000 0.000	0.007 0.001	706 706	7.4 3.8	95,3 90.5	6.91 7.68	27.64 42.86	30.90 45.94	3.09 4.08	5.719 -140.244	0.0297 -0.5502	NONE	1.000 1.000	1.350 0.000	0.007 0.000	25.54 16.47	51.13 29.18	0.12 0.00	0.19 0.00	LRG SML	1.20 0.40
	1330 1335	7095 7095	12 18	2.8 3.2	0.000	0.001 0.001	706 706				NDING NDING					NONE	1.000 1.000							SML	0.40 0.40
	1340 1345	7095 7095	24 30	3.6 4	0.000 0.050	0.001 0.001	706 706			PO	IDING		•			NONE	1.000							SML	0.40
	1348	7095.3 7095.8	30 30	4	0.112 0.125	0.001 0.001	700 650			POI	IDING		-			NONE	1.000							SML	0.40
	1356	7096.3	30	4	0.113	0.001	600			POI	iding iding					NONE	1.000							SML SML	0.40 0.40
	1364	7096.7 7097.2	30 30	4	0.113 0.112	0.001 0.001	550 500			POI	1DING 1DING					NONE	1.000 1.000							SML SML	0.40 0.40
	1368 1372	7097.6 7098.1	30 30	4	0.112 0.112	0.001 0.001	450 400				NDING NDING					NONE	1.000 1.000							SML	0.40 0.40
	1376 1380	7098.5 7099	30 30	4	0.112 0.125	0.08 0.078	480 460	12.1 11.8	70.8 26.5	1.15 1.13	39.2 39.04	39.48 39.32	1.01 0.99	49.804 3.230	0.7920 0.0522	NONE	1.000 1.000	7.075 7.738	0.112 0.125	12.24 11.78	13.92 13.33	0.75 0.81	0.61 0.88	LRG LRG	1.20 1.20
	1384 1388	7099.5 7099.9	30 30	4	0.112	0.074 0.07	440 420	11.4	25.7 24.8	1.12	38.96 38.88	39.24 39.15	0.98 0.98	2.493 2.271	0.0406 0.0373	NONE	1.000	6.909	0.112	11.29	12.77	0.71	0.77	LRG	1.20
	1392	7100.4	30 30	4	0.125	0.063	400	10.5	23,6	1.11	38.88	39.15	0.98	2.820	0.0463	NONE	1.000	6.854 7.616	0.112 0.125	10.80 10.29	12.21 11.66	0.69 0.74	0.75 0.80	LRG LRG	1.20 1.20
	1400	7100.9 7101.3	30	4	0.112 0.112	0.101 0.103	300 290	11,1 11	22 19.9	0.82 0.8	36.56 . 36.4	36.76 36.60	0.74 0.73	3.132 3.161	0.0676 0.0698	NONE	1.000 1.000	5.211 5.095	0.112 0.112	. 8.21 7.97	9.10 8.80	0.57 0.56	0.61 0.60	LRG LRG	1.20 1.20
		7101.8 7102.2	30 30	4	0.113 0.113	0.106 0.112	280 270	10.9 11	17.6 30.3	0.77 0.75	36.16 36	36.35 36.18	0.70 0.68	3.104 6.021	0.0710 0.1411	NONE	1.000 1.000	4.919 4.802	0.113 0.113	7.74 7.50	8,39 8,25	0,55 0,54	0.58 0.57	lrg Lrg	1.20 1.20
		7102.7 7103.1	30 30	4	0.112 0.112	0.122 0.146	260 250	11.2 11.6	28.9 21.3	0.71 0.66	35.68 35.28	35.85 35.44	0.65 0.61	6.138 5.999	0.1513 0.1582	NONE	1.000	4.951 5.537	0 122 0 146	7.29 7.09	7.95 7.66	0.57 0.67	0.61 0.71	LRG LRG	1.20 1.20
		7103.6 7105	30 30	4	0.162 0.207	0.2 0.19	243 243	12.7 12.5	28.3 28.3	0.59	34.72 34.8	34.87 34.95	0.55 0.56	8.961 9.633	0.2623 0.2775	NONE	1.000	6.834 7.190	0.200	7.00 6.98	7.49	0.93 0.97	0.97	LRG LRG	1.20
	1434	7106.5 7108.1	30 30	4	0.187	0.159	243	11.9	29.2	0.63	35.04	35.20	0.58	8.899	0.2450	NONE	1.000	6.798	0.187	6.93	7.50 7.50	0.86	1.01 0.90	LRG	1.20 1.20
	1454	7109.B	30	4	0.165 0.085	0.144 0.047	243 243	11.5 8	32.4 33	0.65 0.9	35.2 37.2	35.36 37.42	0.60 0.81	7.453 2.433	0,1993 0,0482	NONE	1.000 1.000	6.170 4.286	0.165 0.085	6.90 6.53	7.48 7.20	0.75 0.38	0.79 0.40	LRG LRG	1.20 1.20
	1475	7109.8 7109.9	29 10	4	0.003 0.008	0.053 0.072	243 243	8.4 11.4	33.3 29.7	0.88 1.38	36.04 21.04	36.26 21.38	0.79 1.00	3.333 4.985	0.0677 0.0797	NONE	1.000 1.000	2.610 4.501	0.053 0.072	6.74 11.55	7.39 15.73	0.26 0.48	0.27 0.59	LRG LRG	1.20 1.20
	1485 1490	7110 7110	0	4 3.6	0.005 0.000	0.03 0.027	243 243	8.7 8.6	29.7 29.3	2.64	21.12	21.77	1.28	5.183	0.0649	NONE	1,000	2.397	0.030	11.51	22.97	0.23	0.36	LRG	1.20

TABLE 5-6. CHANNEL CONVEYANCE CHARACTERISTICS BASED ON HEC-2 MODEL SIMULATION (CONTINUED).

Channel Cross Section Name	Channel Station	Elev.		Channel Side			t Discharge	HEC-2 Velocity (fps)	Area (f(^2)	Flow Depth (ft)	Top Width	Wetted Perimeter (ft)	Hydraulic Redius (ft)	Calc- ulated Average Shear (lb/ft^2)	Equivalent Slope For Average Shear (fi/ft)		Channel Bend Stress Ratio	Maximum Sheer (ib/ft*2)	Calc- ulated Slope For Rock Sizing (fl/ft)	Average Unit Discharge (cfs/(t)	Unit	Average Unit Discharge Stephenson Rock D50 (ft)	Maximum Unit Discharge Stephenson Rock D50 (ft)	Rock Type+	Target Riprap D50 (ft)	
												•	CHANNEL	2												
HC5-10	1499 1500 1510 1530 1543 1548 1553 1568 1563 1568 1573 1578	7110 7110 7110 7110.1 7110.1 7110.1 7110.1 7110.1 7110.1 7110.1 7110.1 7110.1	0 0 0 0 1 3 5 7 9 11 13 15	3 2.7 2.2 2.2 2.4 2.7 2.9 3.2 3.5 3.7	0.000 0.003 0.003 0.000 0.000 0.000 0.000 0.000 0.000 0.000	0.017 0.016 0.019 0.022 0.004 0.002 0.001 0.001 0.001	243 243 243 243 243 243 243 243 243 243	7.5 7.4 7.9 8.7 4.7 3.4 2.7	PONE PONE PONE	DING DING DING DING			1.56 1.57 1.58 1.62 2.20 2.50 2.68	3.517 1.212 1.554 2.909 5.970 2.610 -62.836	0.0362 0.0123 0.0158 0.0288 0.0485 0.0167 -0.3758	NONE NONE NONE NONE NONE NONE	1.000 1.000 1.000 1.000 1.000 1.000 1.000	1.650 1.212 1.554 2.225 0.548 0.312 0.000	0.017 0.012 0.016 0.022 0.004 0.002 0.000	12.35 12.20 13.35 15.51 11.46 9.31 7.75	24.60 24.57 26.62 30.97 21.57 16.35 13.18	0.15 0.12 0.15 0.22 0.05 0.02 0.00	0.24 0.19 0.24 0.34 0.07 0.03 0.00	LRG LRG LRG LRG SML SML SML SML SML	1.20 1.20 1.20 1.20 1.20 1.20 0.40 0.40 0.40 0.40	

Rock Type

LRG = Large Riprap INT = Intermeditate Riprap SML = Small Riprap RM = Rock Mulch

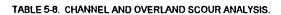
TABLE 5-7. HYDRAULIC JUMP ANALYSIS.

			CHANNEL	1.	PMF DIS	CHARGE -	620	<u>CFS</u>			
STATION	CHANNEL BASE WIDTH	CHANNEL SIDE SLOPES	CHANNEL BOTTOM SLOPE	MANNINGS	FLOW DEPTH	FROUDE NUMBER	TURB- ULENT UNIT FLOW (1.5 * avg. q)	TURB- ULENT DESIGN ROCK D50	DESIGN ROCK D50 (TABLE 5-5)	TARGET ROCK D50 (TABLE 5-5)	RATIO TARGET/ DESIGN D50
(ft)	(ft)		(ft/ft)		(ft)		(cfs/ft)	(ft)	(ft)	(ft)	
			DISC	HARGE = 620	CFS						
50	40	4	0.002	0.035	2.99	0.45	17.9	0.04			
100	40	4	0.004	0.035	2.47	<u>0.62</u>	18.6	0.06		1.2	18.9
157	40	4	0.065	0.035	1.14	2.10	20.9	0.65		1.2	1.9
165	40	4	0.127	0.035	0.95	2.81			0.79	1.2	1.5
175	40	4	0.134	0.035	0.92	2.95			0.81	1.2	1.5
183	40	4	0.134	0.035	0.92	2.95			0.81	1.2	1.5
193	40	4	0.13	0.035	0.94	2.88			0.78	1.2	1.5
203	40	4	0.13	0.035	0.94	2.88			0.78	1.2	1.5
				HARGE = 310			o.				
50	40	4	0.002	0.035	2.06	0.42	9.6	0.02			
100	40	4	0.004	0.035	1.70	<u>0.56</u>	9.9	0.04		1.2	28.7
157	40	4	0.065	0.035	0.78	<i>1.92</i>	10.8	0.42		1.2	2.9
165	40	4	0.127	0.035	0.64	2.58	*		0.79	1.2	1.5
175	40	4	0.134	0.035	0.62	2.71			0.81	1.2	1.5
183	40	4	0.134	0.035	0.62	2.71	•		0.81	1.2	1.5
193	40	4	0.13	0.035	0.62	2.71			0.78	1.2	1.5
203	40	4	0.13	0.035	0.62	2.71			0.78	1.2	1.5
				CHARGE = 62							
50	40	4	0.002	0.035	0.84	0.34	2.1	0.01		1.2	
100	40	4	0.004	0.035	0.69	9.47	2.2	0.02		1.2	79.0
157	40	4	0.065	0.035	0.31	<i>1.</i> 56	2.3	0.15	0.79	1.2	8.2
165	40	4	0.127	0.035	0.26	2.09			0.81	1.2	1.5
175	40	4	0.134	0.035	0.26	2.09			0.81	1.2	1.5
183	40	4	0.134	0.035	0.26	2.09			0.78	1.2	1:.5
193	40	4	0.13	0.035	0.26	2.03			0.78	1.2	1.5
203	40	4	0.13	0.035	0.26	2.03			0.88	1.2	1.4

			CHANNEL H		PMF DIS	CHARGE -	620	CFS			
STATION (ft)	CHANNEL BASE WIDTH (ft)	CHANNEL SIDE SLOPES	ENERGY GRAD. (ft/ft)	MANNINGS n	FLOW DEPTH	FROUDE NUMBER	TURB- ULENT UNIT FLOW (1.5 * avg. q)	TURB- ULENT DESIGN ROCK D50	DESIGN ROCK D50 (TABLE 5-5) (ft)	TARGET ROCK D50 (TABLE 5-5)	RATIO TARGET/ DESIGN D50
			DISC	HARGE = 620	CFS						
1100	40	4	0.007	0.035	2.11	0.80			0.06	1.2	20.0
1200	40	4	0.004	0.035	2.46	0.62			0.04	1.2	30.0
1240	40	4	0.004	0.035	2.46	0.62			0.05	1.2	24.0
1280	38	4	0.018	0.035	1.67	1.21			0.03	1.2	40.0
1303	35	4	0.016	0.035	1.80	1.17			0.14	1.2	8.6
1351	29	4	0.03	0.035	2.88	. 0.63		15	0.3	1.2	4.0
1402	5	4	0.041	0.035	4.82	0.57		1 1 S 1	0.51	1.2	2.4
1470	0	4	0.006	0.035	5.42	0.56			0.09	1.2	13.3

TABLE 5-7. HYDRAULIC JUMP ANALYSIS (CONTINUED).

		CHANNEL Q			PMEDIS	CHARGE -	843	CFS			
STATION (ft)	CHANNEL BASE WIDTH (ft)	CHANNEL SIDE SLOPES	CHANNEL BOTTOM SLOPE (ft/ft)	MANNINGS n	FLOW DEPTH (ft)	FROUDE NUMBER	TURB- ULENT UNIT FLOW (1.5 * avg. q) (cfs/ft)	TURB- ULENT DESIGN ROCK D50 (ft)	DESIGN ROCK D50 (TABLE 5-10) (ft)	TARGET ROCK D50 (TABLE 5-10) (ft)	RATIO TARGET/ DESIGN D50
			DISC	HARGE = 843	CFS				:		
100	30	4	0.013	0.015	2.02	1.49			_		
130	30	4	0.013	0.015	2.02	1.49				_	_
220	30	4	0.011	0.035	2.76	<u>0.89</u>	30.8	0.20		0.60	3.0
230	30	4	0.018	0.035	2.42	1.11	31.9	0.30		0.60	2.0
250	30	4	0.023	0.035	2.25	1.25			0.24	0.60	2.5
260	30	4	0.025	0.035	2.20	1.30			0.26	0.60	2.3
270	30	4	0.025	0.035	2.20	1.30			0.25	0.60	2.4
280	30	4	0.025	0.035	2.20	1.30			0.25	0.60	2.4
			DISC	HARGE = 422	CFS	•					
100	30	4	0.013	0.015	1.37	1.41				_	_
130	30	4	0.013	0.015	1.39	1.37			_		_
. 220	30	4	0.011	0.035	1.87	<u>0.85</u>	16.9	0.13		0.60	4.6
230	30	4	0.018	0.035	1.64	<u> 1.06</u>	17.3	0.20		0.60	3.0
250	30	4	0.023	0.035	1.52	1.19	e*		0.24	0.60	2.5
260	30	4	0.025	0.035	1.49	1.23			0.26	0.60	2.3
270	30	4	0.025	0.035	1.48	1.24			0.25	0.60	2.4
280	30	4	0.025	0.035	1.48	1.24			0.25	0.60	2.4
				HARGE = 84 (
100	30	4	0.013	0.015	0.43	1.71					_
130	30	4	0.013	0.015	0.43	1.74			<u> </u>		_
220	30	4	0.011	0.035	0.75	<u>0.73</u>	3.8	0.05		0.60	12.2
230	30	4	0.018	0.035	0.65	0.90	3.9	0.07		0.60	8.2
250	30	4	0.023	0.035	0.60	1.02	3.9	0.09		0.60	6.7
260	30	4	0.025	0.035	0.58	1.08			0.26	0.60	2.3
270	30	4	0.025	0.035	0.59	1.06			0.25	0.60	2.4
280	30	4	0.025	0.035	0.59	1.06			0.25	0.60	2.4



CHAN- NEL STAT- ION (ft)	CHAN- NEL BASE ELEV. (R+msl)	CHAN- NEL BASE WIDTH (ft)	CHAN- NEL SIDE SLOPE	BOT- TOM SLOPE (N/fl)	DIS- CHARGE (cfs)	MAN- NINGS n	FLOW AREA (ft^2)	FLOW DEPTH (fl)	TOP WIDTH (ft)	WETTED PER- IMETER (ft)	HYD- RAULIC RADIUS (ft)	ROCK TYPE+	TARGET RIPRAP D50 (ft)	PEAK FLOW DURA- TION (min)	BASE FLOW DURA- TION (min)	CLAY AND SHALE USDOT SCOUR DEPTH (ft)	GRADED SAND USDOT SCOUR DEPTH (ft)	FHA SCOUR DEPTH (ft)
4		•								C	CHANNEL	Н						
0	6963.6	40	4	0.0020	620	0.015	97.3	2.02	56.2	56.69	1.72	NO R	оск• —	40	400	4.4	3.9	13.8
20	6963.65	40	4	0.017	620	0.035	83.9	1.78	54.3	54.69	1.53	LRG	1.20	40	400			
203	6970.1	40	4	0.025	620	0.035	73.9	1.59	52.8	53.14	1.39	LRG	1.20	40	400	•		
300	6983.5	40	4	0.045	620	0.035	61.0	1.34	50.8	51.09	1.19	LRG	1.20	40	400			
										•	CHANNEL	N						
0	7072.6	30	4	0.0330	229	0.015	19.0	0.59	34.7	34.85	0.55	NO R	OCK*	40	200	1.7	4.4	9.0
100	7076.2	30	4	0.040	229	0.035	30.7	0.91	37.3	37.53	0.82	INT	0.60	40	200			
140	7078.3	30	4	0.047	180	0.035	25.0	0.76	36.1	36.24	0.69	INT	0.60	40	200		•	
190	7079	30	4	0.022	180	0.035	31.9	0.94	37.6	37.79	0.84	INT	0.60	40	200			
									•	(CHANNEL	Q						
100	7060.9	30	4	0.013	. 843	0.015	60.1	1.64	43.1	43.54	1.38	NO F	OCK*	17	100			
130	7061.3	30	4	0.013	843	0.015	60.1	1.64	43.1	43.54	1.38	NO F	OCK*	17	100	5.1	6.9	14.0
220	7062.4	30	4	0.011	843	0.035	113.4	2.76	52.1	52.78	2.15	INT	0.60	17	100			
230	7062.5	30	4	0.018	843	0.035	95.6	2.41	49.3	49.89	1.92	INT	0.60	- 17	100			
280	7063.7	30	4	0.022	843	0.035	89.3	2.28	48.3	48,81	1.83	INT	0.60	17	100			
Þ.						•				OVERLA	ND FLOW F	ATH OM-2						
OM-2B	<u></u>	1	0	0.052	0.257	0.015	0.1	0.07	1.0	1.14	0.06	NO F	юск•	10	60	0.1	0.3	0.5
OM-2B		1	0	0.063	0.257	0.035	0.1	0.12	1.0	1.23	0.09	RM	0.15	10	60			

(Revised (Revised 05/20/96) 11/24/97)

TABLE 5-9. END PROTECTION AND TOE PROTECTION STRUCTURE DESIGN

Channel Or Overland Name	Channel Base Width (ft)	Channel Side Slope	End Protection Bottom Slope (ft/ft)	Manning's	Discharge (cfs)	Flow Area (ft^2)	Flow Depth (ft)	Top Width (ft)	Wetted Perimeter (ft)	Hydraulic Radius (ft)	Unit Discharge (cfs/ft)	Stephenson Rock D50 (ft)	Rock Type+	Riprap D50 (ft)	End Protection Length (ft)
Н	40	2	0.160	0.035	620	39.301	0.92	43.7	44.10	0.89	14.20	1.17	LRG	1.20	28
N	30	3 ,	0.250	0.035	229	16.923	0.54	33.2	33.39	0.51	6.89	1.20	LRG	1.20	60
Q	30	4	0.130	0.035	843	49.043	1.38	41.0	41.38	1.19	20.54	1.21	LRG	1.20	- 54
Basin 5-4	 ·		0.180					_			. <u>-</u>	<u> </u>	INT	0.6	25
OM-2B	1 -	0	0.270	0.035	0.257	0.100	-0.57	1.0	2.14	0.05	0.26	0.15	RM	0.15	

Rock Type

LRG = Large Riprap INT = Intermeditate Riprap SML = Small Riprap RM = Rock Mulch

CROSS- SECTION NAME	Channe Base Width (ft)	Channel Side Slope	Bottom Slope (ft/ft)	Manning's n	1000-Yr Discharge (cfs)	1000-Yr Flow Area (ft^2)	1000-Yr Flow Depth (ft)	1000-Yr Top Width (ft)	1000-Yr Wetted Perimeter (ft)	1000-Yr Hydraulic Radius {ft)	1000-Yr Unit Discharge (cfs/ft)	1000-Yr Stephenso Rock D50 (ft)	n Rock Type+	Target Riprap D50 (ft)
HCN-1	0	2	0.002	0.035	304	79.879	6.32	25.3	28.26	2.83	12.03	0.03	LRG	1.20
HCN-2	25	3	0.017	0.015	349	28.669	1.02	31.1	31.46	0.91	11.21		O ROCK-	
HCN-3	25	4	0.055	0.035	349	34.844	1.17	34.4	34.68	1.00	10.15	0.35	INT	0.60
HCN-4	0	2	0.002	0.035	75	26.221	3.23	12.9	14.44	1.82	5.81	0.02	LRG	1.20
HCN-5	25	4	0.167	0.035	75	9.091	0.34	27.8	27.84	0.33	2.70	0.40	LRG	1.20
HCS-1	10	3	0.001	0.015	186	42.221	2.44	24.6	25.42	1.66	7.55		IO ROCK-	
HCS-2	10	3	0.001	0.015	127	32.079	2.00	22.0	22.67	1.41	5.77	1	IO ROCK-	
HCS-3	15	4	0.006	0.015	49	3.037	0.18	16.5	1.00	3.04	2.97		IO ROCK-	
HCS-4	15	4	0.09	0.035	49	2.241	0.14	16.1	1.00	2.24	3.04	0.24	SML	Ó. 4 0

+ Rock Type

LRG = Large Riprap INT = Intermeditate Riprap SML = Small Riprap RM = Rock Mulch

TABLE 5-11. SUMMARY OF CONSOLIDATION ANALYSIS.

Dry Soil Dens	emeter (in) e Content (%) sity (PCF) r Soil Specime Gravity en Height (in)		SAMPLE TW	4-1B (25'-27 1.92 36.00 85.00 0.14 2.65 1.00 0.51	40 00 00 45 50
Pressure (ksf)	Change in Specimen	Final Specimen	Height of Void	Final Void	Compression Index
0.000	Height (in)	Height (in)	(in)	Ratio	Сс
0.000	-0.012	1.000	0.486	0.945	
0.500	-0.012	0.988	0.474	0.922	0.131
1.000	-0.032	0.968	0.454	0.883	0.131
1.000	-0.068	0.900	0.404	0.005	0.227
2.000		0.933	0.418	0.814	
4.000	-0.098	0.903	0.388	0.756	0.194
Description of Specimen Dia Initial Mositure Dry Soil Dens Weight of Dry Soil Specific O Initial Specime Height of solid	imeter (in) e Content (%) ity (PCF) Soil Specime Gravity en Height (in)	n (lb)	SAMPLE TW4	1.94 80.00 55.00 0.09 2.65 1.00 0.33	.0 0 0 0 04 50 (Est.)
Pressure	Change in	Final	Height of	Final	Compression
(ksf)	Specimen	Specimen	Void	Void	Index
()	Height (in)	Height (in)	(in)	Ratio	Сс
0.000		1.000	0.667	2.007	······································
	-0.038				
0.500		0.962	0.630	1.893	
	-0.047				0.089
1.000		0.954	0.621	1.867	-
	-0.054	-		-	0.074
2.000	 -	0.946	0.613	1.844	· · · ·
*	-0.064				0.101
	-				

1.814

0.936 0.603

4.000

TABLE 5-12. PREDICTED SETTLEMENT FOR PONDS 4 AND 5.

Compression Index = 0.184 Estimated Void Ratio = 1.000

Well No	Tailings Depth (ft)	Estimated Settlement (ft)
TW4-1C	44.000	1.290
TW4-2C	44.000	1.290
TW4-3C	49.000	1.400
TW4-4C	28.000	0.930
TW4-5C	34.000	1.070
TW5-1C	53.000	1.480
TW5-2C	38.000	1.160
TW5-3	43.000	1.270

TABLE 5-13. INFILTRATION ESTIMATED DURING THE PMF.

Channel Control Site		Maximum Ponding Time	Maxium Ponding Height	Infiltra Dej		Time Required To Remove By Evapotranspiration
		(Hour)	(Feet)	(cm)	(in)	(Days)
	HCT-1	15	5	3.92	1.54	2.71
	HC5-3	16.2	6	4.13	1.63	2.85
	HC5-11	1.5	5.3	2.55	1.00	1.76

NOTE: PONDING TIME IS FOR PORTION OVER TAILINGS MATERIAL

THAT CAN BE VIEWED AT THE RECORD TITLED:

"EXHIBIT 5-1, DRAINAGE AREA AND OVERLAND FLOW PATHS FOR THE MILL AND TAILINGS AREA, EXH5-1.DWG"

WITHIN THIS PACKAGE... OR BY SEARCHING USING THE

THAT CAN BE VIEWED AT THE RECORD TITLED:

"EXHIBIT 5-1A, NORTH TAILINGS AREA DRAINAGE DETAIL, EXH5-1A.DWG"

WITHIN THIS PACKAGE... OR BY SEARCHING USING THE

THAT CAN BE VIEWED AT THE RECORD TITLED:

"EXHIBIT 5-1B, SOUTH TAILINGS AREA DRAINAGE DETAIL"

WITHIN THIS PACKAGE... OR BY SEARCHING USING THE

THAT CAN BE VIEWED AT THE RECORD TITLED:

"EXHIBIT 5-2, MAJOR DRAINAGE BASINS AND SETTLEMENT MONUMENT LOCATIONS, EXH5-2DWG"

WITHIN THIS PACKAGE... OR BY SEARCHING USING THE

D-07X

6.0 LONG-TERM POTENTIAL HAZARDS

Several other possible long-term hazards have been considered during the formation of this reclamation plan. They include animal or root penetration of covers, root uptake of hazardous substances, upward migration of hazardous substances through cover materials, frost damage to the radon/infiltration barrier, earthquake damage to the tailings encapsulation system, flooding by nearby streams and geotechnical stability of the area. After consideration, none of the above are thought to pose a significant threat to the tailings protection which could result in exposure of tailings.

Animal or root penetration of the cover are considered two of the more significant potential long term hazards. The majority of the tailings area is covered with rock mulch and underlying filter. The presence of the rock will prevent animal burrowing in these areas. The thick, dense clay radon/infiltration barrier underneath the rock mulch filter should also serve to prevent burrowing if an animal is persistent enough to dig through the rock mulch. In areas where the rock mulch is not present, the topsoil layer will be relatively shallow and will again be underlain by a dense clay barrier. In addition to the difficulty in digging through the compacted clay barrier, the non-rocked tailings areas should be an undesirable burrowing animal habitat because of exposure to predators. The surfaces are very flat with only sparse grass vegetation expected on non-rocked areas, thus giving very little cover for small animals.

Root penetration of the clay barrier is not expected to be a problem. In rock mulched areas, the rock should prevent emergence of the majority of plant species because of its thickness and low water holding capacity. The underlying clay barrier has a very low permeability $(2.6 \times 10^{-8} \text{ cm/sec})$, and water is not expected to penetrate the barrier more than a few inches under worst case conditions. Thus, the available water in the clay barrier is also

very limited. In non-rocked areas, the depth of uncompacted material will be very limited and will only be capable of supporting shallow rooted plants. Infiltration into the clay barrier will be very limited, and deep rooted plants will not be able to penetrate the dense, thick barrier, or survive on the limited water available.

For the reasons discussed above, root penetration through the barrier resulting in uptake of radioactive materials is not expected. Migration of hazardous substances from the tailings through the clay cover is not expected because of the small permeability of the clay cover. The clay barrier will be compacted to the point where flux of fluids through the barrier is very limited. Soil gas or water transfer across the barrier from one interface to the other is not anticipated.

Frost heaving or damage is associated with relatively moist In order for frost heave to occur, sufficient water must be present to result in expansive ice lenses. According to the Earth Manual (1974), "The freezing of the pore water in saturated fine-grained soils will decrease the density of the mass by expansion, but will not result in an appreciable frost heave unless water movement from below can take place.". The tailings will be dewatered to the point where the near-surface tailings are at or near the wilting point moisture content, and movement of water from the tailings into the clay barrier will be minimal. clay barrier is also designed to preclude water infiltration, and the moisture content in the clay barrier should be relatively constant at or near the predicted long-term moisture content. topsoil material and possibly the top few inches the clay barrier may be subject to frost action, but damage to the barrier is not likely.

An earthquake risk evaluation for the area was conducted prior to construction to the No. 5 tailings dam (Dames and Moore, 1975).



The historic seismicity for central Wyoming is low and the maximum magnitude for earthquakes cited by Dames and Moore, (1975) is a VII on the Modified Mercalli scale. The tailings material will be largely dewatered and liquefaction potential should be relatively low.

The primary area of concern for slope stability is the dam outslope area. This concern is all but eliminated for the Pond 4 dam due to back filling of tailings and cover material on the outslope. Outslope areas on the Pond 5 and Pond 3 dams will be modified under the reclamation plan to enhance hydrologic stability, and this will dramatically increase slope stability to the point where it is no longer a concern.

Several factors will enhance stability of the existing dam areas following completion of the reclamation. The water level within each former impoundment will be near the original ground surface and very little of the upstream dam face will be under saturated conditions. A potential failure surface along the upstream face is no longer considered applicable. Drainage channel outlets in the dams will assure complete drainage of runoff from the tailings and, in conjunction with the clay cap, will prevent resaturation of the tailings. The slope of the outslope areas is currently 2.5 ft horizontal to 1 ft vertical and will be reduced to a maximum of 5 ft horizontal to 1 ft vertical.

A slope stability analysis for the No. 5 dam was conducted by Dames and Moore (1976) prior to construction of the dam. The Modified Bishop method was used for this analysis. Static and earthquake (a=0.025g) loading conditions gave factors of safety of 2.0 and 1.8, respectively, for the maximum section. The static and earthquake loading conditions gave factors of safety of 1.8 and 1.7, respectively, for the end section. Given the dramatic increase in stability that will occur with modifications under the reclamation plan, the potential for slope failures on the dam

outslopes does not appear to be a concern. The existing dam has met NRC Regulatory Guide 3.1.1 criteria for dam stability and will become significantly more stable with reclamation. Refer to additional seismic analysis conducted and reported by Inberg-Miller Engineers under title "SLOPE STABILITY & LIQUEFACTION POTENTIAL ANALYSIS - PROPOSED TAILINGS RECLAMATION, SHIRLEY BASIN MINE SITE, SHIRLEY BASIN, WYOMING".

The resistance of the cover materials to dispersion is not considered an issue for the tailings surface. In general, the susceptibility of the clay radon/infiltration barrier material to dispersion is not considered a significant problem because additional cover of sandy capillary barrier and rock mulch and filter or topsoil will be present. Crumb dispersibility tests were conducted on four clay cover samples (STH-4, 30'-37'; STH-6, 20'-27'; STH-5, 40'-47'; and STH-2, 60'-67') and indicated that the clay was not dispersive. The results of the test are included in Appendix C.

The potential for damage of the topsoil resulting from dispersion is considered very low. A significant portion of each topsoil sample was sand, and this should make the soils resistant to dispersion. On the tailings surface, the topsoil will only be placed on areas of very mild slopes and the probability of piping or erosion of soil is considered very low.

The potential for flooding of the tailings by Spring Creek under PMF conditions was evaluated using the HEC-1 model as described in Section 5. The drainage area for Spring Creek is 27.7 square miles and the flow is routed through a culvert and around the Area 3 pit in the Spring Creek Diversion. Under PMF conditions, the culvert and diversion would most likely fail, and there would not be backwater conditions at the culvert. The one-hour 1 square mile PMP for Shirley Basin is 9.03 inches. The

channel length and elevation change for the drainage is 24,550 feet and 498 feet, respectively, giving a time of concentration of 1.4 hours. The resulting peak flow using the HEC-1 model and a SCS curve number of 75 is 86,750 cfs. When the SCS curve number is increased to 90, the peak flow increases to 112650 cfs.

The Spring Creek floodplain was approximated as a three-stage compound channel. The most constrictive portion of the Spring Creek channel appears to be the section northeast of the No. 5 dam. A ridge northeast of the channel and the No. 5 dam southwest of the channel form the narrowest section of floodplain. Two floodplain cross sections were taken in this region and approximated as the three-stage compound channels. The details of the channel geometry are included in Tables 6-1 through 6-4 (looking upstream). Using a Manning's n value of 0.02, the resulting flow depth for two cross sections and two flow depths ranges from 17.5 feet to 19.5 feet. This places the maximum flood stage approximately a minimum of 300' from the covered tailings under the most severe runoff conditions. The less severe combination of cross section and discharge places the maximum flood stage approximately 450 feet from the covered tailings.

TABLE 6-1. PMF ANALYSIS FOR SPRING CREEK CROSS SECTION 1 AT 11265Ø CFS.

MULTI-COMPOUND CHANNEL:

Channel- SPRING CREEK Storm-PMP Yrs., 1 hr.

INPUT:

DISCHARGE = 11265ØCFS SLOPE = .00300 FT./FT. MANNING'S ROUGHNESS COEFFICIENT = . Ø2ØØØ

RIGHT SIDE SLOPE OF FIRST STAGE = 7.00 :1 LEFT SIDE SLOPE OF FIRST STAGE = 13.00 :1 BOTTOM WIDTH OF FIRST STAGE = .00 FT. = 3.000 FT. DEPTH OF FIRST STAGE RIGHT SIDE SLOPE OF SECOND STAGE= 8.00 :1 LEFT SIDE SLOPE OF SECOND STAGE = 24.00 :1 BOTTOM WIDTH OF SECOND STAGE = 60.00 FT. DEPTH OF SECOND STAGE = 7.500 F1.

STAGE = 300.00 FT.

RIGHT SIDE SLOPE OF THIRD STAGE = 26.00 :1 LEFT SIDE SLOPE OF THIRD STAGE = 53.00 :1 BOTTOM WIDTH OF THIRD STAGE = 300.00 FT.

OUTPUT:

=> AREA $= 7362.63 \text{ FT.}^2$

=> WETTED PERIMETER OF STAGE1 = 60.3 FT.

=> WETTED PERIMETER OF STAGE2 = 240.6 FT.

=> WETTED PERIMETER OF STAGE3 = 713.1 FT. => WETTED PERIMETER = 1014.0 FT.

= 7.261 FT.=> HYDRAULIC RADIUS

=> TOP WIDTH OF FLOW = 1012.81 FT. = 19.523 FT.

=> TOTAL DEPTH OF FLOW

=> FROUDE NUMBER = 1.000 => AVERAGE VELOCITY = 15.30 FT./SEC. => SHEAR USED IN DESIGN = 1.36 LBS/FT^2

=> SHEAR STRESS COEFFICIENT = 1.000

TABLE 6-2. PMF ANALYSIS FOR SPRING CREEK CROSS SECTION 1 AT 86750 CFS.

MULTI-COMPOUND CHANNEL: Channel- SPRING CREEK

Storm-PMP Yrs., 1 hr.

INPUT:

= 8675Ø.ØØ CFS DISCHARGE SLOPE = .00300 FT./FT. MANNING'S ROUGHNESS COEFFICIENT = .Ø2ØØØ RIGHT SIDE SLOPE OF FIRST STAGE = 7.00 :1 LEFT SIDE SLOPE OF FIRST STAGE = 13.00 :1 .ØØ FT. BOTTOM WIDTH OF FIRST STAGE $= 3.0000 \, \text{FT}.$ DEPTH OF FIRST STAGE RIGHT SIDE SLOPE OF SECOND STAGE= 8.00 :1 LEFT SIDE SLOPE OF SECOND STAGE = 24.00 :1 BOTTOM WIDTH OF SECOND STAGE = 60.00 FT. = 7.500 FT. DEPTH OF SECOND STAGE TOP WIDTH OF SECOND STAGE = 300.00 FT. RIGHT SIDE SLOPE OF THIRD STAGE = 26.00 :1 LEFT SIDE SLOPE OF THIRD STAGE = 53.00 :1 BOTTOM WIDTH OF THIRD STAGE = 300.00 FT.

OUTPUT:

=> AREA = 6006.71 FT.^2

=> WETTED PERIMETER OF STAGE1 = 60.3 FT.

=> WETTED PERIMETER OF STAGE2 = 240.6 FT.

=> WETTED PERIMETER OF STAGE3 = 601.1 FT.

=> WETTED PERIMETER = 902.0 FT.

=> HYDRAULIC RADIUS = 6.659 FT.

=> TOP WIDTH OF FLOW = 900.85 FT.

=> TOTAL DEPTH OF FLOW = 18.106 FT.

=> FROUDE NUMBER = .986

=> AVERAGE VELOCITY = 14.44 FT./SEC.

=> SHEAR USED IN DESIGN = 1.25 LBS/FT^2

=> SHEAR STRESS COEFFICIENT = 1.000

TABLE 6-3. PMF ANALYSIS FOR SPRING CREEK CROSS SECTION 2 AT 11265Ø CFS.

MULTI-COMPOUND CHANNEL:

Storm-PMP Yrs., 1 hr. Channel- SPRING CREEK

INPUT:

DISCHARGE = 11265Ø CFS SLOPE = .00300 FT./FT. MANNING'S ROUGHNESS COEFFICIENT = .02000 RIGHT SIDE SLOPE OF FIRST STAGE = 12.00 :1 LEFT SIDE SLOPE OF FIRST STAGE = 8.00 :1 BOTTOM WIDTH OF FIRST STAGE = .00 FT. DEPTH OF FIRST STAGE = 5.0000 FT.RIGHT SIDE SLOPE OF SECOND STAGE= 28.00 :1 LEFT SIDE SLOPE OF SECOND STAGE = 24.00 :1 BOTTOM WIDTH OF SECOND STAGE = 100.00 FT. DEPTH OF SECOND STAGE $= 5.000 \, \text{FT}.$ = 360.00 FT. TOP WIDTH OF SECOND STAGE RIGHT SIDE SLOPE OF THIRD STAGE = 14.00 :1

LEFT SIDE SLOPE OF THIRD STAGE = 46.00 :1 BOTTOM WIDTH OF THIRD STAGE = 360.00 FT.

OUTPUT:

 $= 7010.65 \text{ FT.}^2$ => AREA => WETTED PERIMETER OF STAGE1 = 100.5 FT. => WETTED PERIMETER OF STAGE2 = 260.2 FT. => WETTED PERIMETER OF STAGE3 = 536.5 FT. => WETTED PERIMETER = 897.2 FT. => HYDRAULIC RADIUS = 7.814 FT. => TOP WIDTH OF FLOW = 896.03 FT. => TOTAL DEPTH OF FLOW = 18.934 FT. => FROUDE NUMBER = 1.012 => AVERAGE VELOCITY = 16.07 FT./SEC. => SHEAR USED IN DESIGN = 1.46 LBS/FT^2

=> SHEAR STRESS COEFFICIENT = 1.000

TABLE 6-4. PMF ANALYSIS FOR SPRING CREEK CROSS SECTION 2 AT 86750 CFS.

MULTI-COMPOUND CHANNEL:

Channel- SPRING CREEK

Storm-PMP Yrs., 1 hr.

INPUT:

DISCHARGE = 8675Ø.ØØ CFS SLOPE = .00300 FT./FT. MANNING'S ROUGHNESS COEFFICIENT = .02000 RIGHT SIDE SLOPE OF FIRST STAGE = 12.00 :1 LEFT SIDE SLOPE OF FIRST STAGE = 8.00 :1 BOTTOM WIDTH OF FIRST STAGE = .ØØ FT. = 5.000 FT. DEPTH OF FIRST STAGE RIGHT SIDE SLOPE OF SECOND STAGE= 28.00 :1 LEFT SIDE SLOPE OF SECOND STAGE = 24.00 :1 BOTTOM WIDTH OF SECOND STAGE = 100.00 FT. DEPTH OF SECOND STAGE $= 5.000 \, \text{FT}.$ TOP WIDTH OF SECOND STAGE = 360.00 FT. RIGHT SIDE SLOPE OF THIRD STAGE = 14.00 :1 LEFT SIDE SLOPE OF THIRD STAGE = 46.00:1 BOTTOM WIDTH OF THIRD STAGE = 360.00 FT.

OUTPUT:

=> AREA = 5748.44 FT.^2

=> WETTED PERIMETER OF STAGE1 = 100.5 FT.

=> WETTED PERIMETER OF STAGE2 = 260.2 FT.

=> WETTED PERIMETER OF STAGE3 = 447.4 FT.

=> WETTED PERIMETER = 808.2 FT.

=> HYDRAULIC RADIUS = 7.113 FT.

=> TOP WIDTH OF FLOW = 807.10 FT.

=> TOTAL DEPTH OF FLOW = 17.452 FT.

=> FROUDE NUMBER = .997

=> AVERAGE VELOCITY = 15.09 FT./SEC.

=> SHEAR USED IN DESIGN = 1.33 LBS/FT^2

=> SHEAR STRESS COEFFICIENT = 1.000

7.Ø GROUND-WATER CONSIDERATIONS

Ground-water restoration on the east side of the No. 5 tailings area in the Surficial aquifer started in 1984. Collection of impacted water was the initial approach to clean up the seepage in this area. Fresh-water injection into a horizontal recharge line downgradient of the collection has been in progress since 1986. Fresh-water injection, in combination with the collection, is gradually restoring the ground-water quality downgradient of the No. 5 tailings. The average rate of injection in 1992 was 69 gpm and the average rate of collection was 29 gpm.

Operation of the collection and injection system to restore the ground water downgradient of the No. 5 tailings in the Surficial aquifer will continue for several more years. The results of monitoring of wells in this area will be used to determine when the aquifer is adequately restored. The latest report on the fresh-water injection and collection system was submitted to the NRC in February 1993. Dewatering of the tailings is a key part of the restoration of the ground water in the Surficial aquifer. Dewatering of the tailings will begin in late 1993 and will be attempted to be dewatered at a similar rate to the decline in the ponded water. Collection of the tailings solution in the tailings will continue for several years. Collection wells were installed in the summer of 1993 Restoration of the in the No. 4 and 5 tailings. aquifer under the tailings will be conducted as the tailings are dewatered. Pathfinder's present monitoring program, established under license conditions to define changes in the ground-water quality adjacent to the tailings, will be used to define ground-water quality changes in the future. With the dewatering of tailings, seepage of tailings solution into the Surficial aquifer will Recharge of the Surficial aquifer through the tailings will be prevented by the clay infiltration barrier on the tailings. Section 5.5 presents estimates of infiltration into the clay barrier. This evaluation shows that infiltration through the barrier should not occur during the PME event. The 2.5 to 3 feet of low permeability material should be very adequate in restricting infiltration into the reclaimed tailings.

8.0 TAILINGS AREA/MILL RECLAMATION VOLUME ESTIMATES

The following estimates of volumes are provided to allow preliminary assessment of the cost of reclamation. Upon plan approval, a more detailed volume analysis will be performed for bonding purposes.

Estimated Volume

Rock	
Large Riprap Intermediate Riprap Small Riprap Rock Mulch Filter	16400 yd ³ 7700 yd ³ 16400 yd ³ 89500 yd ³ 119200 yd ³ 249200
Contaminated Excavation Low-Grade Waste Pile Ore Pad Southwest Area Windblown Tailings	454000 yd ³ 70000 yd ³ 96800 yd ³ 67000 yd ³ 687800
Contaminated Fill Pond 4 Pond 5	391000 yd ³ 290000 yd ³ 681000
Uncontaminated Cut Area 2/8 Channel Cut North Area Channel Cut Pond 4 Channel Cut Freshwater Pond Channel Cut	228000 yd ³ 100000 yd ³ 69000 yd ³ 70000 yd ³ 467000
Mill Area Fill	70000 yd³
Radon Barrier Clay Sand	1158000 yd ³ 215000 yd ³ 1374000
Topsoil	318500 yd^3

Some of the volume estimates are somewhat speculative. In particular, the cleanup depths are estimates and the contaminated excavation volumes could change a great deal. Also, the topsoil volume estimate includes partial replacement in windblown tailings areas and this may not be necessary.

The primary distinction between general earth volumes is that of contaminated or uncontaminated material. The contaminated materials includes the low-grade waste pile, ore pad cleanup, windblown cleanup, and southwest area cleanup. The following estimation techniques were used in deriving the volume estimates:

Windblown Tailings Cleanup - 83 acres @ 0.5 feet excavation.

Ore Pad Cleanup - 14.7 acres @ 4 feet excavation.

Southwest Area Cleanup - 100 acres @ 0.5 feet excavation.

Topsoil volume estimates are based on a general placement thickness of 0.67 feet with 0.5 feet replacement in the windblown area. The following areas were used in developing topsoil volume estimates:

Windblown Area Replacement - 83 acres @ 0.5' thickness

Southwest Area Replacement - 130 acres @ 0.67' thickness

Tailings Area Placement - 102.7 acres @ 0.67' thickness

The radon barrier volumes are based on areas as presented in Exhibit 10-3. The volume estimates do not include grading estimates for the No. 5 dam outslope, access road recontouring, or tailings recontouring. The No. 5 dam outslope grading is intended to be approximately a cut/fill balance.

The fill areas within the tailings are the southwest side of the No. 4 pond and the base of the existing No 5. pond. These fill estimates are based on pond topography determined from soundings and are subject to some adjustment.

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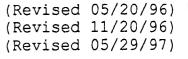
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(Revised 05/20/96) (Revised 11/20/96) (Revised 05/29/97)

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A.1 TEST HOLE LITHOLOGIC LOGS

The lithologic logs that were developed during the drilling of test holes and wells in the mill and tailings area are presented in Appendix A in Section A.1. Lithologic logs for wells that were located in No. 3, No. 4, and No. 5 tailings areas are presented initially, followed by those placed in the South and North overburden piles. Lithologic logs from wells in Tailings No. 3, No. 4, and No. 5 are listed with the prefix TW3, TW4, and TW5, respectively. Wells NS-1 and NS-2 are located on the northeast edge of the No. 5 tailings area. More than one lithologic log is presented on one page to save space. Table A.1-1 of Appendix A presents the lithologic log for sites TW3-1, TW4-1B, and TW4-1C.

Several test holes were drilled in the South and North overburden pile areas to define material properties of these two piles for cover material. The lithologic logs for these test holes follow the tailings logs. STH-1 through STH-11 sites are located on the South overburden pile. NTH-1 through NTH-8 are located in the North overburden pile area.

Exhibit 10-1 presents the location of the test holes. Appendix C presents the materials laboratory results for samples taken from the test holes, while Appendix D gives the radiological laboratory reports.

The lithologic logs present a description of the cuttings that were observed during the drilling of these test holes. In general, five foot intervals were used to define the lithologic conditions.

Shelby tubes were used to collect undisturbed samples from the

tailings. Soil radium concentrations for the tailings were measured only from Shelby tube samples. The five foot log samples from some test holes in the overburden pile were used for radiological analysis.

TABLE A.1-1. LITHOLOGIC LOGS FOR TW3-1, TW4-1B, & TW4-1C.

WRLL: TW3-1 DATE: 3/5/93 DRILLER: NOWN ENGINEER: GLH

DEPTH	DESCRIPTION
0-5 5-15 15-20	TAN SAND, SILT AND CLAY VERY FINE TO VERY COARSE SAND AND CLAY RED CLAY AND VERY FINE TO VERY COARSE SAND
20-25	AND FINE GRAVEL TAN VERY FINE TO VERY COARSE SAND N/ 30% CLAY
25-30	RED CLAY W/ 40% TAN VERY FINE TO VERY COARSE SAND
30-45	TAN VERY FINE TO VERY COARSE SAND, LITTLE RED CLAY
45-50 50-55	VERY COARSE SAND W/ 20% RED CLAY VERY FINE TO VERY COARSE SAND
55-60 60-65	TAN AND RED CLAY W/ SOME SAND OLIVE CLAY
65-70 70-75	VRRY FINE TO VERY COARSE SAND W/ LITTLE CLAY OLIVE CLAY
WEL	

DRILL TIME LOG

12:30	ON SITE
13:45	PILOT TO 80'
14:25	START BRANING
15:20	STOP REAMING
15:35	CASING IN
15:40	START WASHING
15:43	START SAND, SAND TO 26'
16:00	FINISH W/ SAND, BENTONITE TO 7.5
16:05	START AIR
16:30	STOP AIR
16:45	OFF SITE

WELL: TM4-1B DATE: 3/2/93 DRILLER: NONN ENGINEER: TGM

DEPTH DESCRIPTION

Ø-5	FINE TO MEDIUM TAN SAND
Ø-5 3-5	SHELBY TUBE
5-10	FINE TO MEDION TAN AND GREY SAND
19-29	FINE TO COARSE GREY SAND
20-25	FINE TO COARSE GREY SAND. SOME SLIMES
25-30	FINE TO MEDIUM GRRY SAND. ≈ 50% SLIMES
30-35	BROWN CLAY - 30% FINE TO MEDIUM SAND
35-40	BROWN CLAY
40-45	BROWN CLAY = 20% FINE TO COARSE WHITE SAND
45-50	BROWN CLAY = 40% COARSE BLACK SAND
50-60	MEDIUM TO VERY COARSE ORANGE AND WHITE SAND,
	SOME BROWN CLAY
60-65	RED CLAYSTONE - 30% MEDIUM TO VERY COARSE
	SAND
65-68	RED AND WHITE CLAYSTONE

WELL: TW4-1C DATE: 3/2/93 DRILLER: NOWN ENGINEER: TGM

DEPTH DESCRIPTION

Ø-5	TAN MEDIUM SAND, SHELBY TUBE 3-5'
5-30	GREY FINE TO MEDIUM SAND
30-50	BROWN CLAY = 30% MEDIUM TO COARSE SAND
50-55	GREY FINE TO HEDION SAND = 10% BROWN CLAY

DRILL TIME LOG

12:30	SET OP - SET SHELBY TOBE @ 3-5
12:35	MIXING MOD
13:10	@ 68' WITH 5" DRAG BIT, GRTTING RRADY TO LOG
13:29	REAMING - 7 7/8" TRICONE
14:30	REAMED 12 1/4"
14:33	SRTTĪNG ČAŠING - 20′ #35
14:54	5″ CASING
15:21	ALL CASING IN
15:30	AIR LIFT, VERY DIRTY, 2-3 gra
15:56 16:10	ATR LIFT, VERY DIRTY, 2-3 gpm END AIR LIFT, 5+ gpm, FAIRLY CLEAN DRILLER'S LEAVING
10.10	NUTUUN O PROTING

DRILL TIME LOG

DRILLER'S ON SITE	
TRYING TO TAKE SHELBY TUBES	
€ 40'. 5" HOLR	
LOGGING	
RRANING 12 1/4" TRICONR	
SRTTING CASING	
CASING IN	
SAND IN	
	PRODUCING
PTHR SAND	Induction
	DRILLER'S ON SITE TRYING TO TAKE SHELBY TUBES 40'. 5" HOLE LOGGING REAHING 12 1/4" TRICONE SETTING CASING CASING IN SAND IN AIR LIFT. 12+ gpm. VERY DIRTY. FINE SAND END AIR LIFT, 12 gpm. CLEAN

TABLE A.1-2. LITHOLOGIC LOGS FOR TW4-2B & TW4-2C.

WELL: TW4-2B DATE: 2/26/93 DRILLER: NONN ENGINEER: TGM

DEPTH	DESCRIPTION	DRILL T	INE LOG
0-5 5-10 10-25 25-40 40-45	VERY FINE TO MEDIUM GREY SAND VERY FINE TO MEDIUM GREY SAND, SOME FINES FINE TO MEDIUM GREY SAND VERY FINE TO MEDIUM GREY SAND VERY FINE TO MEDIUM GREY SAND ≈ 20% BROWN CLAY	7:50 9:15 9:30 10:30 10:45 11:00	DRILLER'S ON SITE - WAITING FOR EQUIPMENT DRILLER'S SETTING UP TAKING SHELDY TUBE 9 95'- 5" PILOT LOGGING REAMING 9 5/6"
45-50	BROWN AND RED CLAY, ≈ 20% FINE TO MEDIUM SAND	11:54 12:08	€ 80°- KRRP PLOGGING BIT CIRCOLATING
50-55 55-60	BROWN CLAY, ≈ 20% FINE TO MEDIUM SAND MEDIUM TO VERY COARSE CLAYEY SAND - RED AND WHITE AND GREY SAND	12:22 13:29 13:38	ĞÊTTĬÑĞ ŘŘÁDY TO SET CASING CASING AND SAND IN 20′ #35 PERF ON BOTTOM AIR_LIFT, ≈ 10 gpm. CLOUDY, LOTS OF FINE
60-80	MEDIÚM TO VERY COARSE CLAYRY SAND - RED AND White Sand	14:12	SAND
80-90	HEDIUM TO VERY COARSE RED AND WHITE AND GREY SAND, SOME FINES		RND AIR LIFT. ≈ 15 gpm. SLIGHTLY CLOUDY, VERY LITTLE SAND
90-92	NEDIÚN TO VERY COARSE WHITE AND GREY SAND - SONE FINES		
92-95	BROWN CLAY		

WELL: TW4-2C DATE: 3/3/93 DRILLER: NONN RNGINEER: TGM

DEPTH	DRSCRIPTION
0-5 5-15 15-20	FINE TO MEDIUM GRRY SAND VERY FINE TO MEDIUM GRRY SAND VERY FINE TO MEDIUM GRBY SAND, ≈ 40% BROWN CLAY
20-40 40-45	BROWN CLAY BROWN CLAY AND MEDIUM TO VERY COARSE SAND (50/50)
45-47	BROWN CLAY AND MEDIUM TO VERY COARSE SAND

DRILL TIME LOG

14:30	SET UP - TAKING SHELBY TUBE 3-5', TAKE SHELBY TUBE 40-42
15:00 15:12 15:52	LOGGING REANING 12 1/4" CASING IN 40" - #25
16:20 16:42	AIR LIFT. 2 gpm. VERY DIRTY, LOTS OF SAND RNGINER LEAVING, DRILLER STAYING UNTIL 17:00
17:00	END AIR LIFT. START AGAIN TOMORROW
3/4/93	
7:30 7:45	DRILLER'S ON SITE AIR LIFT, 2 gpm. SLIGHTLY CLOUDY

TABLE A.1-3. LITHOLOGIC LOGS FOR TW4-3B & TW4-3C.

WELL: TW4-3B DATE: 3/4/93 DRILLER: NONN ENGINEER: TGM

DRPTH	DESCRIPTION	DRILL T	INR LOG
Ø-5 5-10	VERY FINE TO MEDIUM TAN SAND GREY TO BROWN SANDY CLAY. VERY FINE TO MEDIUM SAND	10:00 10:50 11:11	MOVING TO SITE DRILLING @ 80', 5" PILOT LOGGING
10-15	GREY TO BROWN CLAYRY SAND, FINE TO COARSE SAND	11:30 12:42	LOGGING REANING 7 7/8° @ 90'
15-30 30-50 50-55 55-65	VRRY FINE TO MEDIUM GREY SAND BROWN CLAY BROWN CLAY 3', BLACK CLAY 2'	14:02 14:44 15:35	RKAMING 7 7/8" @ 90' REANED 12 1/4" @ 90' 5" CASING IN ALL CASING IN
	CLAYRY SAND, MEDIUM TO VERY COARSE, ORANGE AND WHITE SANDS	15:40 16:20	AIR LIFT, VERY DIRTY. 3 gpm = 4 gpm. STILL PRODUCING FINE SAND END AIR LIFT DRILLER'S LEAVING
65-70	3' MEDIUM TO VERY COARSE ORANGE AND WHITE SAND, 2' RED CLAYSTONE	16:36 17:10	RND AIR LIFT DRILLER'S LEAVING
70-75	FINE TO VERY COARSE ORANGE AND WHITE SAND - 1-2' WHITE CLAY LENSE		
75-85	PINE TO VERY COARSE ORANGE AND WHITE SAND, THIN WHITE CLAY LENSE	^	
85-90 90-95	YRLLOW CLAY WHITE CLAY	•	

NELL: TN4-30 DATE: 3/5/93 DRILLER: NONN RNGINEER: GLE

DEPTH	DESCRIPTION	

9-5	TAN VERY FINE TO HEDIOM SAND W/ CLAY
5-10	GRRY TO TAN VERY PINE TO MEDION SAND
Ø-35	GRRY VERY FINE TO COARSE SAND
35-40	BROWN CLAY AND GREY VERY PINE TO MEDIUM SAND
10-50	BROWN CLAY W/ SAND PROBABLY FROM ABOVE -
	SAMPLES 40-50 NOT BAGGED

DRILL TIME LOG

8:30	PILOT START
9:30	DRILLING @ 50' - START LOGGER
9:40	FINISH LOGGING
9:50	START RRANING
10:35	STOP REALING
10:45	CASING IN HOLR TO ≈ 50'
10:48	START WASHING
19:54	START SAND
11:20	START AIR LIFTING
11:27	Q ≈ 10 gpm
12:10	Q ≈ 10 gpm STOP AIR ≈ 10 gpm

TABLE A.1-4. LITHOLOGIC LOGS FOR TW4-4B & TW4-4C.

WELL: TW4-4B DATE: 3/1/93 DRILLER: NONN ENGINEER: TGM

DRPTH	DESCRIPTION	DRILL TI	INE LOG
0-5 5-10 10-30 30-35	VERY FINE TO COARSE TAN SAND FINE TO MEDIUM GREY AND TAN SAND VERY FINE TO MEDIUM GREY SAND BROWN AND RED CLAYSTONE W/ 30% FINE TO MEDIUM SAND	7:20 8:30 9:00 9:15 9:40	DRILLER'S ON SITE, WARMING UP PUSHING SHELBY TUBES HIXED HUD ### 70'5" LOGGING
35-45	BROWN AND RED CLAYSTONE W/ 15% FINE TO MEDIUM SAND	10:00 10:01	HAD TROUBLE RODDING OUT - FINISHED LOGGING FILLED COMPRESSOR WITH WATER, WORKING ON RIG
45-65	GRRY CLAY W/ 30% MEDIUM TO VERY COARSE SAND	10:40	RIG RRADY
65-70	WHITE CLAYSTONE	11:28	© 70° 95/16° TRICONE, WASHING - CLAY STICKING SETTING CASING
	•	11:45	SETTING CASING
		11:56 12:20	CASING IN
	•	12:55	ATR LIFT, 15+ gpm END DEVELOPMENT, CLEAR, JUST A FRW SAND GRAINS

WELL: TW4-40 DATE: 3/1/93 DRILLER: NUNN ENGINEER: TGM

UKPTH	DESCRIPTION
0-5	YERY FINE TO COARSE TAN SAND
5-10	FINE TO MEDIUM GREY AND TAN SAND
10-15	PINE TO MEDIUM GREY SAND
15-20	PINE TO MEDIUM GREY SAND
20-25	VERY FINE TO MEDIUM GREY SAND
25-30	VERY FINE TO MEDIUM GREY SAND
30-35	RED CLAYSTONE W/ GREY SAND FROM ABOVE

DRILL TIME LOG

	A.M
13:00 13:20	RRADY TO DRILL - CAN'T, SET SHELBY TUBES
13:30	LOGGING
13:40	LOGGED - GETTING READY TO REAM
13:57	CHANGING BIT
14:20	€ 33' - 12 1/4" TRICONE
14:35	CASING IN. 20 OF \$25 SLOT 5", POURING SAND
15:00	AIR LIFT - SLIGHTLY CLOUDY, SOME SUN, 15+
	gpm RND AIR LIFT - STILL PRODUCING SMALL AMOUNT
15:30	RND AIR LIFT - STILL PRODUCING SHALL AMOUNT
	IN SAND .

TABLE A.1-5. LITHOLOGIC LOGS FOR TW4-5B & TW4-5C.

WELL: TW4-5B DATE: 2/26/93 DRILLER: NUNN ENGINEER: TGH

DRPTH	DESCRIPTION	DRILL TIME L	<u>og</u>
Ø-5	FINE TO COARSE TELLOW/TAN SAND, SMALL PRACTION, SMALL GRAVEL	15:00 ON 1 15:30 TAK	SITE ING SHRLBY TUBES, GOT 1 TUBE 5 1/2-8'
5-10	PINE TO COARSE YELLOW/TAN SAND, SMALL PRACTION, SMALL GRAVEL PINE TO COARSE GREY SAND	15:40 DRII	LLING
10-15	FINE TO COARSE GRRY SAND	16:10 LOG	D'-5" DRAG BIT GING
10-15 15-30 30-35 35-40	PÎNR TÔ MEDÎÛN ĞRRY SAND ≈ 10% PINES PINE TO MEDIUM GREY SAND	16:20 DRI	LLER'S LRAVING
35-49	GRRY CLAY W/ ≈ 30% MRDIAN TO COARSK SAND	2/27/93	
40-45 45-50	BROWN CLAY W/ ≈ 30% MEDIUM TO COARSE SAND REDDISH BROWN CLAYSTONE	7:25 AT	GATE
45-50 50-55	WHITE SAND. MEDIUM TO VERY COARSE, SMALL	7:50 ON 1	SITE
55-65	GRAVEL WHITE AND RED SAND, MEDIUM TO VERY COARSE,	9:05 STAI 10:00 STAI	RT RRANING RT CASING
	SNALL GRAVEL	10:15 CAS	ING TO ≈ 60.5°
65-70	LĪĞĦŦ ŦĀN CLAYSTONE - SANDS PROBABLY CONING PROM ABOVE	10:20 SAN 10:30 BEN	D TO 41.5°. 8-12-6 BAGS TONITE TO 31°. 8 BAGS
	thou about	10:40 STA	RT AIR LIPTING
		11:10 = 2 11:31 STO	gpm P AIR LIFTING
		11-91 910	n nin pitting

WELL: TW4-5C DATE: 2/27/93 DRILLER: NUNN ENGINEER: GLH

DRPTH_	DESCRIPTION	DRILL TI	NR LOG
0-5 5-15 15-20	TAN SANDY CLAY, FILL FINE TO COARSE GREY SAND VERY FINE TO COARSE GREY SAND W/ SOME RED SLINE	12:00 12:30 12:45 13:28	START DRILLING STOP DRILLING - LOGGING START REANING CASING IN
20-25 25-35 35-40 40-42	VERY FINE TO MEDIUM GREY SAND W/ SOME SLIMES VERY FINE TO COARSE GREY SAND GREY CLAY W/ SOME SAND VERY FINE TO COARSE SAND AND CLAY	13:30 13:35 14:00 15:00	START WASHING START SAND START AIR, ≈ 10 gpm STOP AIR

TABLE A.1-6. LITHOLOGIC LOGS FOR TW5-1B & TW5-1C.

NELL: TW5-1B DATE: 2/15/93 DRILLER: NONN ENGINEER: TGM

DEPTH	DRSCRIPTION	DRILL TIME LOG
9-2 3-5 5-15 15-49 49-69 69-65 65-75 75-89 89-85 85-99 99-93		12:00 ON SITE - TAKING 3 SAMPLES 12:38 THANING VALVES 12:45 DRILLING 13:04 0 40' 13:30 0 93' 13:35 LOGGING 14:00 PROBE STUCK 14:30 REAMING - 7 7/8" TRICONE 15:00 CLAY STICKING - HOLE MON'T CLEAN 15:30 REAMING - 12 1/4" TRICONE 16:30 FROZE UP - CAN'T KEEP PUMP THANED
		8:00 DRILLER'S ON SITE 8:40 AT WELL - DOZER CLEANING SHOW 10:50 BACKHOR CLEANING PITS 11:20 DRILLING - 12 1/4 TRICONE 12:20 € 80' 12:30 € 95' - CIRCULATING 13:00 SETTING 5 CASING 13:20 WASHING 13:30 POURING SAND 14:41 START AIR DEVELOPMENT 14:48 10+ gpm, MILEY, SOME FINE SAND 15:07 10+ gpm, SOME FINE SAND 15:12 END DEVELOPMENT, A LITTLE BIT OF VERY FINE SAND, 10+ gpm

WELL: TW5-1C DATE: 2/24/93 DRILLER: NONN ENGINEER: TGH

DEPTH DESCRIPTION	<u>-</u>
0-10 BROWN AND TAN FINE TO MEDIUM SAN	D
10-25 GREY FINE TO MEDIUM SAND	אט אט אס אט איי
25-30 GREY FINE TO MEDIUM SAND - GRADI	.NG TO BROWN
30-55 BROWN CLAY AND SILT	
55-60 BROWN CLAY - GRADING TO MEDIUM S	SAND

DRILL TIME LOG

15:30	SRT OP
15:38	DRILLING
15:56	• 20'
16:05	• 60', WAITING FOR LOGGER
17:00	LOGGED, DRILLER'S LEAVING

2/25/93

7:3Ø	DRILLER'S ON SITE, WARMING OP
8:55	DRILLING, REANING 7 7/8" TRICORE
9:18	DRILLING, REAMING 12 3/4" TRICONE
10:58	AIR LIFT, ~ 2 gpm, VERY MILKY, VERY LITTLE
11:36 12:08	SAND AIR LIFT. ≈ 3 gpm, STILL VERY MILKY RND AIR LIFT, STILL SLIGHTLY MILKY, ≈ 3 gpm

TABLE A.1-7. LITHOLOGIC LOGS FOR TW5-2B, TW5-2C, & TW5-3.

DEPTH DESCRIPTION COVER
TAILINGS. FINE TO MEDIUM TAN SAND
GREY TO TAN FINE TO MEDIUM SAND - TAILINGS
GRADING - FINE TO MEDIUM SAND TO BROWN CLAY
REDDISH BROWN CLAYSTONE
VERY FINE TO MEDIUM SAND - SOME SMALL GRAVEL
FINE TO COARSE SAND
WHITE CLAYSTONE - SAND CAVING FROM ABOVE 0-6" 0-10 10-35 35-40 40-50 50-73 73-76

ON SITE - PITS SLOUGHING, MOVED SOUTH ≈ 25 TRYING TO GET SHELBY TUBES - SAND TOO SOFT DELLING P 75 OF THE PROPERTY OF THE PROP 2/26/93 7:45 8:30 DRILLER'S ON SITE
START DEVELOPMENT - AIR LIFT. ≈ 5 gpm.
SLIGHTLY CLOUDY. SOME FINE SAND
STOP AIR LIFTING. ADD 20 2 PERF. PUSHED
DOWN AS FAR AS POSSIBLE
AIR LIFT. CLEARING UP. STILL SOME FINE SAND
END AIR LIFT. STILL PRODUCING SMALL AMOUNT
OF FINE SAND
MOVING TO TWS-2C 9:05 9:13 10:00

DEPTH DESCRIPTION FINE TO MEDIUM TAN SAND = 25% FINES; MIXED COVER AND TAILINGS
FINE TO MEDIUM GREY SAND = 15% FINES MEDIUM TO COARSE GREY SAND MEDIUM TO COARSE GREY SAND FINE TO MEDIUM GREY SAND = 25% FINES BROWN CLAY

DESCRIPTION DEPTH 0-2 2-4 4-6 6-8 5-10 10-15 COVER SHELBY TUBE SHELBY TUBE SHELBY TUBE SHELBY TUBE SHELBY TUBE TAN AND GREY: FINE TO MEDIUM SAND \$15% BROWN CLAY SAND. FINE TO MEDIUM SAND \$15% BROWN FINE TO MEDIUM GREY SAND FINE TO COARSE TAN SAND: SOME CLAY \$10% DRILL TIME LOG

10:20

DRILL TIME LOG

SETTING UP

DRILLING

DRILLING # 40'

TRYING TO LOG - PROBE WON'T WORK

REANING - 12 1/4" TRICONE # 43'

CASING IN 40', #25 SLOT, 5"-2' STUB ON TOP

POURING SAND

3 SACKS BENTONITE: INTERMITTENT AIR LIFT:

TRYING TO SETTLE SAND

AIR LIFT - 25 gpm - LOTS OF SAND, MILKY,

WON'T SKAL AT TOP

AIR LIFT - 25+ gpm

AIR LIFT - 30+ gpm; STILL PRODUCING FINE

SAND

END AIR LIFT 13:10 13:30 14:06 14:26

DRILL TIME LOG

ON SITE, SETTING OP PIT DOG, DRILLER STILL SETTING OP DRILLING W/ AIR & SAMPLING - SHRLBY TOBES MIXING MOD AND DRILLING DRILLING @ 20 CASING PLACING SAND, 30 BAGS - 21 BELOW SURFACE OUT OF SAND, LEAVE ONTIL MONDAY GOT 20 BAGS 8-12 SAND, PUT 14 IN HOLE 2/15/93 8:10 8:40 DRILLERS ON SITE
WARMING RODIPHENT
SAND 2' FROM SURFACE, TOPPED W/ 2 BAGS
BENTONITE
SAND DROPS WHEN AIR IS APPLIED, WATER
BUBBLING UP ANNULOS; ADDED 2 BAGS BENTONITE 10:00

TABLE A.1-8. LITHOLOGIC LOGS FOR NS-1 & NS-2.

WELL: NS-1 DATE: 3/8/93 DRILLER: NOWN ENGINEER: TGM

DEPTH	DESCRIPTION
0-5 5-10 10-15	SANDY CLAY, MIXED COVER CLAYRY SAND SANDY CLAY
15-20 20-25	SANDY CLAY, RED AND WHITE
25-50 59-55	BROWN AND RED CLAYSTONE WHITE CLAYSTONE
55-60 60-65 65-70	DPN AND WITTP CLAVCTONF
70-75	METTE SANDY CLAY, MEDIUM TO VERY COARSE SAND METTE SANDY CLAY W/ SMALL GRAVEL MEDIUM TO VERY COARSE SAND, LENSE OR RED CLAYSTONE
75-8Ø 8Ø-85 85-9Ø	MRDION TO COARSE SAND AND GRAVEL, SOME CLAY
90-95	MRDIUM TO COARSE ORANGE AND WHITE SAND WHITE SANDY CLAY, COARSE SAND
95-100	RED CLAYSTONE AND WRITE SANDY CLAY

DRILL TIME LOG

7:45 8:00	DRILLER'S ON SITE MOVING TO WELL LOCATION SETTING UP
8:3Ø	SRTTING UP
9:13 9:37 9:40	e 60°, 5° PILOT e 100', 5° PILOT LOGGING RRAMING 9 5/16° CASING IN
10:35	RKAMING 9 5/16" CASING IN
12:20 12:25	CARIL IN
12:38 13:14	4-5 gom. MILKY, PRODUCING SOME SAND
	RND AIR LIFT, * 3 gpm. CLEAR WITH A LITTLE BIT OF VERY FINE SAND

WELL: NS-2 DATE: 3/8/93 DRILLER: NONN ENGINEER: TGM

DEPTH	DESCRIPTION
0-5	BROWN CLAY
0-5 5-15 15-20	LIGHT BROWN CLAYSTONE, FIRM LIGHT BROWN CLAYSTONE (3'), WHITE SANDY SILTSTONE (2') WHITE SANDY SILTSTONE
20-25	HEITE SANDY SILTSTONE
25-30 30-35	WHITE SANDY SILTSTONE W/ SOME SMALL GRAVES WHITE SANDY SILTSTONE
35-40 40-45	SANDY SILTSTONE W/ VERY COARSE RED SAND SANDY SILTSTONE W/ VERY COARSE SAND AND SMALL GRAVEL
45-50 50-55	OLIVE CLAYSTONE
כנ-שכ	OLIVE CLAYSTONE W/ WHITE CLAY AND SILTSTONE

DRILL TIME LOG

13:45 14:01	e LOCATION, SETTING UP DRILLING
14:35 14:49	055 - LOGGING DUNPED 2 BAGS PLUG IN HOLE, REAMING 9 5/16"
15:37	TO 47 CASING IN
15:50 16:00	SAND IN
16:27 16:50	ATR LIFT, YERY DIRTY, 3-4 gpm END AIR LIFT, 3 gpm, SLIGHTLY CLOUDY DRILLER'S LEAVING

TABLE A.1-9. LITHOLOGIC LOGS FOR STH-1 & STH-1A.

WELL: STH-1 DATE: 4/12/93 DRILLER: NUNN ENGINEER: TGM

DEPTH DESCRIPTION

0-5 BROWN, SANDY CLAY 5-10 BLUISH GRRY CLAY, ≈ 30% BROWN, SANDY CLAY 10-40 BLUISH_DARK CLAY

NOTE: NO RETURNS 40'-70'

DRILL TIME LOG

12:30 ON SITK 16:30 DRILLER'S LEAVING: 0 40'

4/28/93

7:50 DRILLER'S ON SITE; SETTING UP

WRLL: STH-1A DATR: 4/12/93 DRILLER: HUNN RNGINEER: TGM

DEPTH DESCRIPTION

SHELBY TUBE SAMPLE 60-63.5 SAND ON BOTTOM, CLAY ON TOP 63.5-67 SAND - CLAY & DARK CARBONACEOUS SHALE, SHALE ON TOP

70-73 RED & GREY CLAY 73-76 BROWN CLAY 76-80 LIGHT TO DARK GREY CLAY

NOTE: NO RETURNS 49'-70'

DRILL TIME LOG 8:01 DRILLING 10:00 @ 60' - GRTTING TOBE SAMPLES 60'-67' 11:00 DONE @ 90' - LOGGING

TABLE A.1-1Ø. LITHOLOGIC LOGS FOR WELLS STH-2 & STH3.

WELL: STH-2 DATE: 4/28/93 DRILLER: NONN ENGINEER: TGM

DEPTH	DESCRIPTION
0-10 10-15 15-25	MIXED TAN & BROWN SAND W/ SOME GREY SHALE TAN & ORANGE SAND W/ SOME GREY CLAY TAN & WHITE FINE TO VERY PINE SAND, SOME CLAY
25-30 30-40 40-45 45-50	1/2 ABOVE: 1/2 GRAYISH BLUE CLAY GRAYISH BLUE CLAY GREY TO BLACK CLAY ADDING WATER
50-55 55-75 75-80	MIXED COARSE TO VERY COARSE SAND W/ SMALL PRACTION CLAY FROM ABOVE BLUISH GREY CLAY BLUISH GREY CLAY GRADING TO BROWN CLAY
80-85 85-90 90-95 95-100	TAN & ORANGE SAND. ~ 30% DARK COLORED CLAY VRRY PINE WHITE SAND FINE TO MEDION WHITE SAND. ~ 10% BLUE CLAY TAN & WHITE FINE TO COARSE SAND

DRILL TIME LOG

11:27 SAMPLR 10'-17' 12:40 SAMPLR 40'-47' 14:00 PUT WATER IN THROUGH RIG 14:30 TRIED TO LOG: HOLE CAVED € ≈ 15'; DRILLER

WELL: STH-3 DATE: 4/28/9: DEILLER: NOWN NGINEER: TGM

0-5 BROWN CLAY
5-40 BLUISH GRAY CLAY
CAVED IN - HAD TO MOVE

DRILL TIME LOG

15:00 DRILLING 15:30 SANPLE 0 20'

4/29/93 9:15 DRILLER'S @ 80' 9:50 DRILLER'S MOVING TO STH-4 TABLE A.1-11. LITHOLOGIC LOGS FOR STH-4, STH-5, & STH-6.

DESCRIPTION

DRILL TIME LOG

BROWN CLAY SOME COARSE SAND BLUE & BROWN CLAY BLUISH GRAY CLAY

11:10 HOVING TO STH-5

DESCRIPTION

DRILL TIME LOG

BROWN & TAN (OXIDIZED) SAND VERY FINE TO MEDIUM BROWN & TAN SAND W/ BLUE CLAY LENSES PREDOMINANTLY BLUE CLAY, SOME SAND RUNG CLAY

DESCRIPTION

DRILL TIME LOG

MOVED TO STH-7

16:00

BROWN & BLOR CLAY BLUE CLAY BLUE CLAY W/ SOME

LIGHT BROWN SAND W/ = 30% BLUE CLAY DARK CLAY & BROWN CLAY

TABLE A.1-12. LITHOLOGIC LOGS FOR STH-7, STH-8, & STH-9.

DRILL TIME LOG

DONE - MOVING TO STH-8

DEPTH DESCRIPTION

DRILL TIME LOG

LIGHT BROWN CLAY
BROWN CLAY
BLUK CLAY
BLUK CLAY W/ SAND LENSE, 1/2 & 1/2

BLUE CLAY CLAY - GRADING TO LIGHT COLORED COARSE TO YERY COARSE SAND FINE TO HEDIUM TAN & LIGHT BROWN SAND W/ BLUE CLAY BELOW

DESCRIPTION DEPTH

0-10 10-30 30-35

12:59 MOVING TO STH-10

TABLE A.1-13. LITHOLOGIC LOGS FOR STH-10 & STH-11.

WELL: STH-16 DATE: 5/3/93 DRILLER: NOWN RNGINEER: TGM

DRPTH DRSCRIPTION

DRILL TIME LOG

9-29 29-25 25-39 BROWN & BLUISH CLAY BRADING DARKER 1IXED CLAY: BLUK BROWN CL BLUISH GRAY CLAY

30-35 BLUISH GRAY CLAY 35-40 <u>Gray & Bro</u>wn Clay W/ Very Coarse Sand & 8:00 SET UP - START DRILLING

WELL: DATE: DRILLER:

DEPTH DESCRIPTION

DRILL TIME L

0-5 BROWN CLAY 5-10 BLUISH CLAY 10-15 DARK GRAY CLAY

10-15 DARK GRAY CLAY 15-30 GRAY & BLUISH CLAY 10:13 FOLDING DOWN; MOVING TO NORTH DUMP

TABLE A.1-14. LITHOLOGIC LOG FOR WELL NTH-1 & NTH-2.

NELL: NTH-1 DATE: 5/3/9: DRILLER: NONN ENGINEER: TGM

DEPTH DESCRIPTION

DRILL TIME LOG

0-20 LIGHT BROWN CLAYRY SAND 20-25 1/2 LIGHT BROWN SANDY CLAY, 1/2 BLUISE 25-90 BLUISH CLAY 90-95 SAND LENGE, VERY FINE TO MEDIUM TAN SA 95-115 BLUE CLAY W/ VERY FINE TO FINE SAND 15-120 THE TO COASER SAND W/ DUR OF AS

11:10 SETTING UP 12:21 @ 60' 14:42 HOVING TO NTH-2

WELL: NTH-2 DATE: 5/3/93 DRILLER: NOWN ENGINEER: TGM

DEPTH DESCRIPTION

DRILL TIME LOG

7-5 YERY FIRE LIGHT BROWN SAND W/ SOME SILT
5-10 LIGHT BROWN SILTY SAND
10-20 BLOUSH GRAY CLAY
20-25 VERY FIRE TO MEDIUM WHITE SAND
30-35 FIRE TO MEDIUM LIGHT BROWN SAND
30-35 FIRE TO VERY COARSE TAN SAND
830-35 FIRE TO VERY COARSE TAN SAND W/ STRINGER OF
BLUE CLAY
40-45 TAN & BROWN CLAYRY SAND
11-50 FIRE TO COARSE LIGHT BROWN SAND

15:30 0 50' DRILLER'S FUELING TRUCK; GETTING READY TO LEAVE

LITTLE CLAY

100-115 YERY FINE TO MEDIUM ORANGE SAND

TABLE A.1-15. LITHOLOGIC LOGS FOR NTH-3 & NTH-4.

WELL: NTH-3 DATE: 5/4/93 DRILLER: NONN RNGINERR: TGM

DEPTH	DESCRIPTION
0-5 5-10 10-15 15-20 20-25 25-35	GREENISH BROWN CLAY WHITE SANDY CLAY: YERY FINE SAND BROWN & TAN CLAY: SOME SAND DARK CARBONACKODS CLAY BLUISH CLAY - SHALE-LIKE STRUCTURE BLUISH CLAY - SHALE-LIKE STRUCTURE, SHADING DARKER
35-40 40-45 45-50	BLUISH CLAY - SHALE-LIKE STRUCTURE BLUISH CLAY - SHALE-LIKE STRUCTURE, SHADING DARKER VARY COARSE SAND, SOME CLAY AS ABOVE FINE TO COARSE SAND, SOME BLUE CLAY REDDISH BROWN SAND - FINE TO COARSE W/ SMALL GRAVEL
50-60	REDDISH BROWN SAND - FINE TO COARSE W/ SMALL GRAVEL: LIGHTER COLOR
60-80	GRAVEL: LIGHTER COLOR VERY FINE TO MEDIUM LIGHT REDDISH SAND, SOME CLAY
80-90	KINK TO VERY COLERSE LIGHT COLORED SEND -
90-95	SAALL GRAVEL FINE TO VERY COARSE LIGHT COLORED SAND - SMALL GRAVEL; GETTING FINER VERY FINE TO MEDIUM LIGHT COLORED SAND, SOME RED, CLAY
95-105	VERY FINE TO MEDIUM LIGHT COLORED SAND, SOME
105-120	FINE TO VERY COARSE ORANGISH SAND

DRILL TIME LOG

8:00 DRILLER'S SET UP 10:50 SAMPLING # 40' 12:20 MOVING TO NTH-4

WELL: NTH-4 DATE: 5/4/9: DRILLER: NOWN ENGINEER: TGM

DRPTH	DESCRIPTION
9-19 19-15	BROWN CLAY VERY FINE LIGHT CLAYEY SAND
19-15 15-35 35-40	DARK CLAY - RIOR TINT
40-50 50-70	VERY FINE TO MEDIUM GRAY SAND
79-75	FRACTION SAME AS ABOVE - MORE CLAY
75-80 80-85 85-105	MEDIUM TO VERY COARSE CLAYEY SAND
105-120	MEDIUM TO VERY COARSE LIGHT COLORED SAND MEDIUM TO VERY COARSE ORANGE SAND, SOME CLAY

DRILL TIME LOG

12:30 DRILLER'S RATING LUNCH 12:40 SETTING UP 15:45 PUBLING TRUCK, LEAVING

TABLE A.1-16. LITHOLOGIC LOGS FOR NTH-5 & NTH-6.

WELL: NTH-5 DATE: 5/5/9 DRILLER: NUNN ENGINEER: TGH

DRPTH	DESCRIPTION
0-15 15-20 20-60	VERY FINE LIGHT COLORED CLAYEY SAND 1/2 DARK CLAY - 1/2 LIGHT CLAYEY SAND DARK (BLOISH) CLAY
60-65 65-85	1/2 DARY CLAY_ 1/2 ORANGE CAND
90-95 95-110	LIGHT BROWN CLAYRY SAND LIGHT BROWN CLAYRY SAND, SOME ORANGE SAND
110-120	ORANGE SAND W/ ≈ 20% LIGHT COLORED CLAY

DRILL TIME LOG

8:00 DRILLBR'S SET UP 11:20 0 120: BAINING HARD, LIGHTNING 11:40 RAIN QUIT, BACK TO DRILLING 12:00 MOVING TO NTH-6

WELL: NTH-6 DATE: 5/5/93 DRILLER: NUMN ENGINEER: TGM

DRPTH	DESCRIPTION
Ø-1Ø 1Ø-15	VERY FINE TO FINE LIGHT COLORED CLAYEY SAND VERY FINE TO FINE LIGHT COLORED CLAYEY SAND: LENSE OF DARKER CLAYEY SAND
15-20	VERY FYNE TO FINE LIGHT COLORED CLAYEY SAND; 18-20 DARK CLAY - CARBONACEOUS
20-25 25-30 30-35	DARK CLAY - CARBONACKOUS - SHALRY DARK CLAY, ~ 30% FINE SAND DARK CLAY LRSS SAND
35-60 60-65 65-80	ĐẶRK CĂBBÒNĂCKOUS CLAY 4 DARK CLAY: 1 ORANGE CLAYEY SAND ORANGE CLAYEY SAND

DRILL TIME LOG

12:06 SETTING OP 13:45 DONE - GETTING READY TO MOVE

TABLE A.1-17. LITHOLOGIC LOG FOR NTH-7 & NTH-8.

WELL: NTH-7 DATE: 5/10/9 DRILLER: NONN ENGINEER: TGM

DEPTH DESCRIPTION

DRILL TIME LOG

0-20 BROWN 20-25 DARK, 25-75 DARK, 75-80 1 CLA

BROWN SANDY CLAY DARK, BLDISH CLAY & BROWN SANDY CLAY DARK, BLDISH CLAY 1 CLAY: 4 MEDION TO COARSE PINKISH SAN MEDIUM TO COARSE PINKISH SAND !:40 DRILLER'S SETTING UP 1:40 DONE - @ 130', TROUBLE W/ RETURN WOVING

WELL: NTH-8 DATE: 5/10/9: DRILLER: NONN ENGINEER: TGM

DEPTH DESCRIPTION

0-15 LIGHT BROWN SANDY CLAY 15-30 WHITE SILTY SAND 30-50 LIGHT BROWN SANDY CLAY 56-25 RENTER GROWN CLAY

5-100 1 CLAY AS ABOVE: 4' HEDIUM TO VERY COARSI

A.2 TEST HOLE GEOPHYSICAL LOGS

The geophysical logs for the test holes are presented in Section A.2 of Appendix A. Geophysical logs were developed for one test hole drilled in the No. 3 pond area, ten test holes on the No. 4 tailings area, and five test holes drilled in the No. 5 tailings area. One of the No. 5 test holes, NS-2, was drilled on the northeast side of the No. 5 dam. Geophysical logs for test holes OP-1 through OP-16 in the ore pad area are also presented in this section. Five geophysical logs (LGW) from the low grade ore pile on the west side of the No. 3 tailings were developed to define radiological conditions in this area. Concluding the section are two geophysical logs that were developed on the South overburden pile.

geophysical logs present а natural resistivity and SP. Resistivity and SP logs were not measured above the water level in the drill hole. The gamma log is very useful in the tailings in defining the lenses of slimes and sand layers. Depth in feet is presented on the left-hand scale of the geophysical log. The gamma log is generally presented on the left side of the log with increasing gamma values to the right. resistivity log is typically presented on the right side of the graphs as a solid line, while the SP is presented as a dashed line. Gamma activities of less than 6,000 CPS and greater than 1,000 CPS are typically indicative of the tailings sand. Gamma values greater than 6,000 CPS indicate a tailings slime. Gamma values from the test holes are also very useful in defining depths of

contaminated materials. For example, the log for test hole OP-6, which is presented in Figure A.2-19, shows that the ore stock pile pad in this region has elevated gamma values to a depth of 8'-9'. The material in this zone exceeds the site clean-up standard for RA-226 as demonstrated by the gamma log.

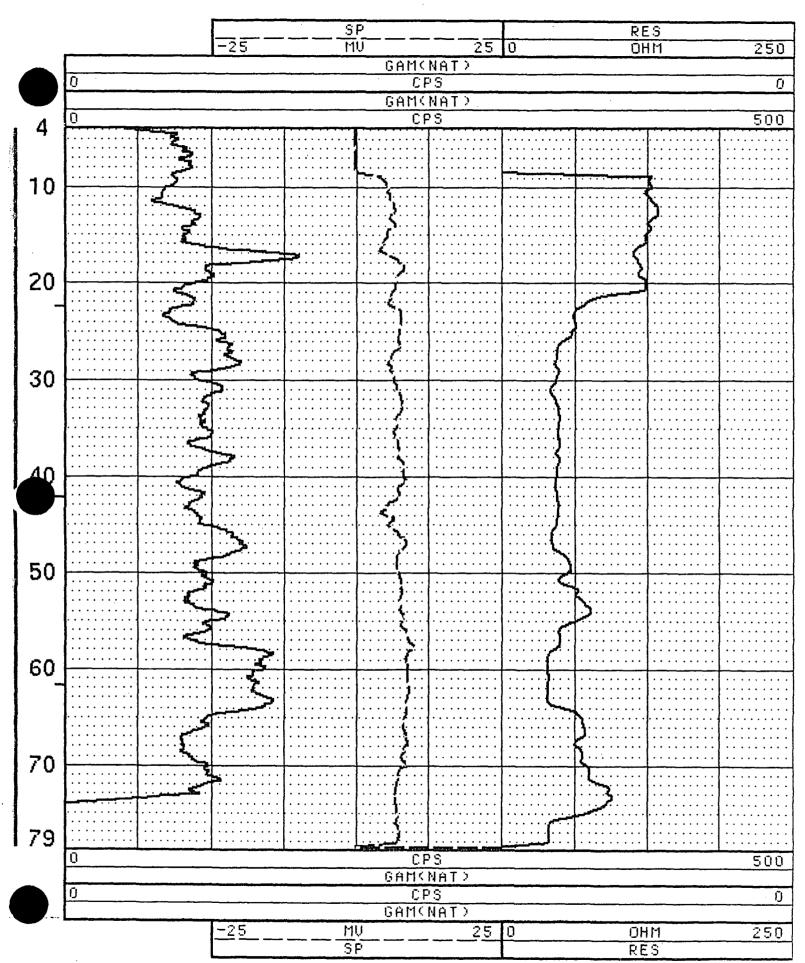


FIGURE A.2-1. GEOPHYSICAL LOG FOR TW3-1.

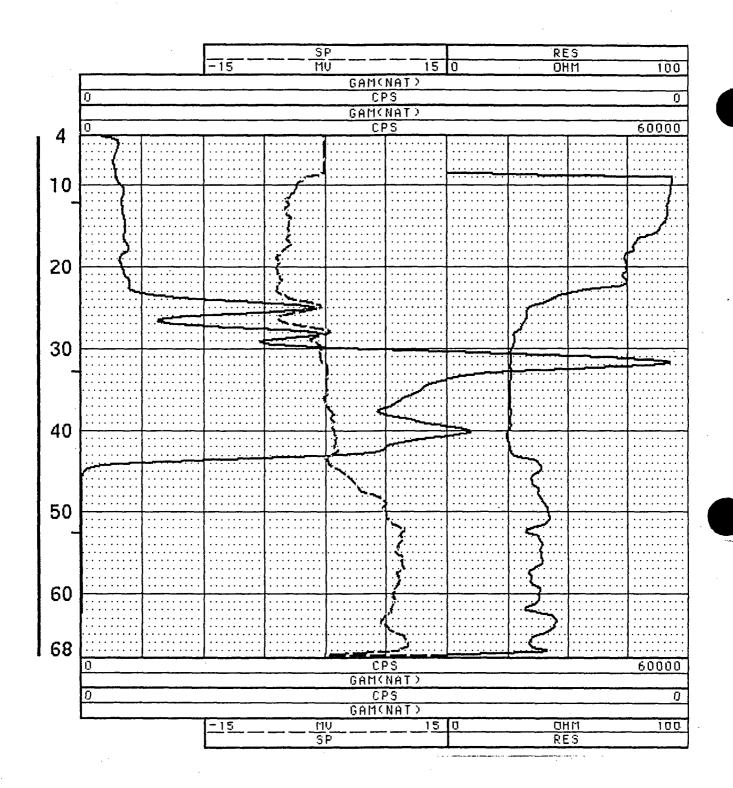
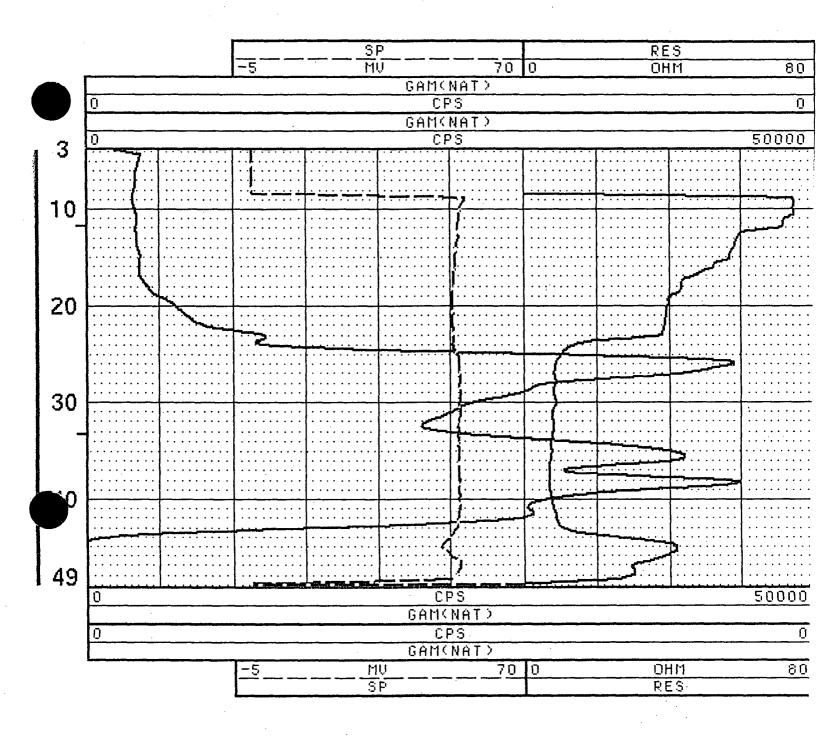


FIGURE A.2-2. GEOPHYSICAL LOG FOR TW4-1B.



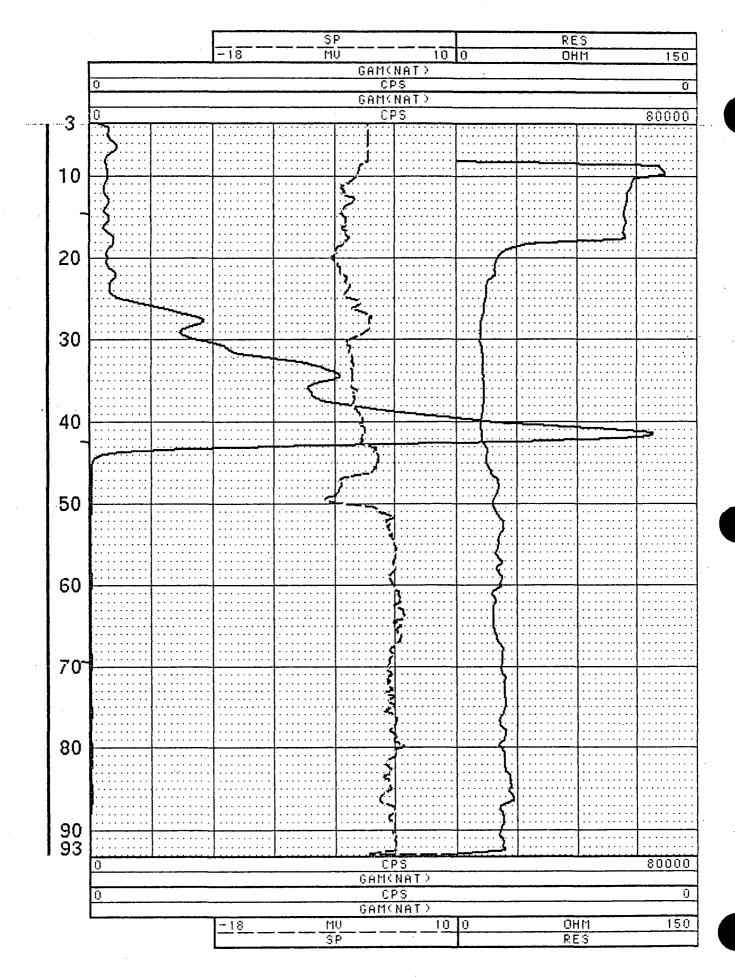


FIGURE A.2-4. GEOPHYSICAL LOG FOR TW4-2B.

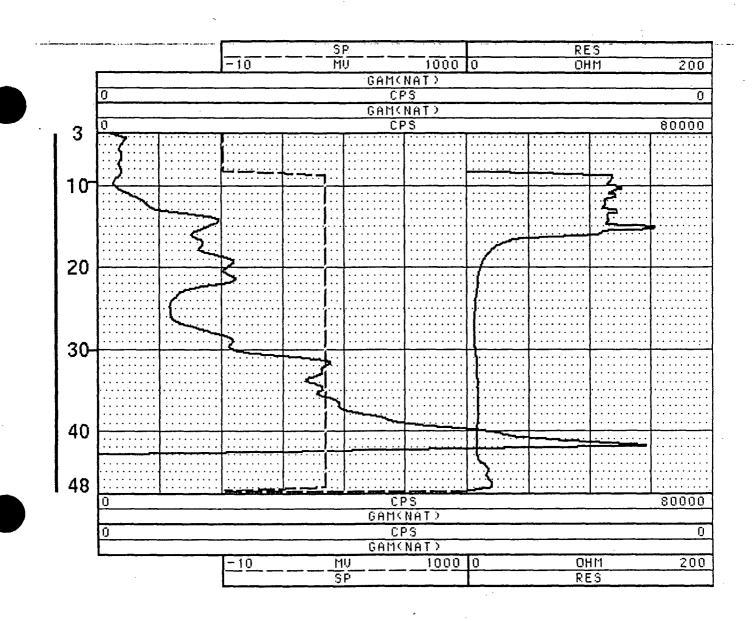


FIGURE A.2-5. GEPHYSICAL LOG FOR TW4-2C.

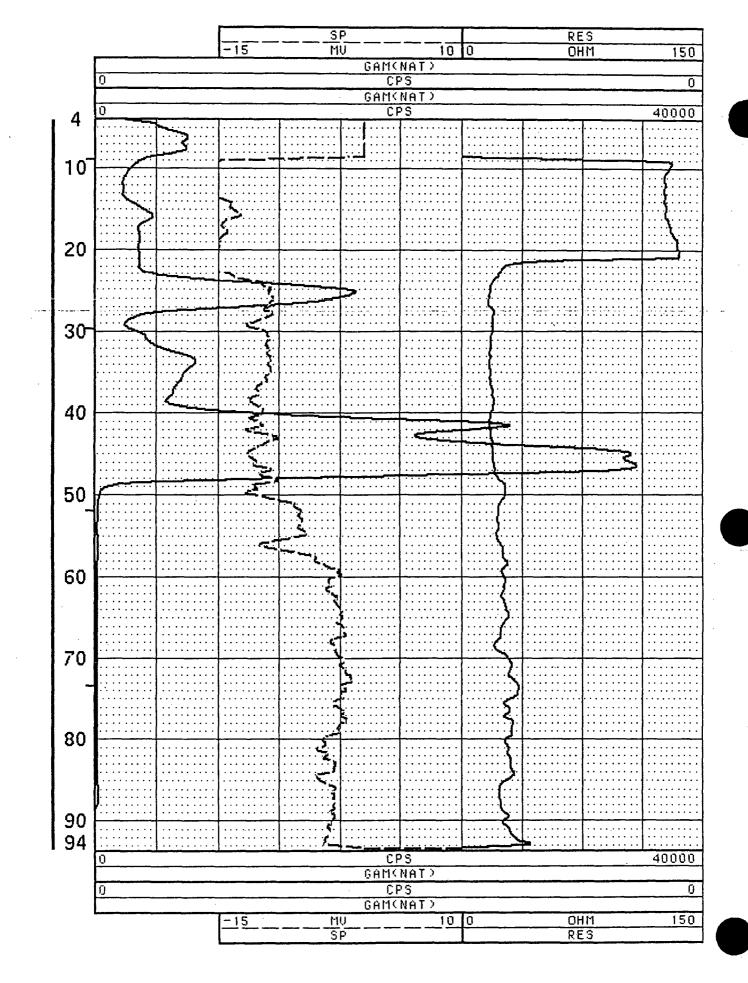
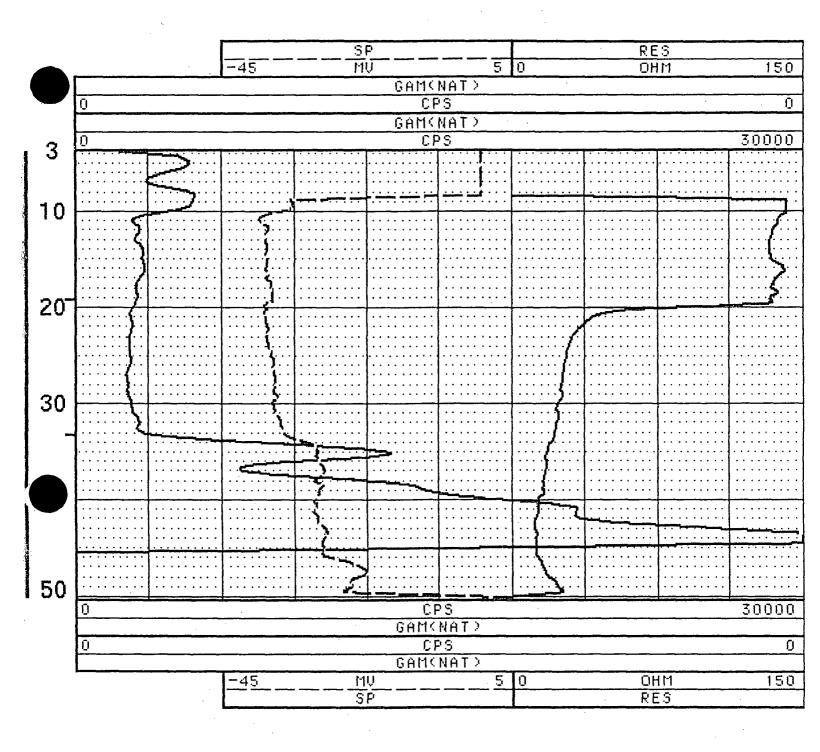


FIGURE A.2-6. GEOPHYSICAL LOG FOR TW4-3B.



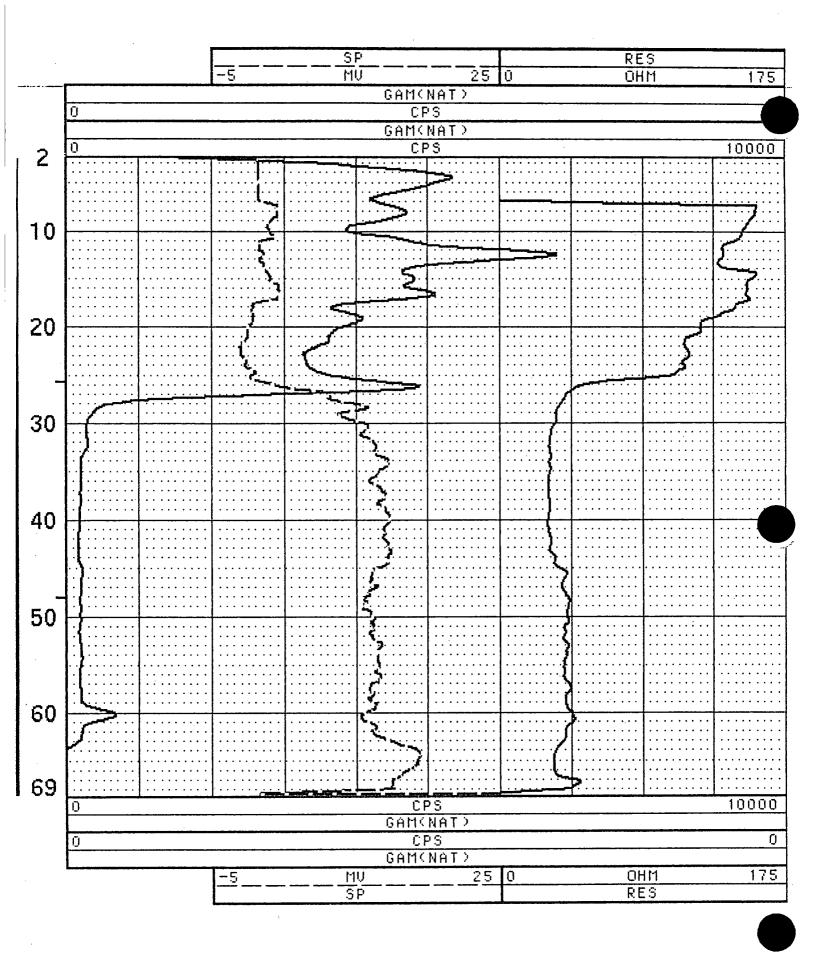
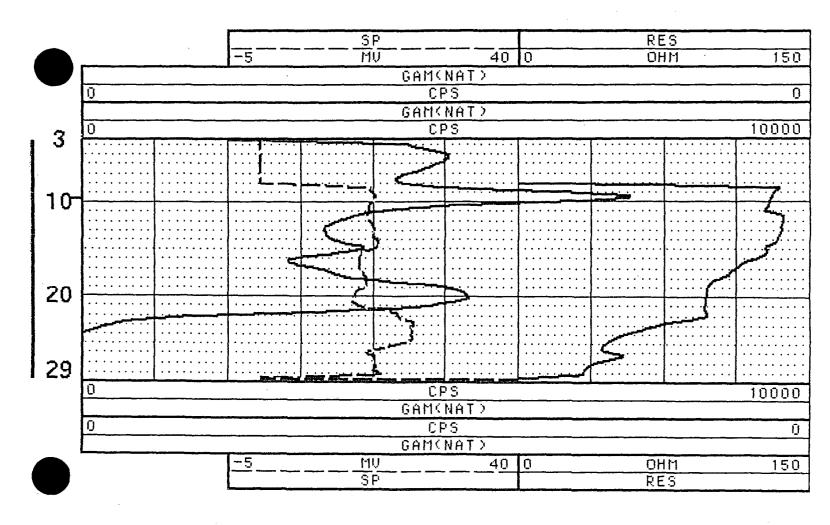


FIGURE A.2-8. GEOPHYSICAL LOG FOR TW4-4B.



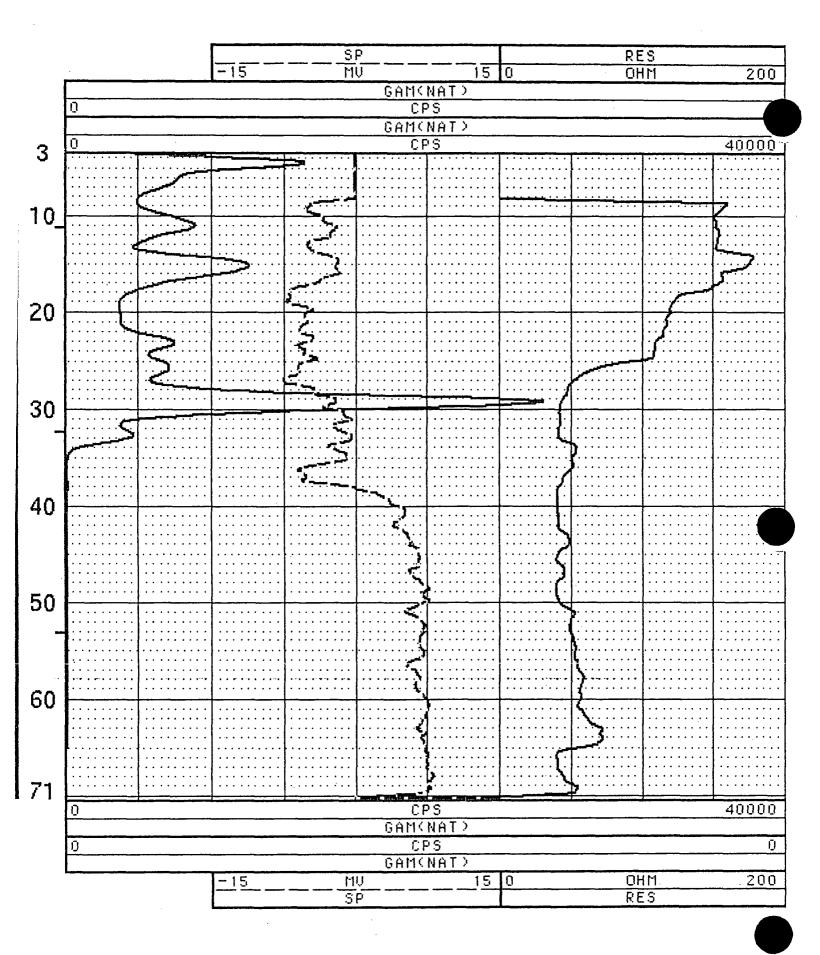
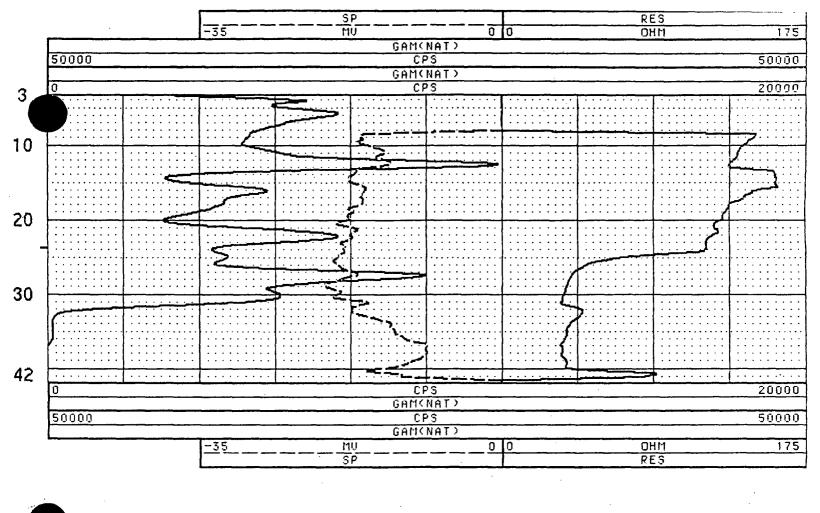
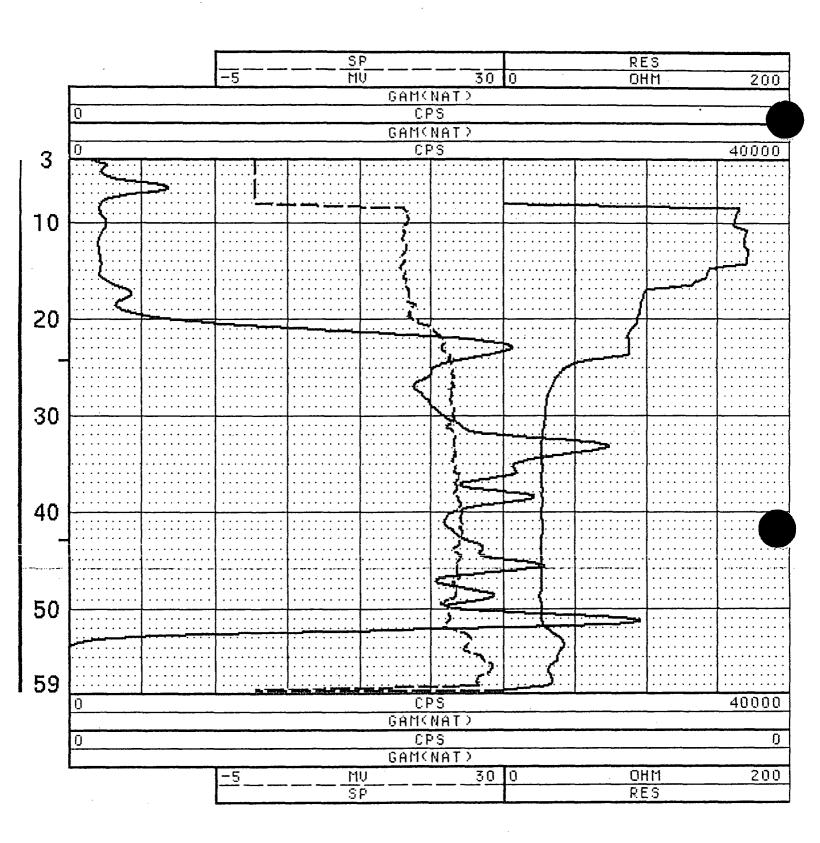


FIGURE A.2-10. GEOPHYSICAL LOG FOR TW4-5B.





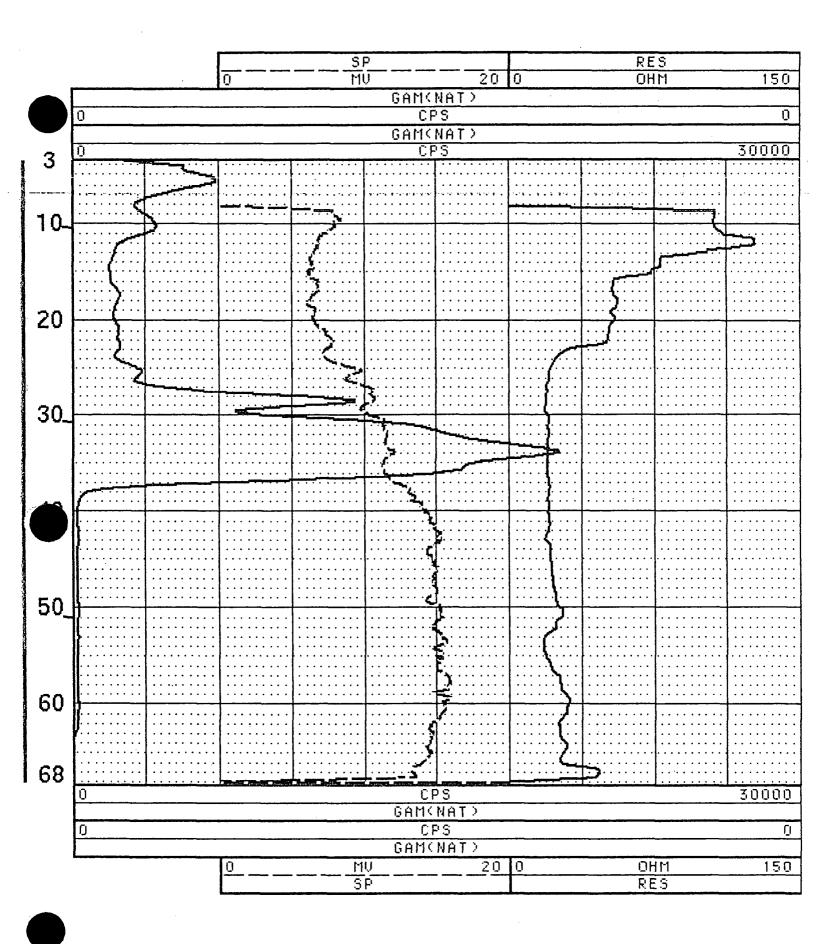
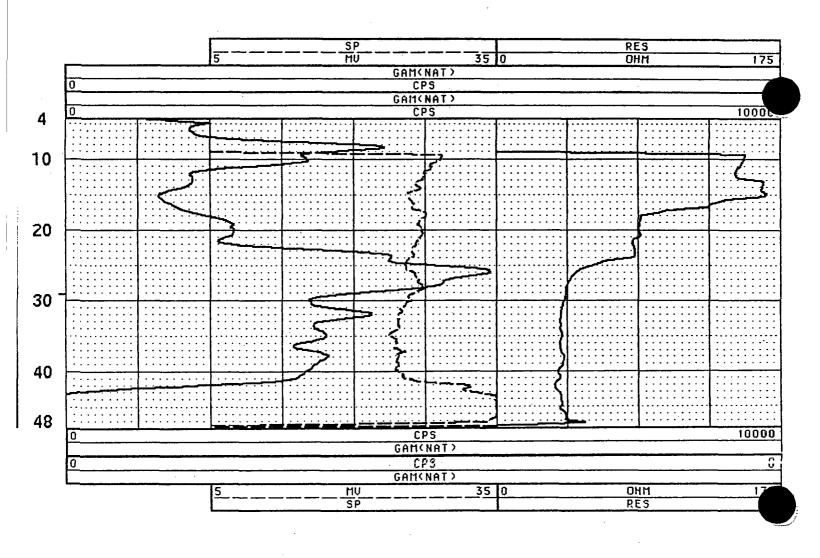


FIGURE A.2-13. GEOPHYSICAL LOG FOR TW5-2B.



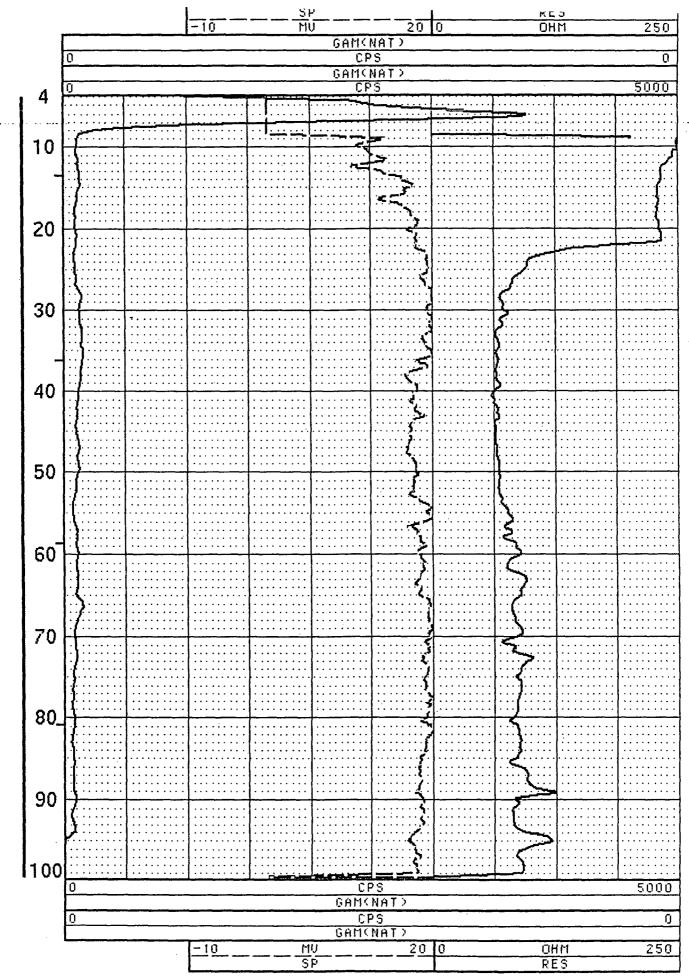
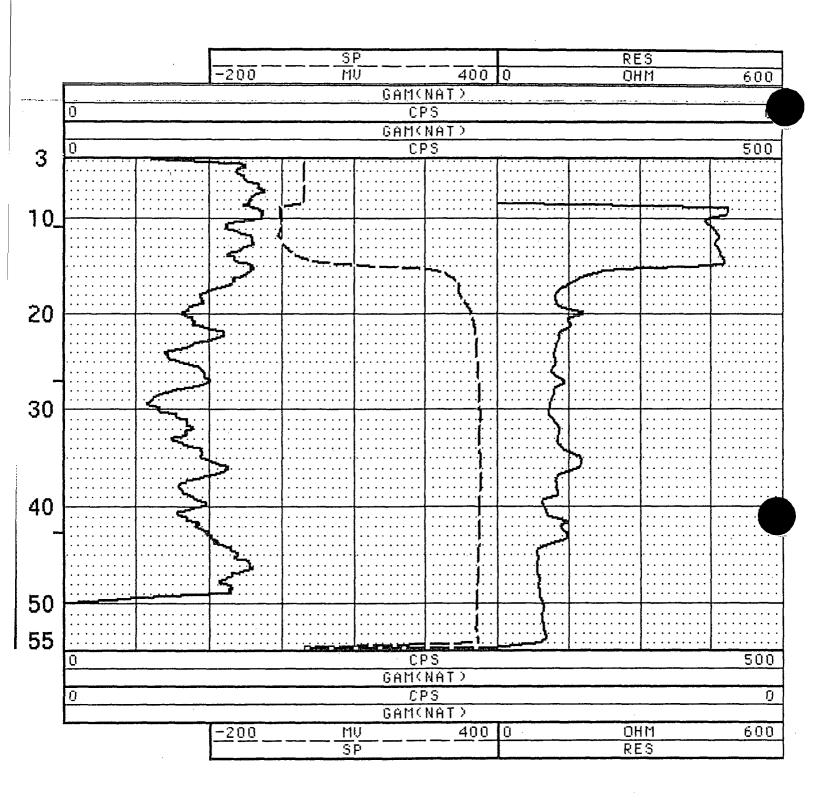
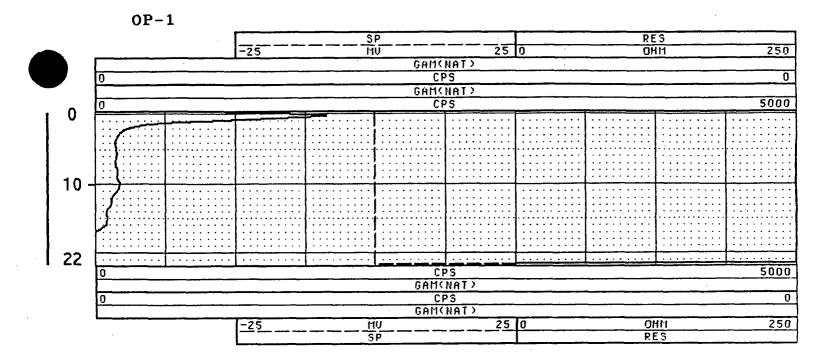


FIGURE A.2-15. GEOPHYSICAL LOG FOR NS-1.





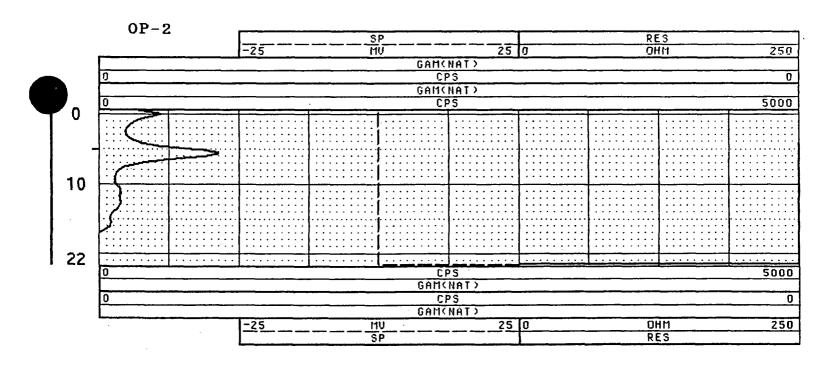
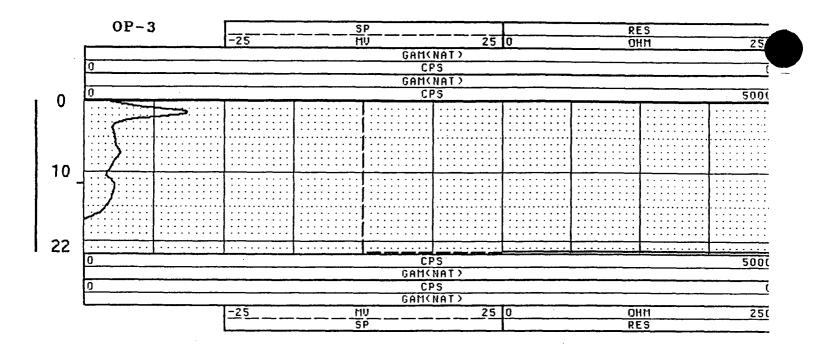
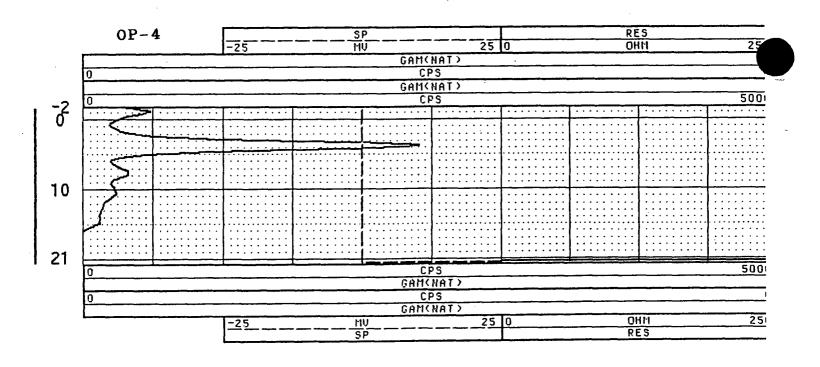
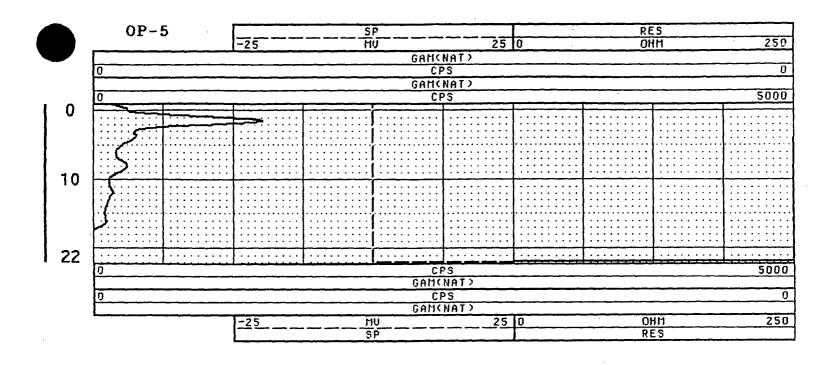


FIGURE A.2-17. GEOPHYSICAL LOGS FOR OP-1 & OP-2.







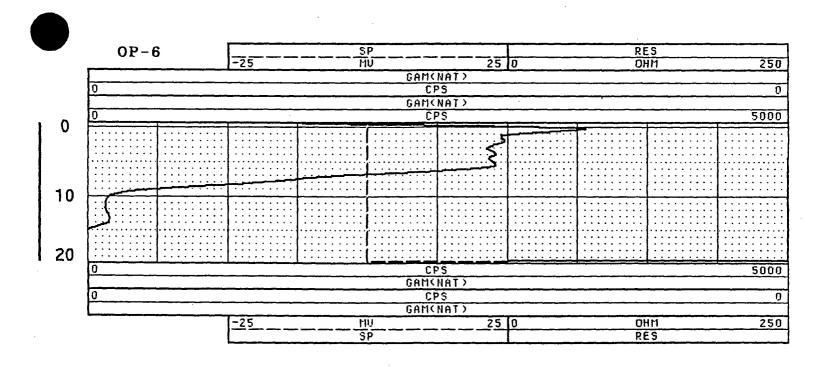
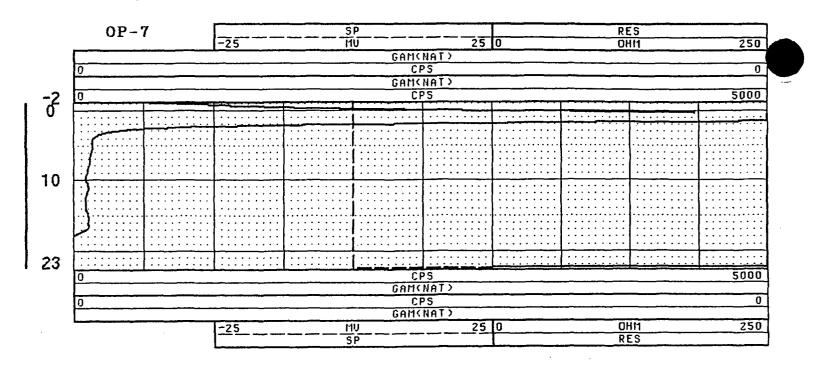


FIGURE A.2-19. GEOPHYSICAL LOGS FOR OP-5 & OP-6.



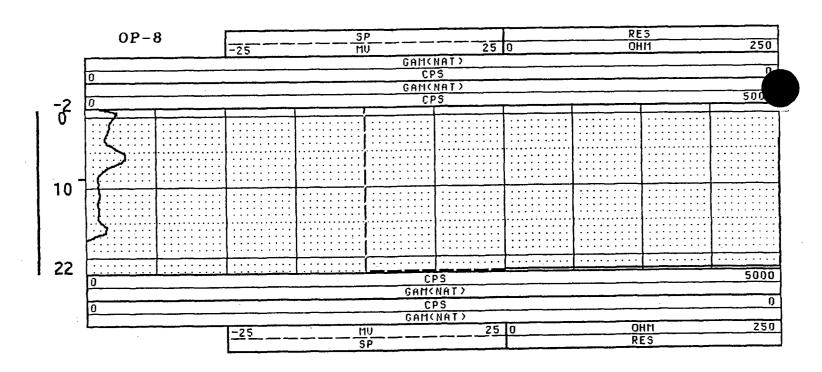
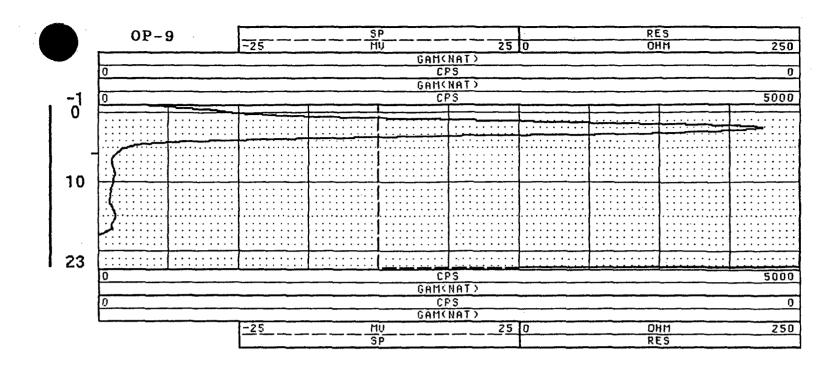


FIGURE A.2-20. GEOPHYSICAL LOGS FOR OP-7 & OP-8.



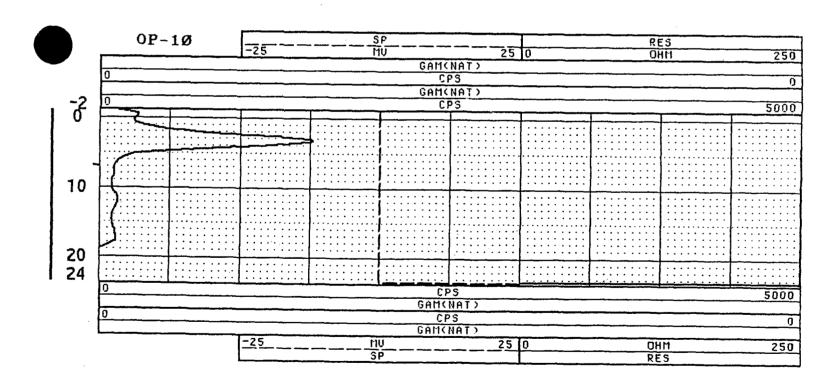
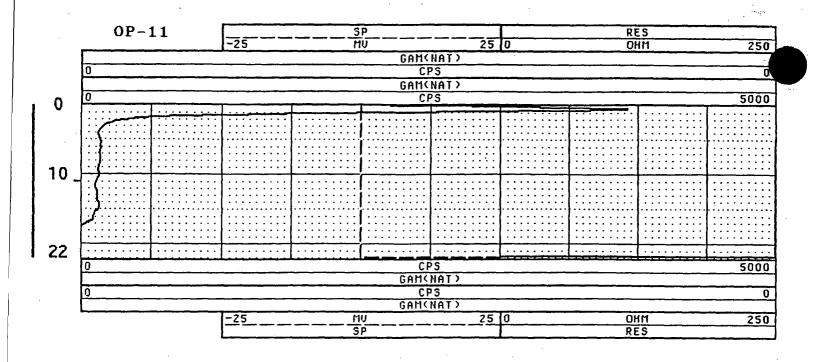


FIGURE A.2-21. GEOPHYSICAL LOGS FOR OP-9 & OP-10.



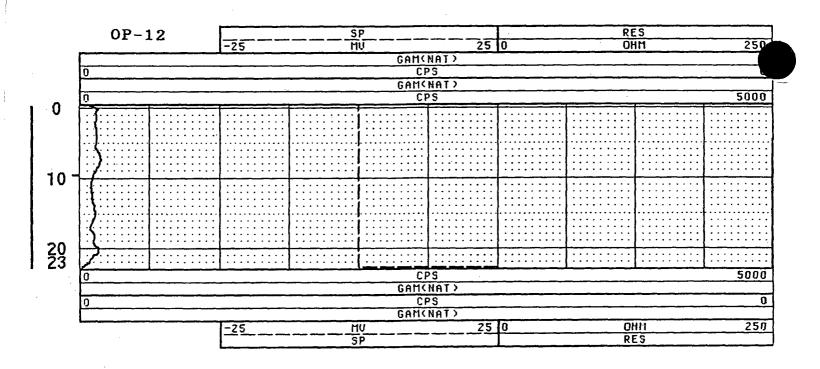
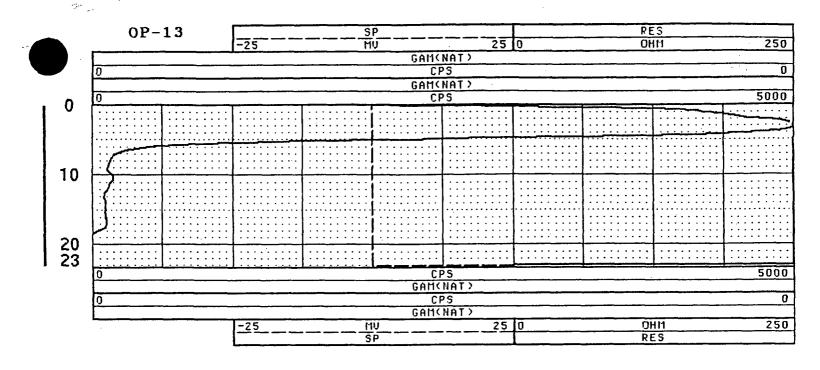


FIGURE A.2-22. GEOPHYSICAL LOGS FOR OP-11 & OP-12.



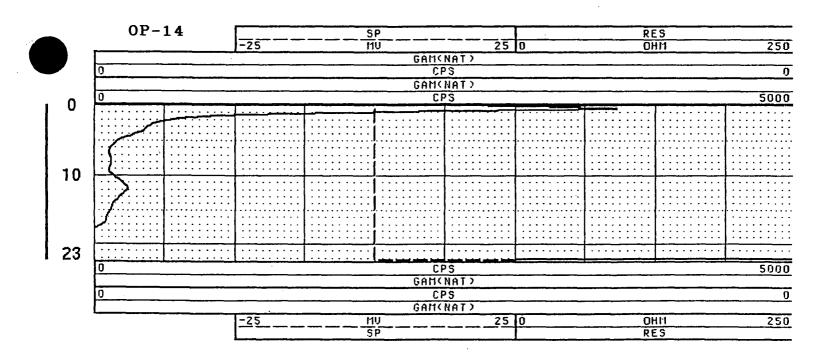
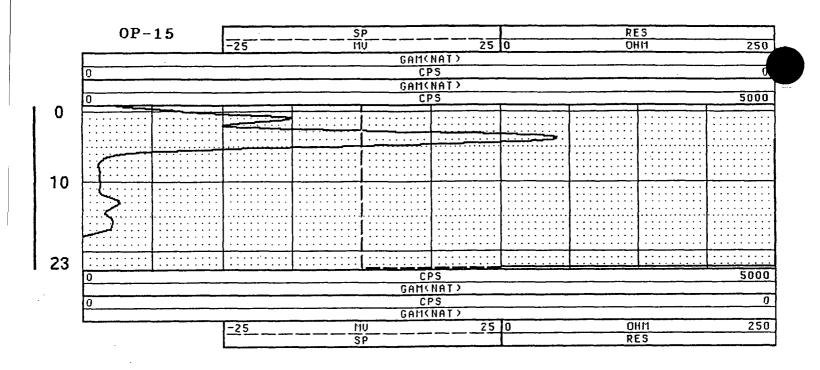
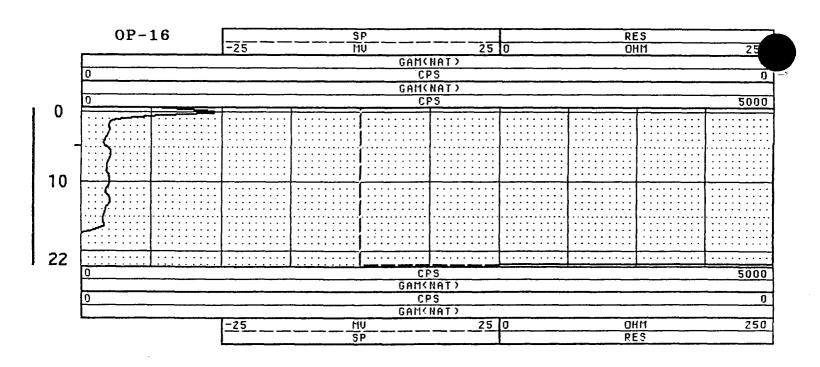


FIGURE A.2-23. GEOPHYSICAL LOGS FOR OP-13 & OP-14.





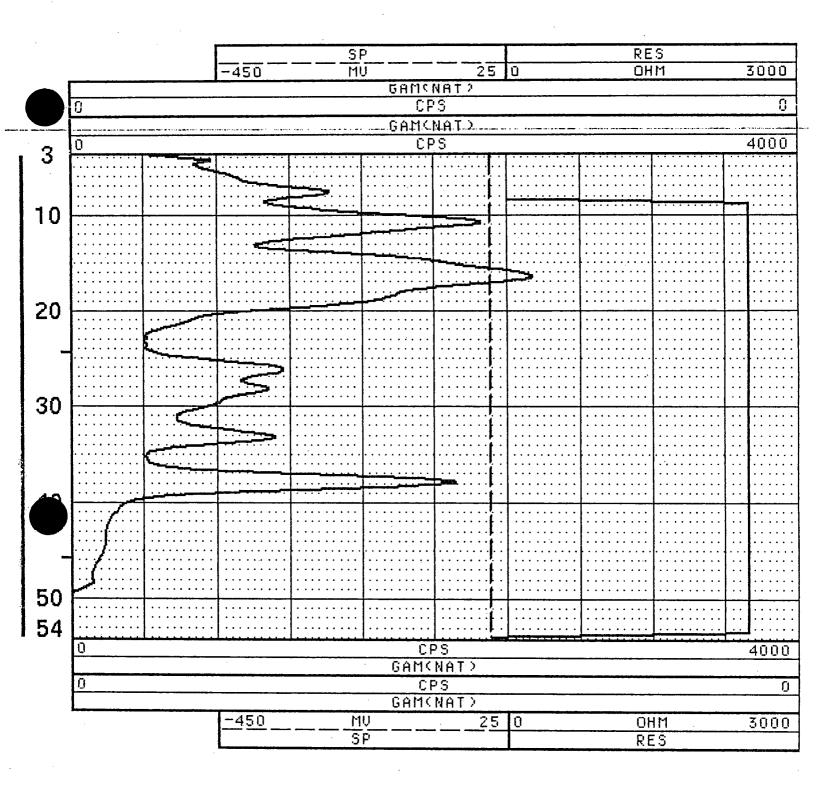


FIGURE A.2-25. GEOPHYSICAL LOG FOR LGW-1.

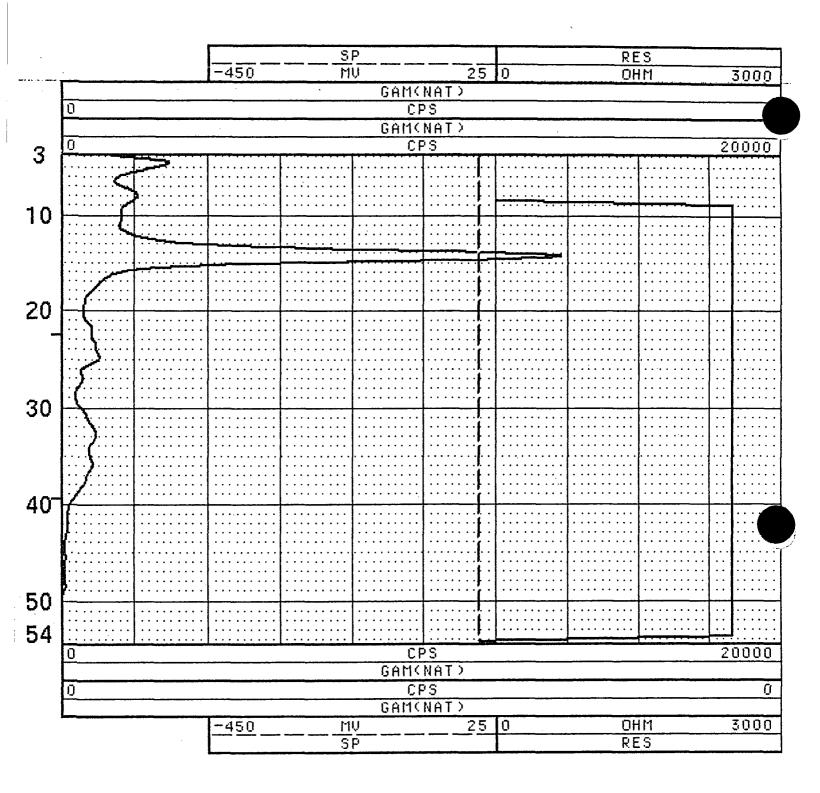


FIGURE A.2-26. GEOPHYSICAL LOG FOR LGW-2.

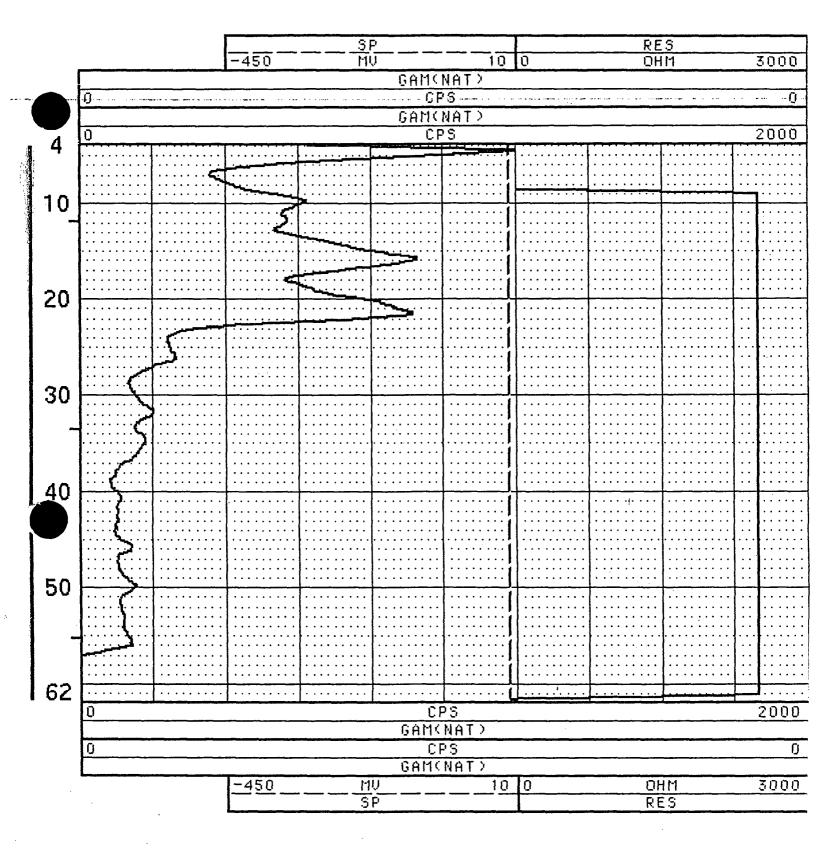
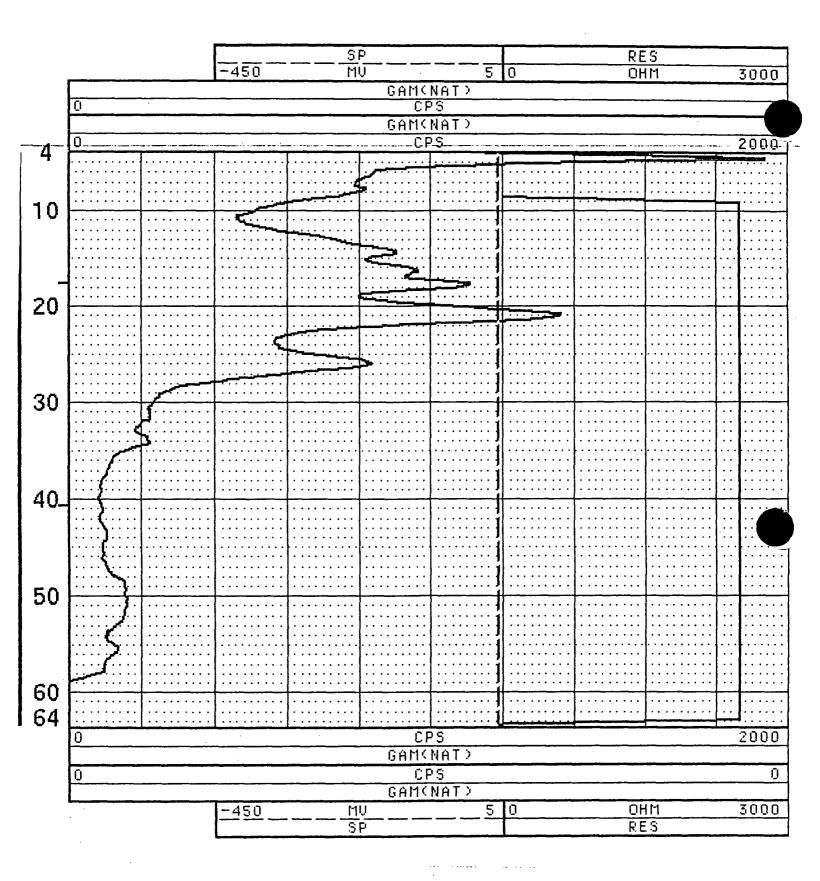


FIGURE A.2-27. GEOPHYSICAL LOG FOR LGW-3.



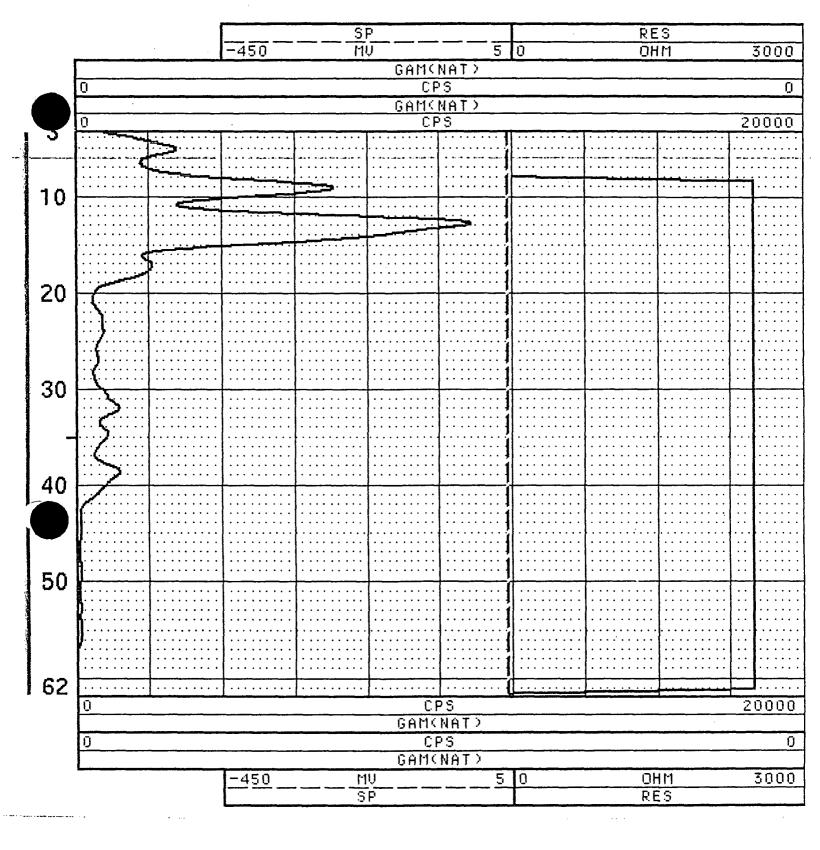


FIGURE A.2-29. GEOPHYSICAL LOG FOR LGW-5.

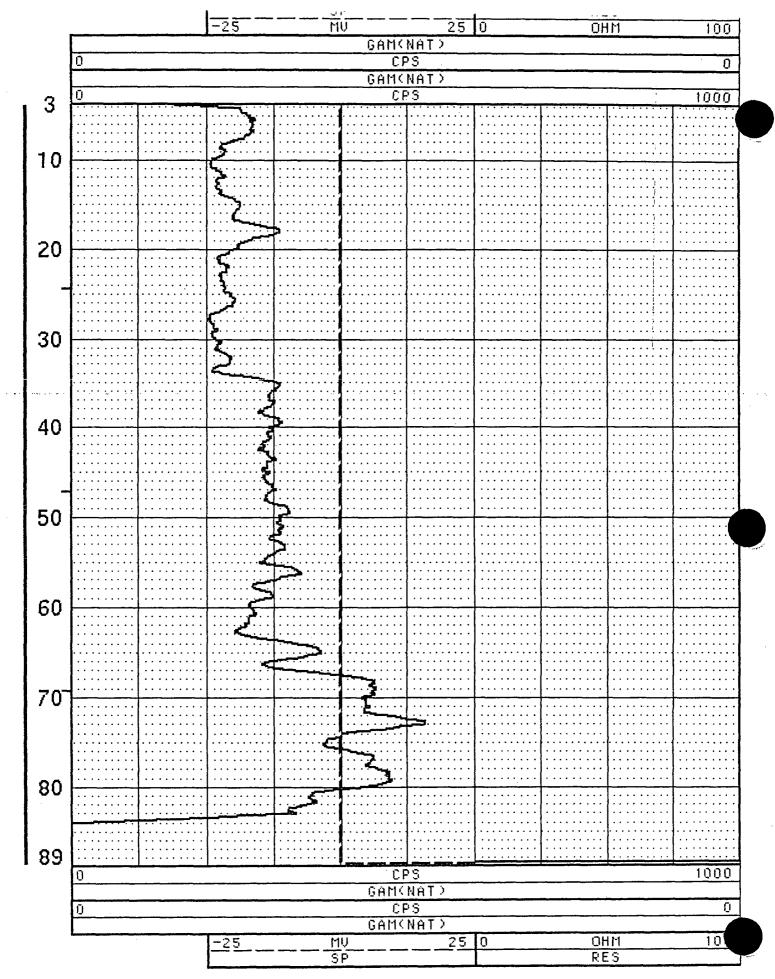


FIGURE A.2-30. GEOPHYSICAL LOG FOR STH-1A.

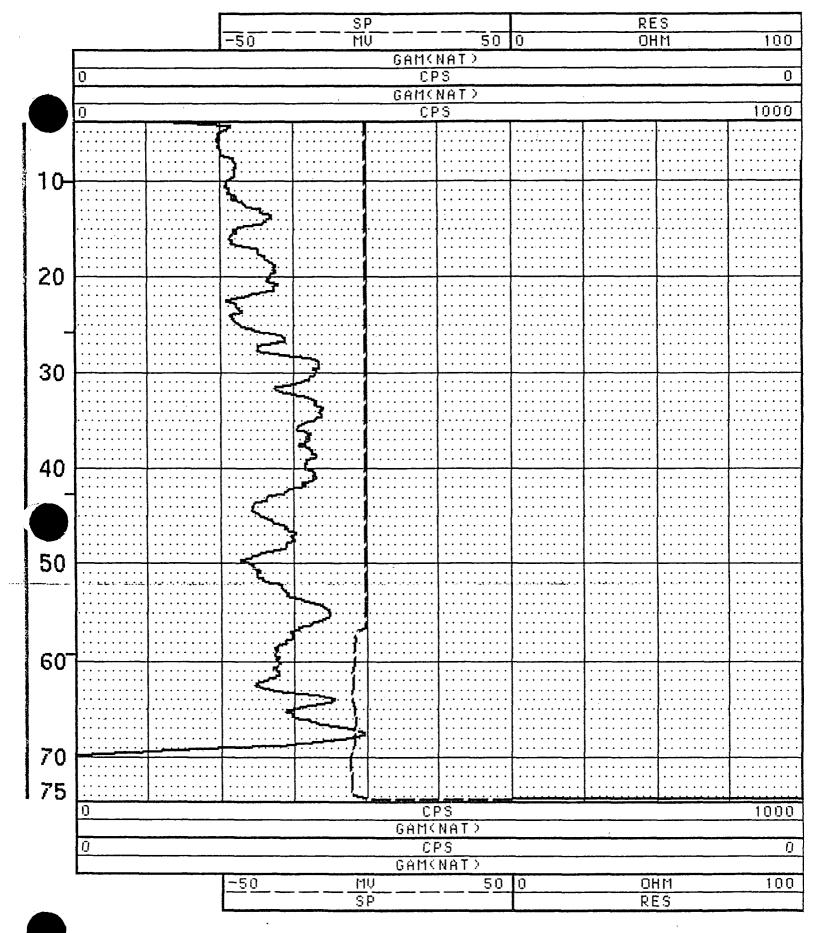


FIGURE A.2-31. GEOPHYSICAL LOG FOR STH-3.

A.3 TEST HOLE PREDICTED RADIUM-226 CONCENTRATIONS

This section presents the predicted radium concentrations from the gamma downhole logs for the test holes. These profiles were prepared by converting gamma log values to Ra-226 values using the relationships described in Section 2.4. A predicted radium concentration for each of the geophysical logs was developed.

These figures present the clean-up standard of 8 pCi/gm as a vertical dashed line on the graphs. Therefore, it is easy to see the levels that exceed the estimated concentrations greater than 8 pCi/gm. As an example, Figure A.3-22 shows that the radium concentration of the upper portion of OP-6 is greater than 8 pCi/gm, while the level below 9' is less than this concentration.

Section A.1 of Appendix A presents the lithologic logs for the test holes. Section A.2 presents the geophysical logs for the test holes that were used to develop the estimated radium-226 concentrations.

The estimated radium concentrations in the tailings vary considerably, with higher values in the slimes than in the sandy portion of the tailings. The radium concentration is generally lower in the upper portion of the tailings and higher toward the base of the tailings.

The estimated radium concentration in the bottom of test hole TW4-2C is large and does not define the base of the elevated radium levels. A paired deeper test hole exists at most of the tailings sites and should be used to define the lower depth of elevated concentration. Figure A.3-4 shows that the elevated

concentrations extend to a depth of 42 feet at well TW4-2B.

Appendix D presents the laboratory radium values that were measured from the test hole materials.

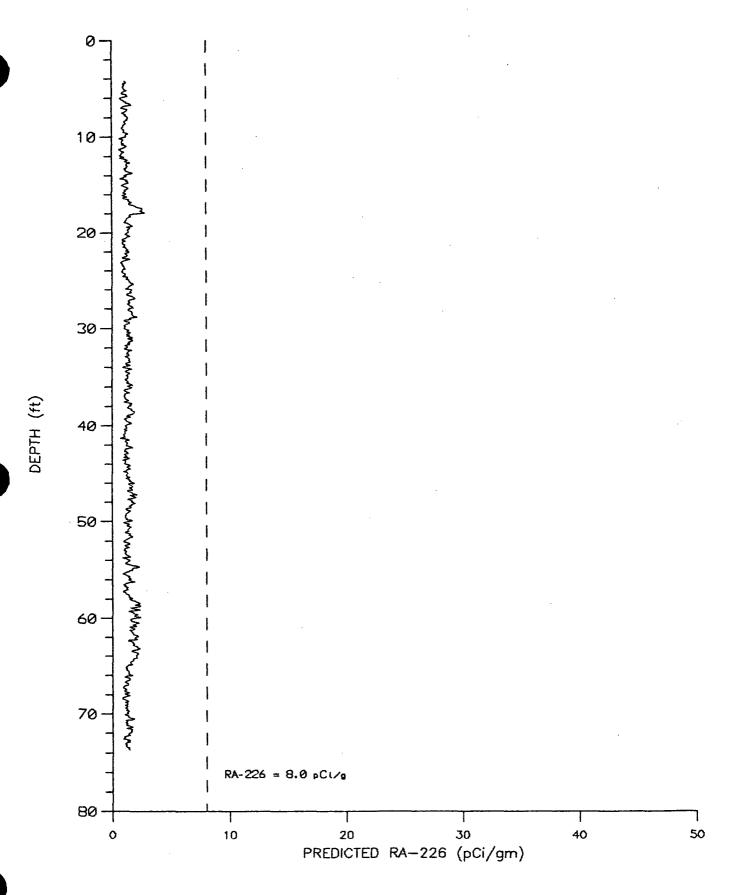


FIGURE A.3-1. PREDICTED RA-226 (pCt/g) IN TEST HOLE TW3-1.

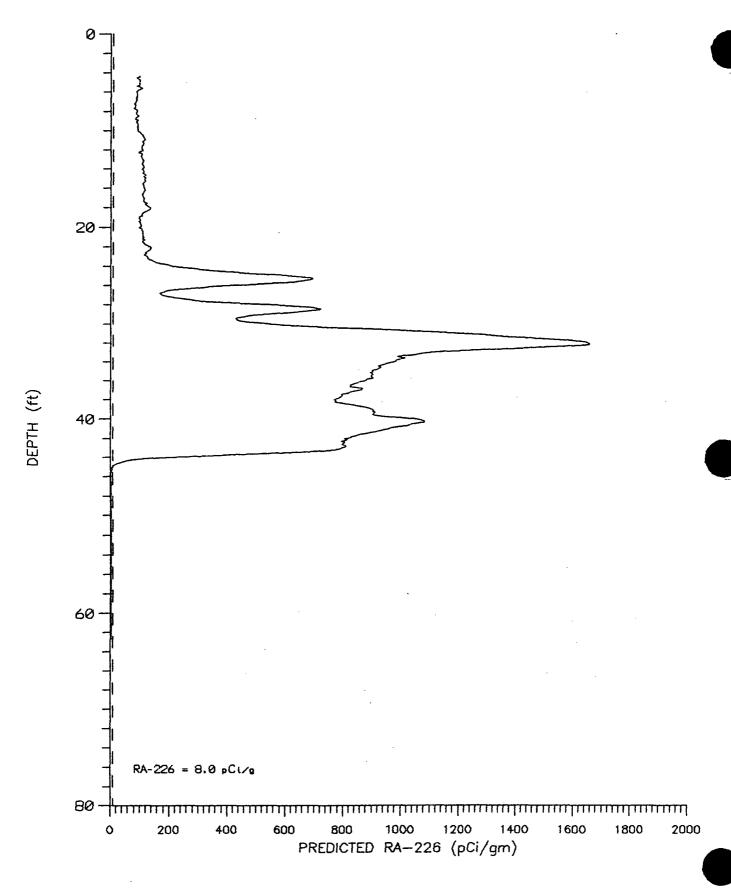


FIGURE A.3-2. PREDICTED RA-226 (pCt/g) IN TEST HOLE TW4-18.

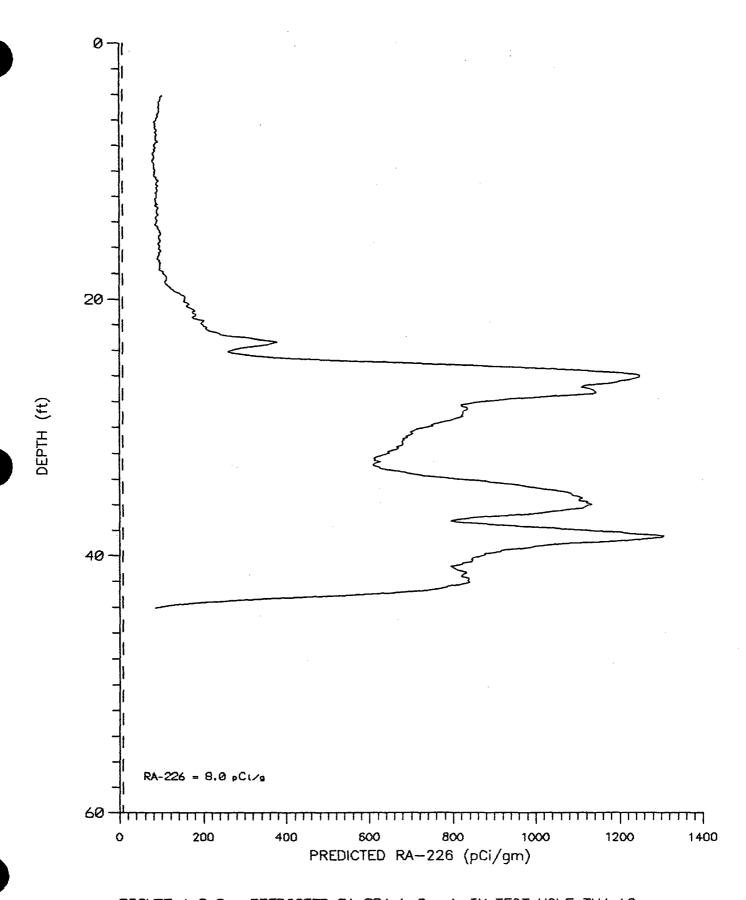


FIGURE A.3-3. PREDICTED RA-226 (pCt/g) IN TEST HOLE TW4-1C.

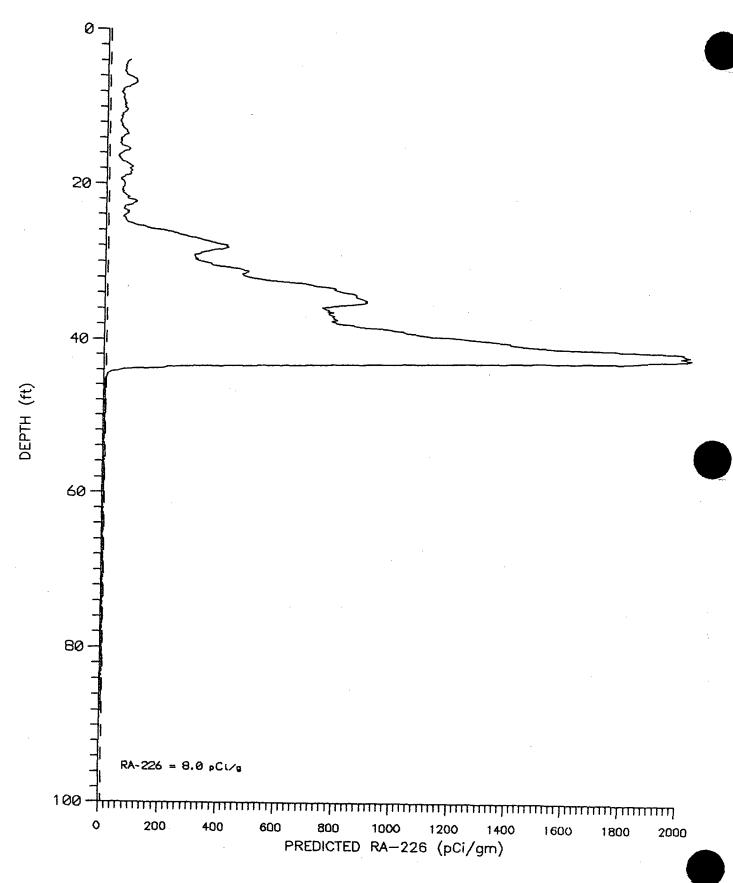


FIGURE A.3-4. PREDICTED RA-226 (pCt/g) IN TEST HOLE TW4-2B.

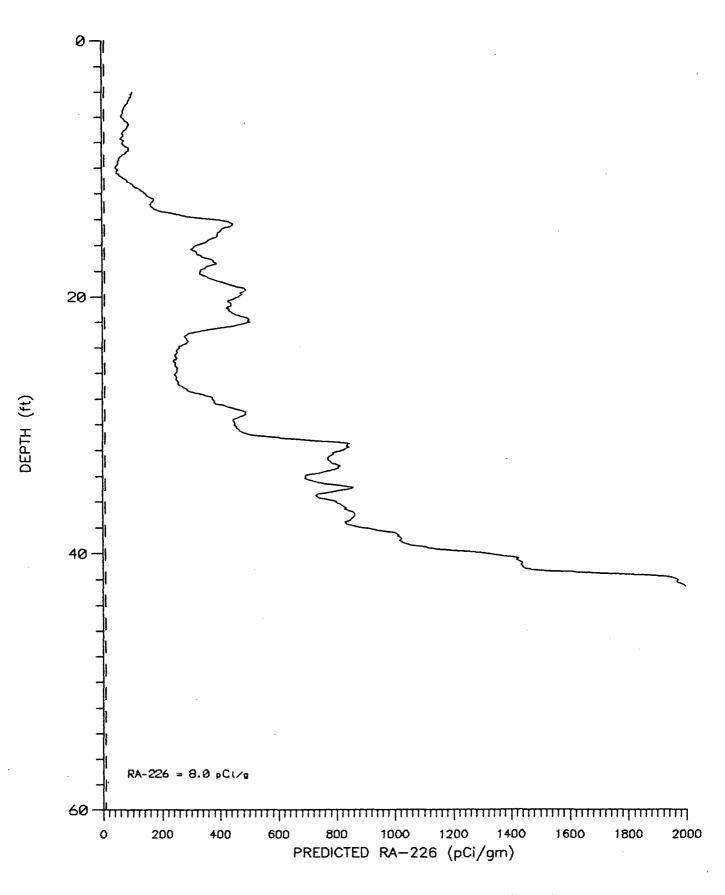


FIGURE A.3-5. PREDICTED RA-226 (pCt/g) IN TEST HOLE TW4-2C.

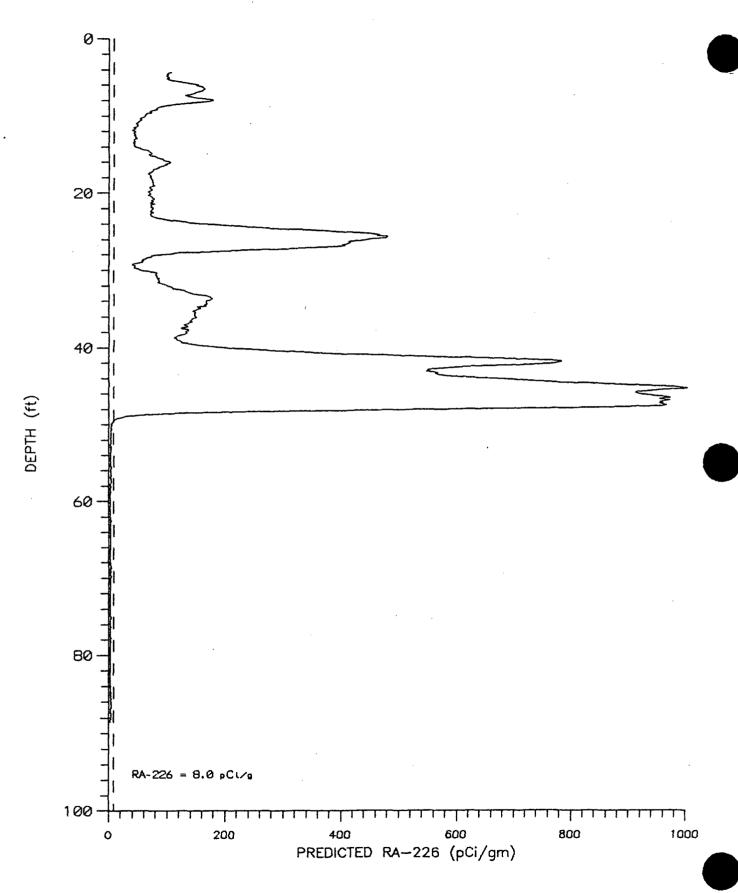


FIGURE A.3-6. PREDICTED RA-226 (pCt/g) IN TEST HOLE TW4-3B.

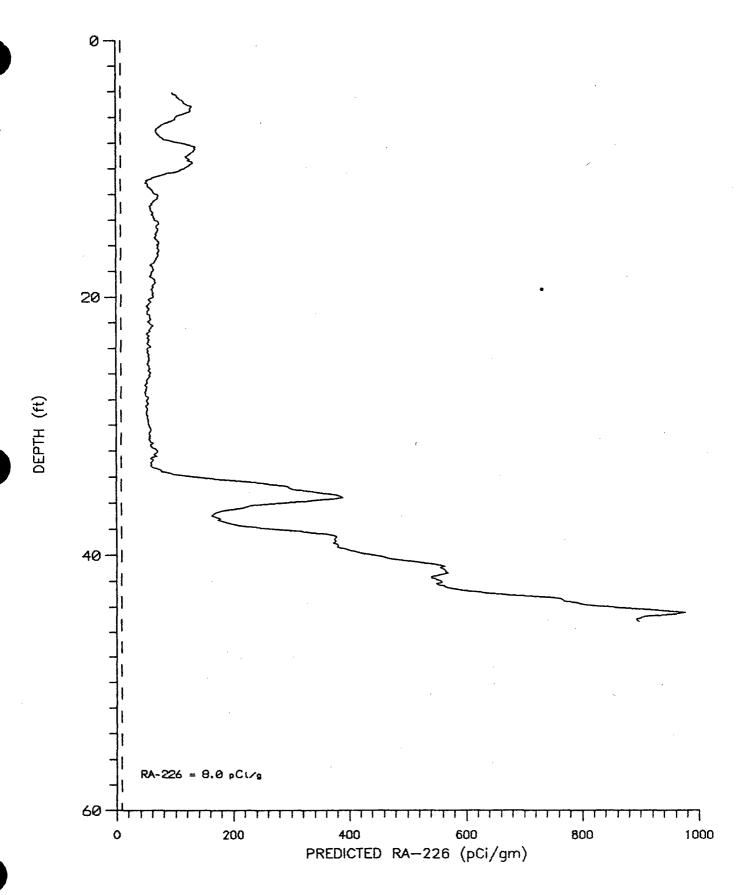


FIGURE A.3-7. PREDICTED RA-226 (pCt/g) IN TEST HOLE TW4-3C.

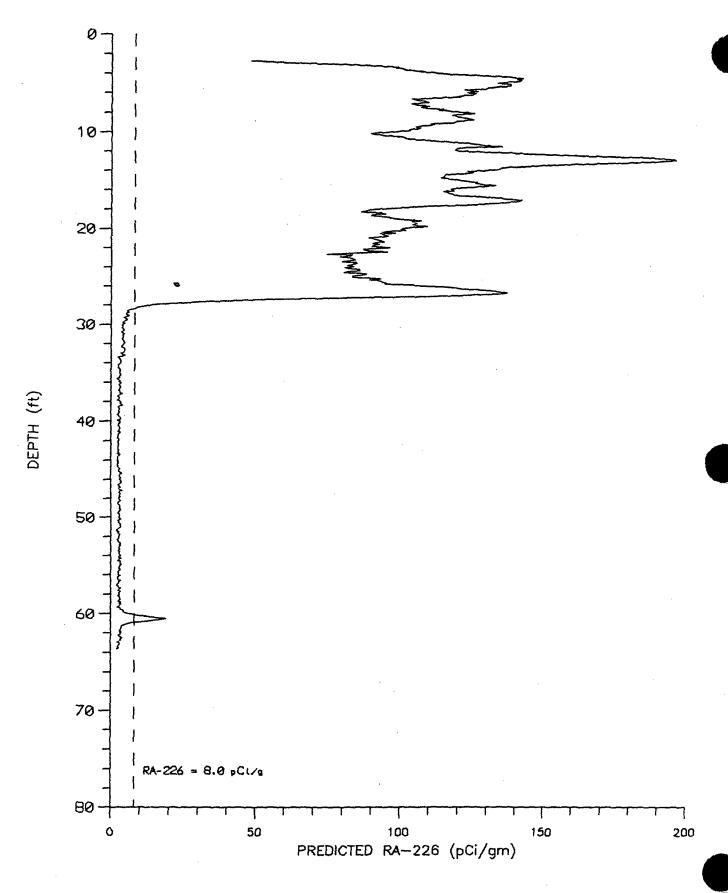


FIGURE A.3-8. PREDICTED RA-226 (pCt/g) IN TEST HOLE TW4-48.

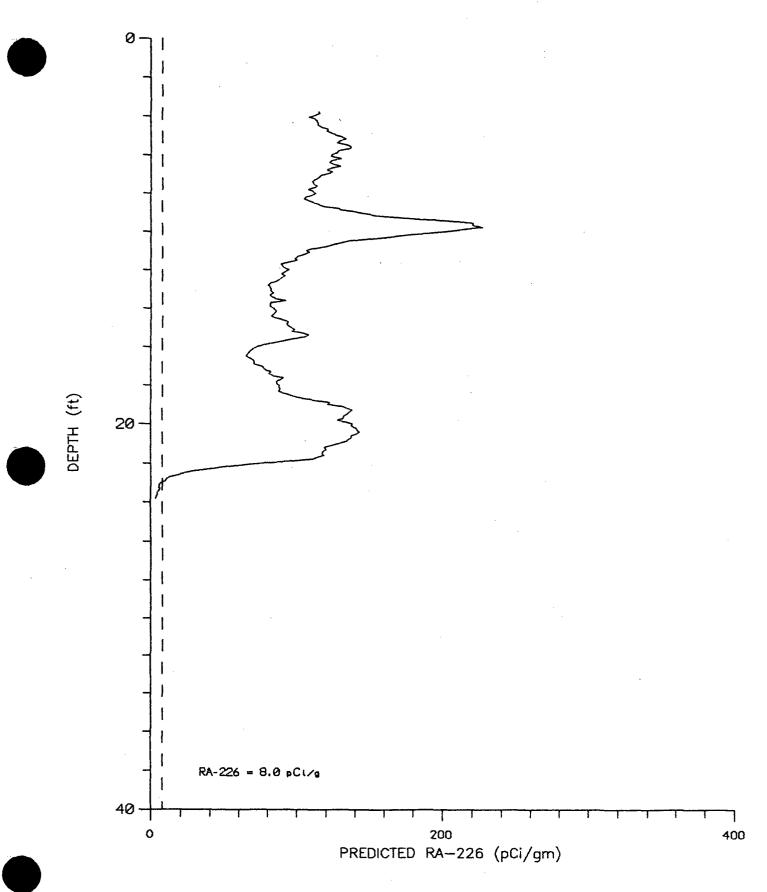


FIGURE A.3-9. PREDICTED RA-226 (pCt/g) IN TEST HOLE TW4-4C.

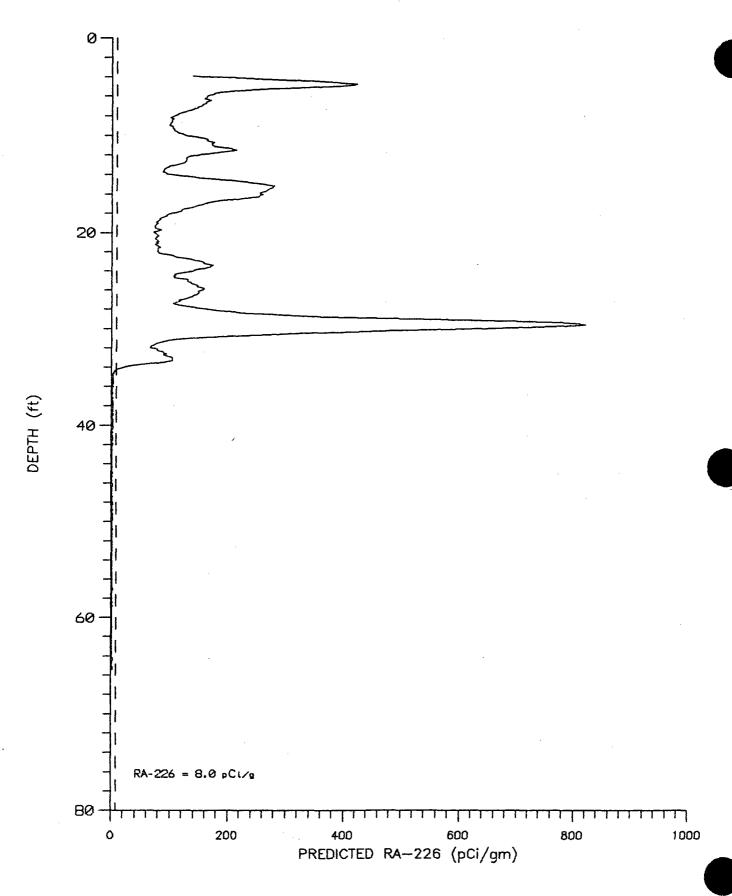


FIGURE A.3-10. PREDICTED RA-226 (pC1/g) IN TEST HOLE TW4-5B.

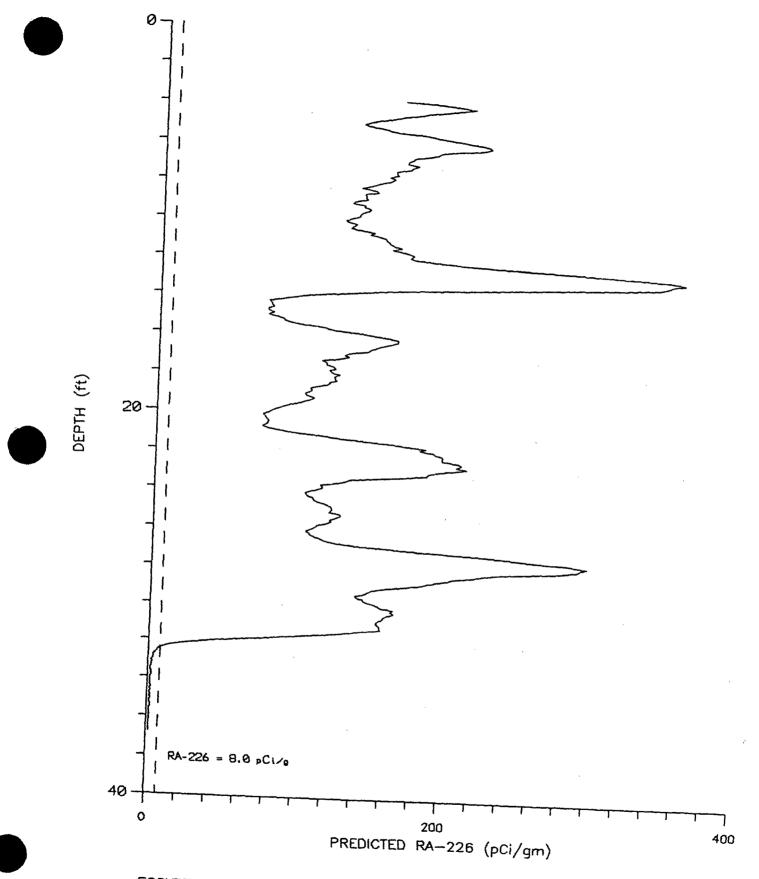


FIGURE A.3-11. PREDICTED RA-226 (pC1/g) IN TEST HOLE TW4-5C.

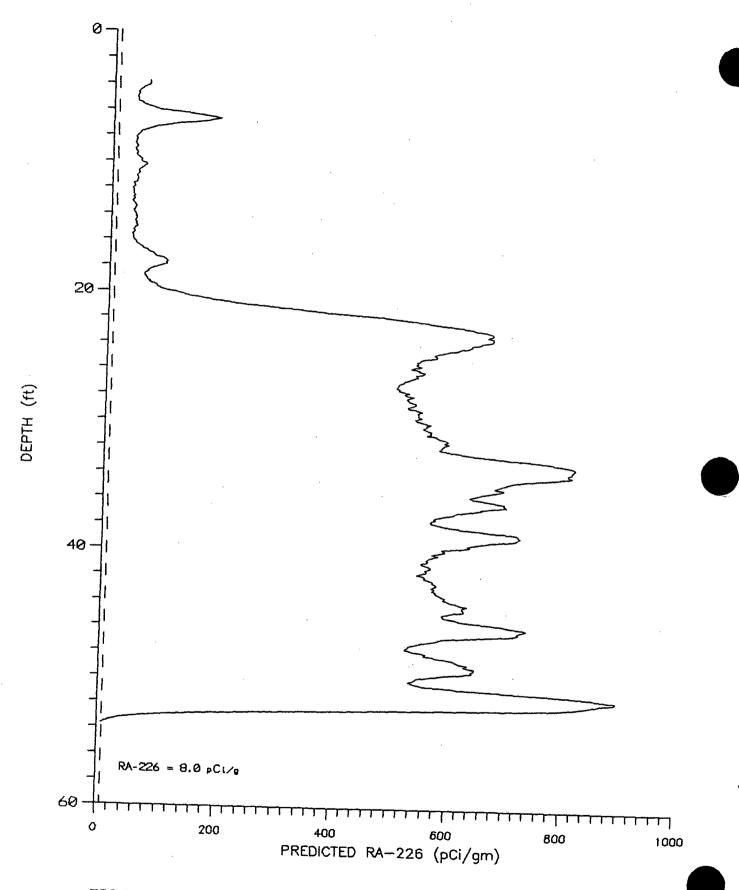


FIGURE A.3-12. PREDICTED RA-226 (pC1/g) IN TEST HOLE TW5-1C.

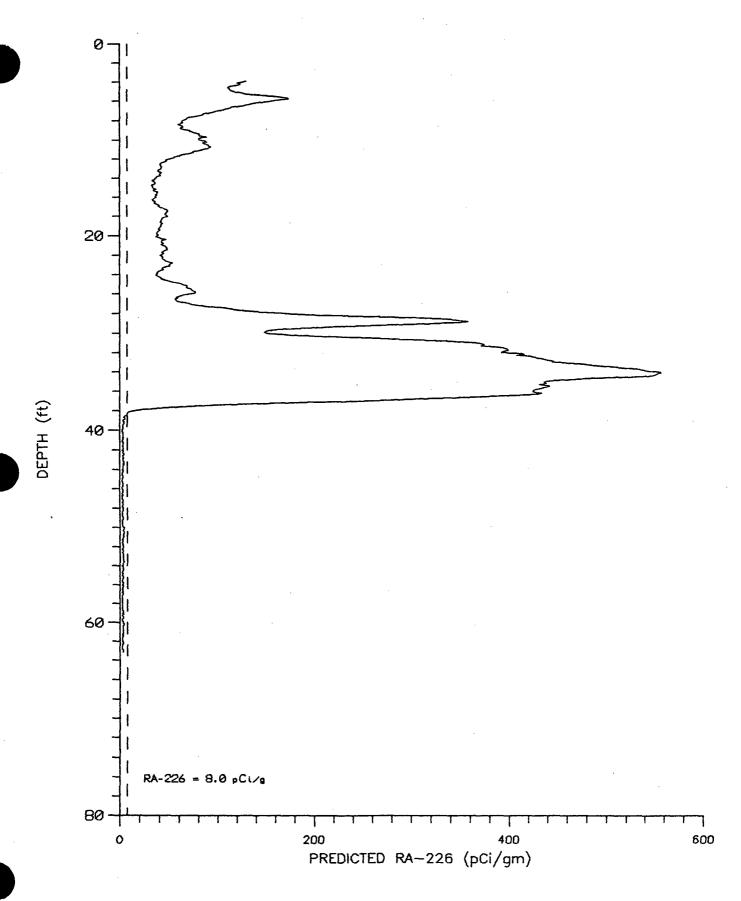


FIGURE A.3-13. PREDICTED RA-226 (pC1/g) IN TEST HOLE TW5-2B.

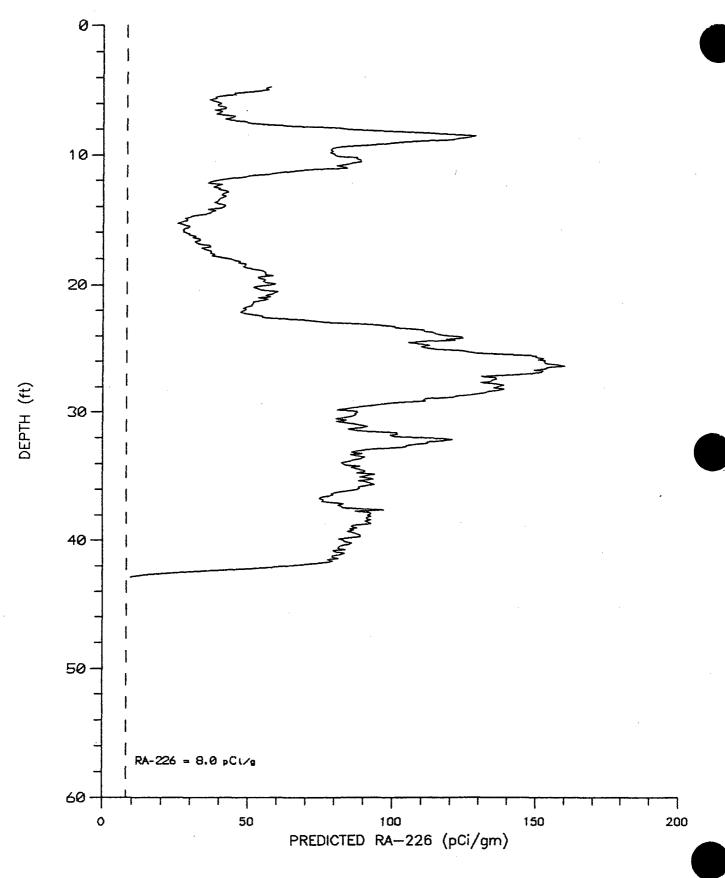


FIGURE A.3-14. PREDICTED RA-226 (pC1/g) IN TEST HOLE TW5-3.

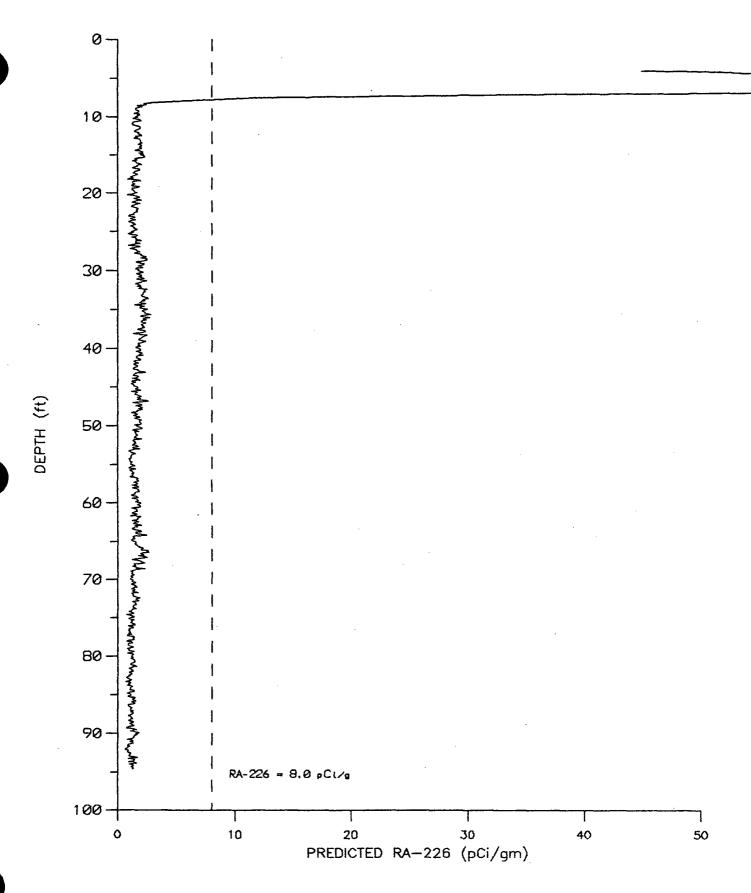


FIGURE A.3-15. PREDICTED RA-226 (pC1/g) IN TEST HOLE NS-1.

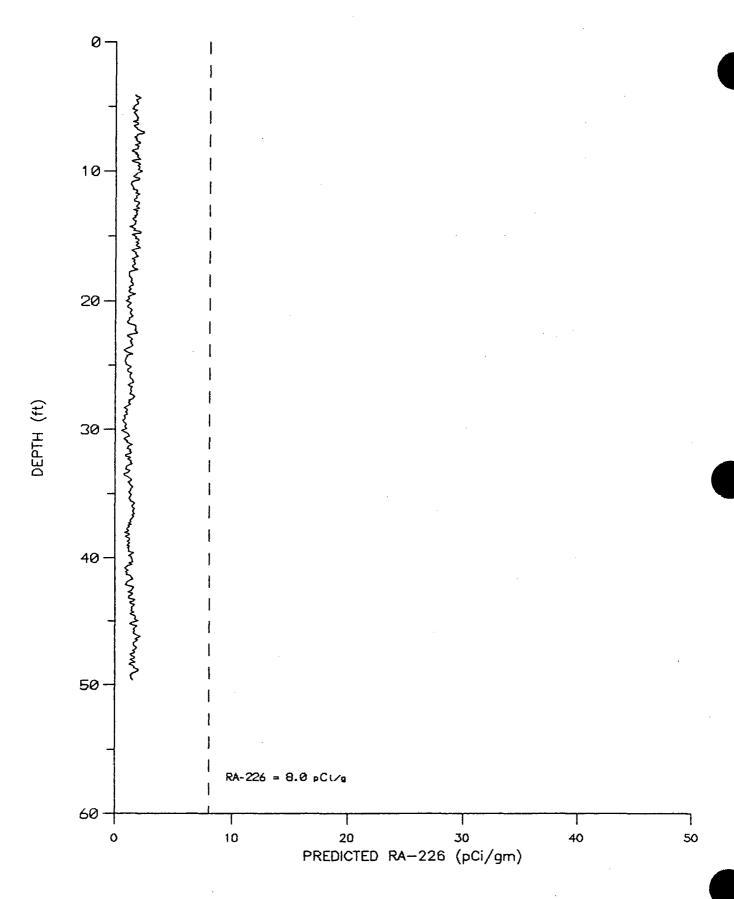


FIGURE A.3-16. PREDICTED RA-226 (pC1/g) IN TEST HOLE NS-2.

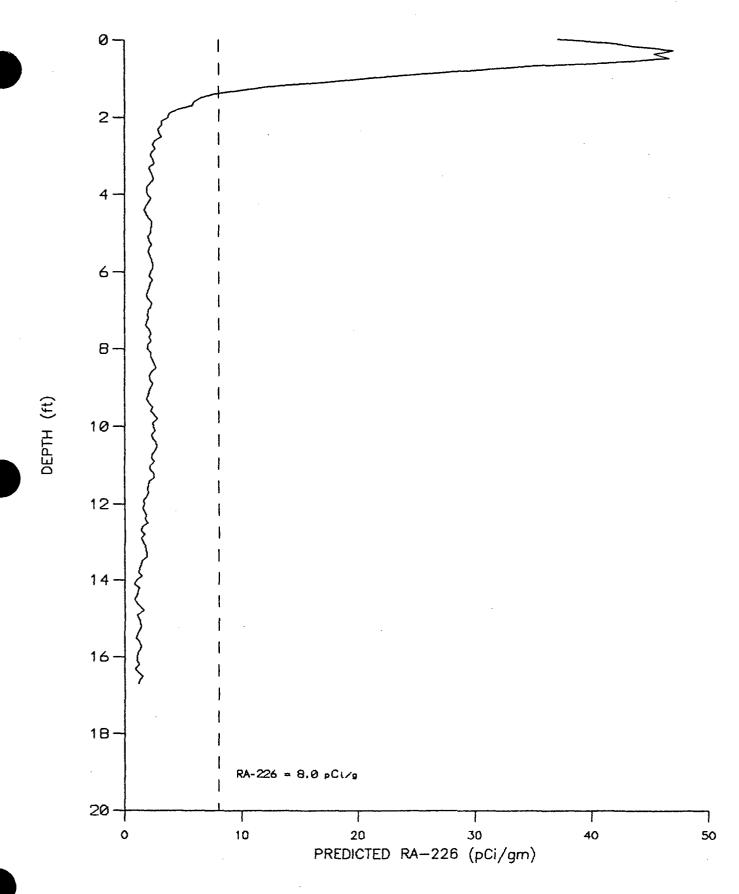


FIGURE A.3-17. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-1.

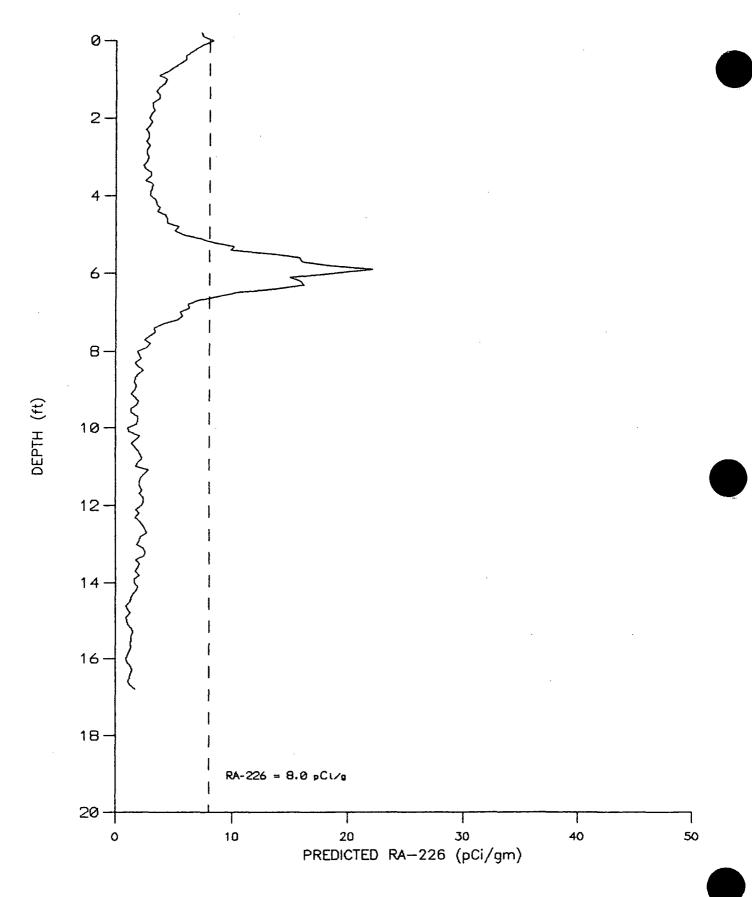


FIGURE A.3-18. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-2.

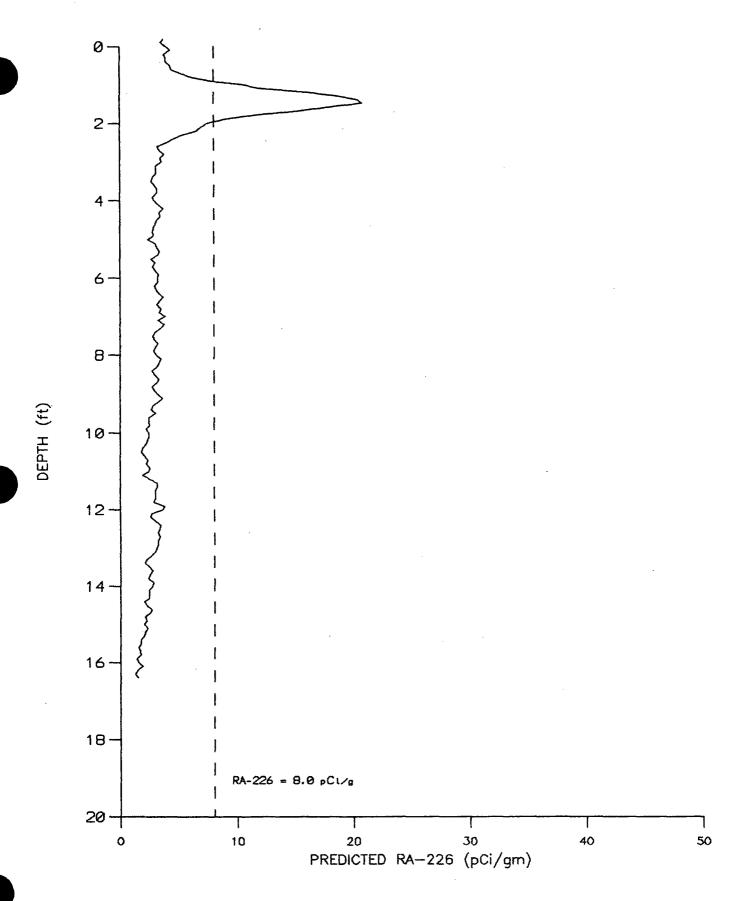


FIGURE A.3-19. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-3.

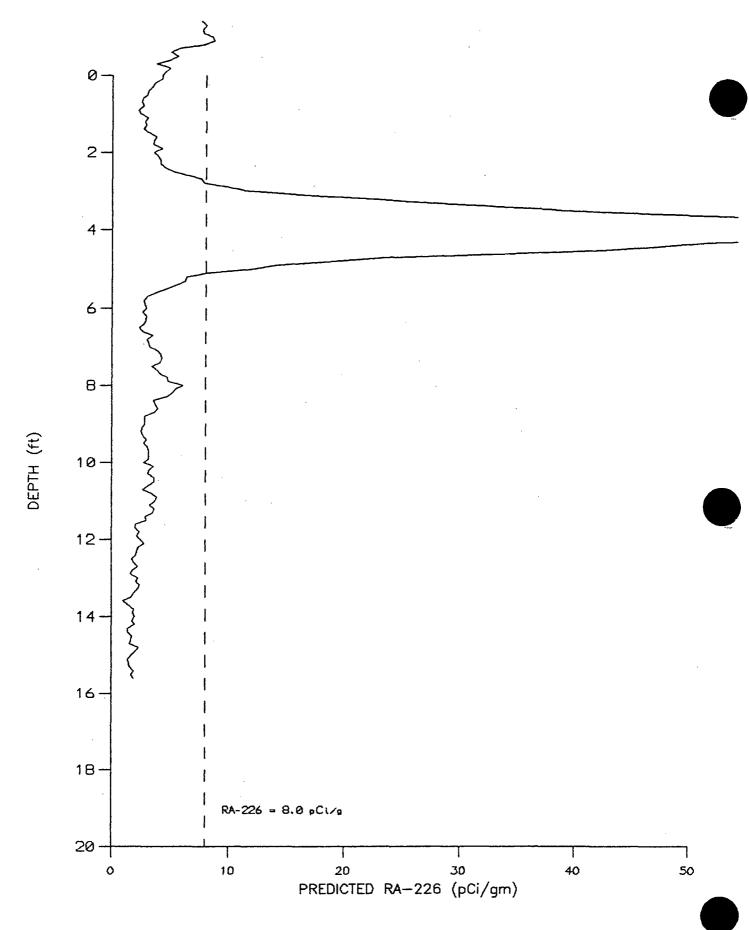


FIGURE A.3-20. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-4.

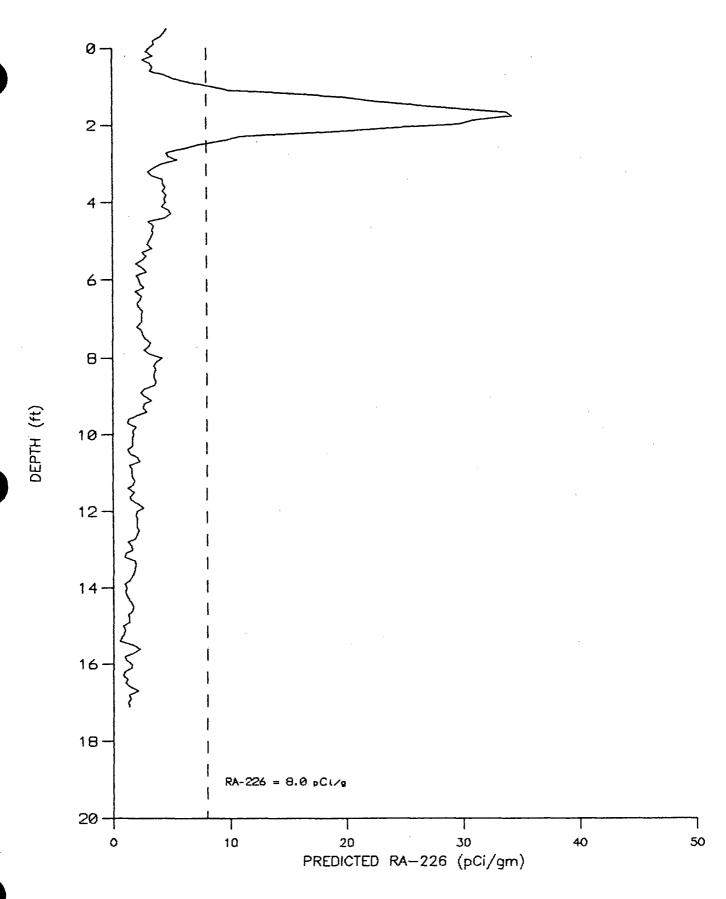


FIGURE A.3-21. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-5.

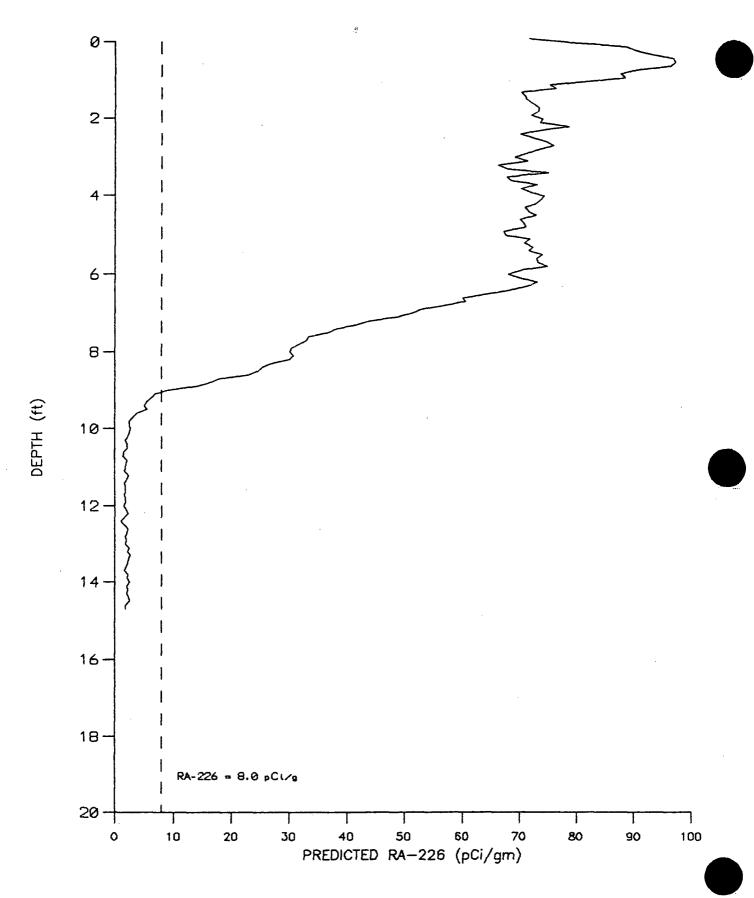


FIGURE A.3-22. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-6.

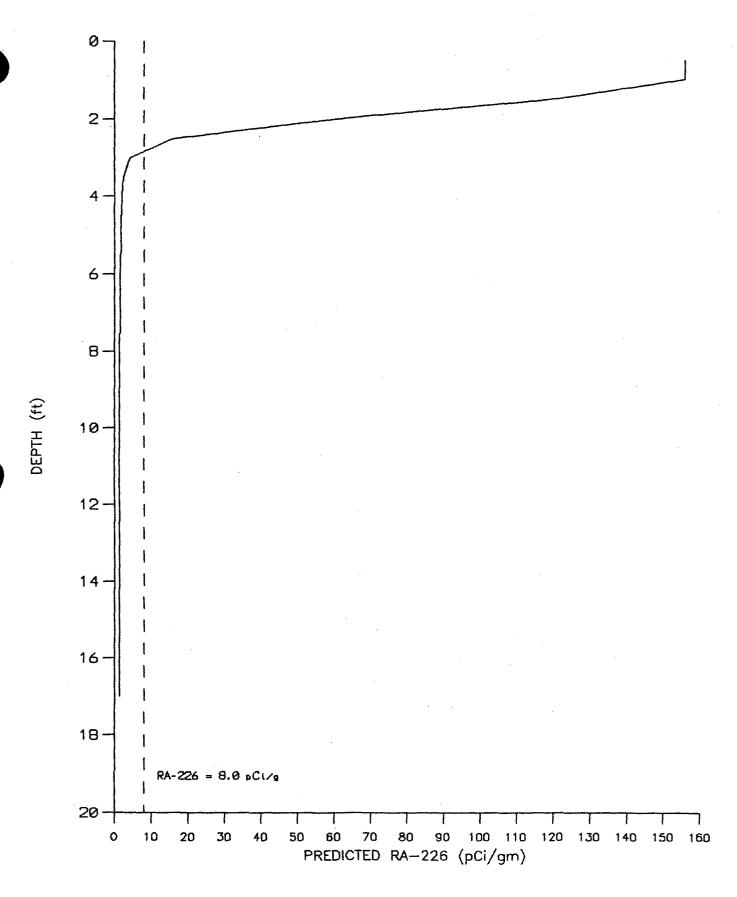


FIGURE A.3-23. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-7.

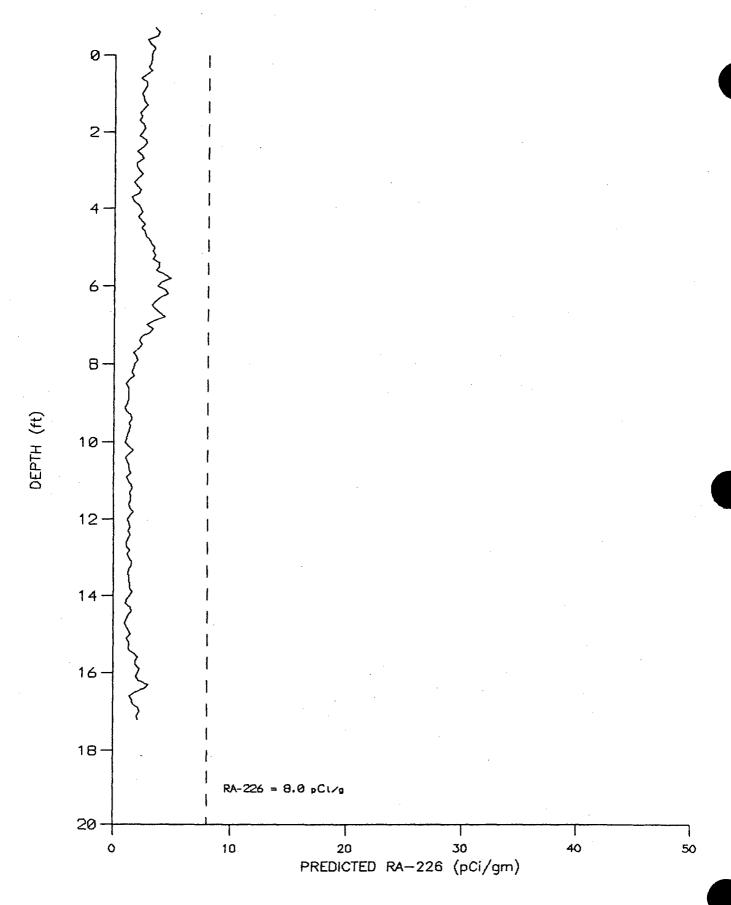


FIGURE A.3-24. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-8.

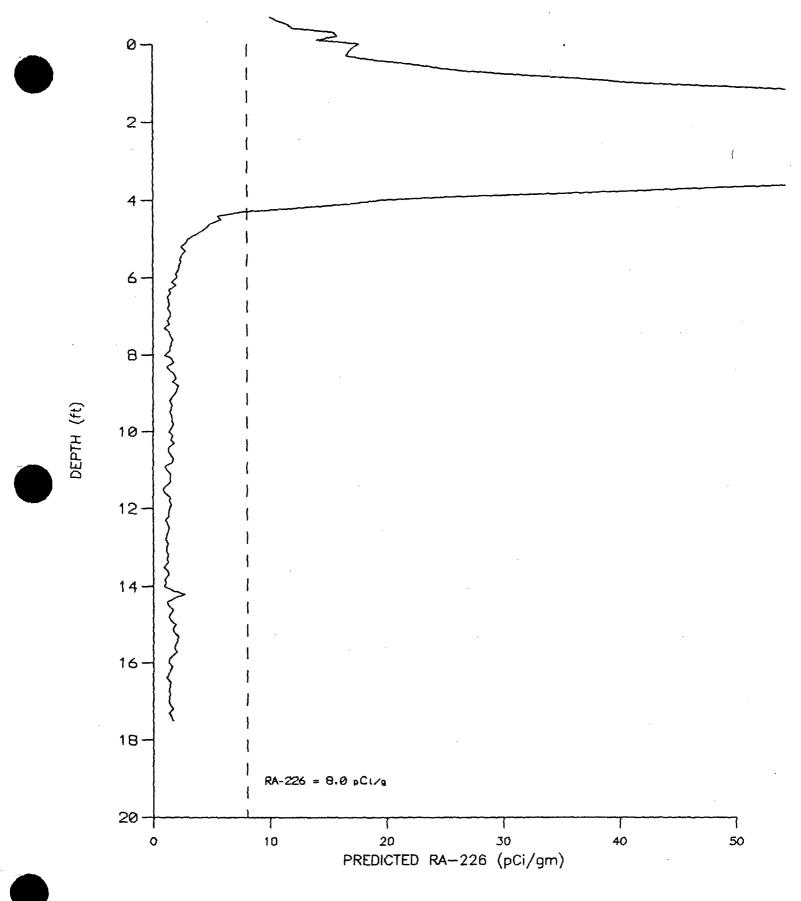


FIGURE A.3-25. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-9.

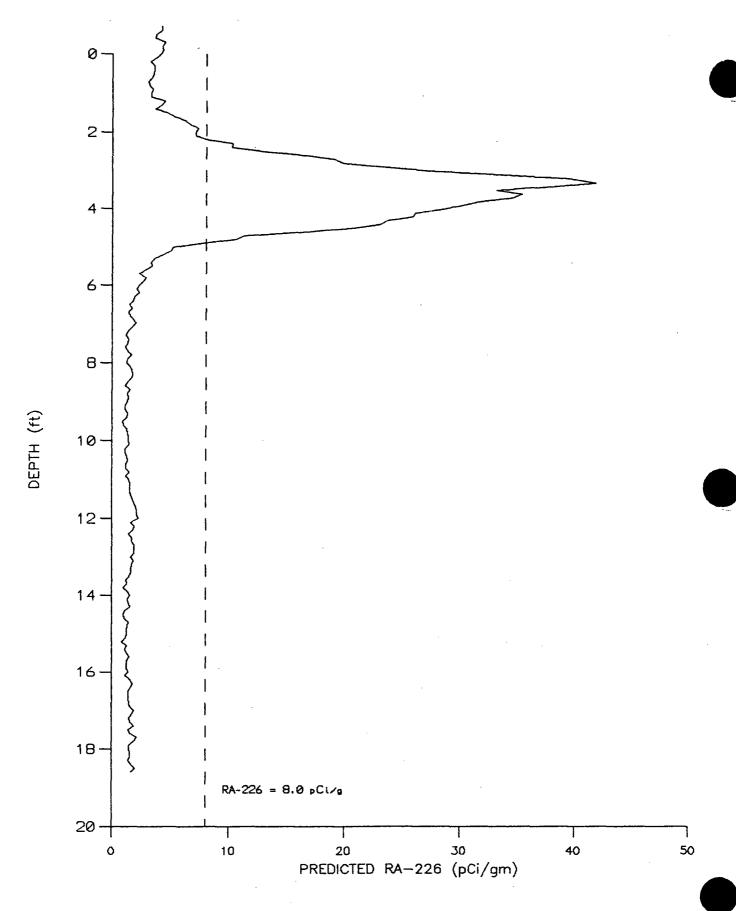


FIGURE A.3-26. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-10.

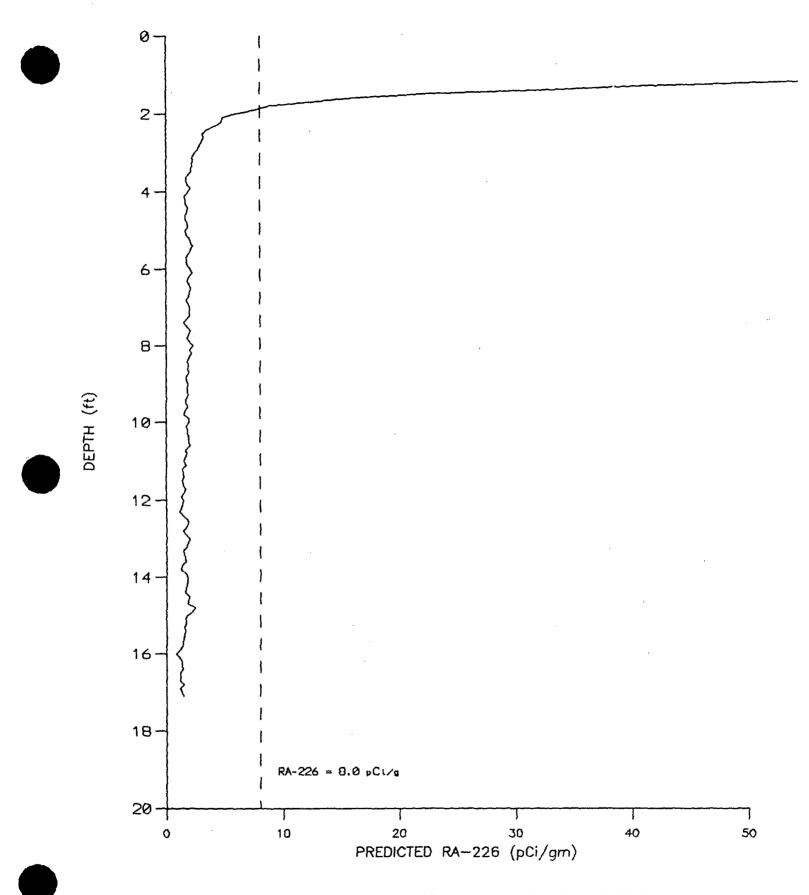


FIGURE A.3-27. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-11.

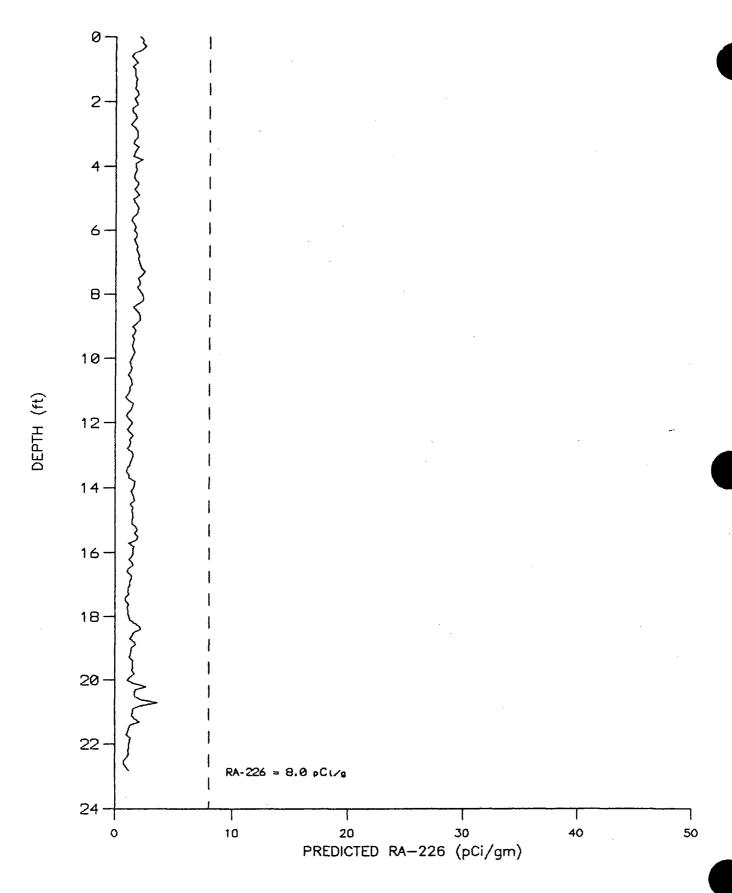


FIGURE A.3-28. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-12.

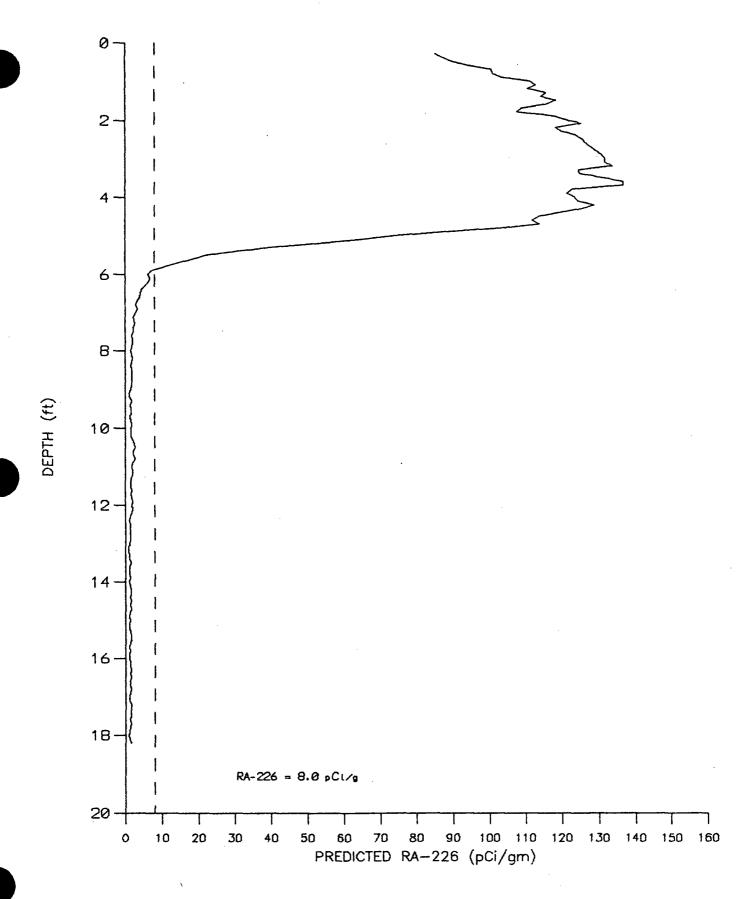


FIGURE A.3-29. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-13.

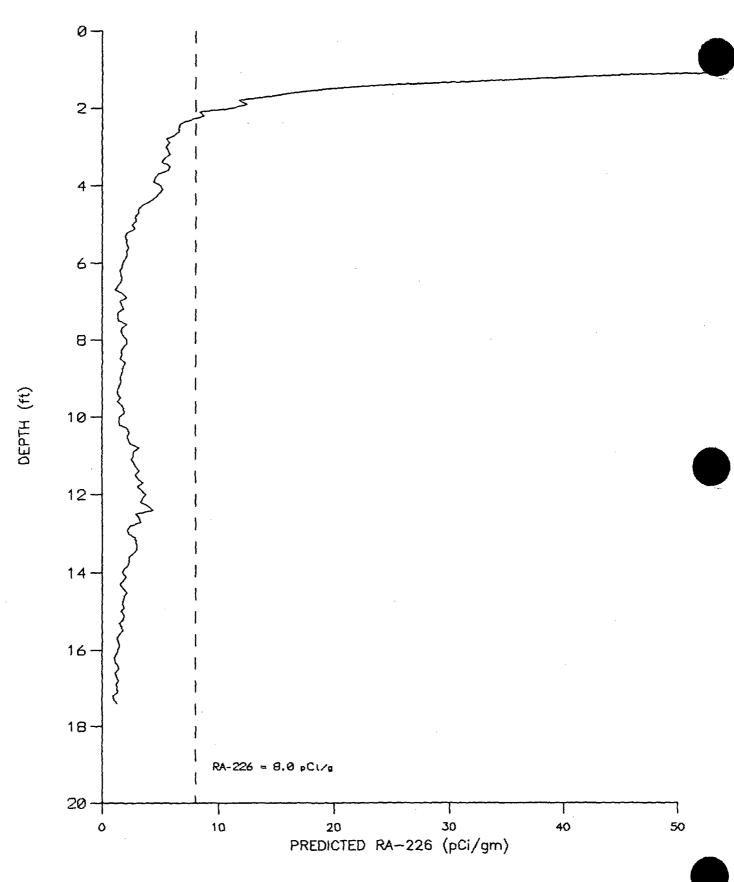


FIGURE A.3-30. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-14.

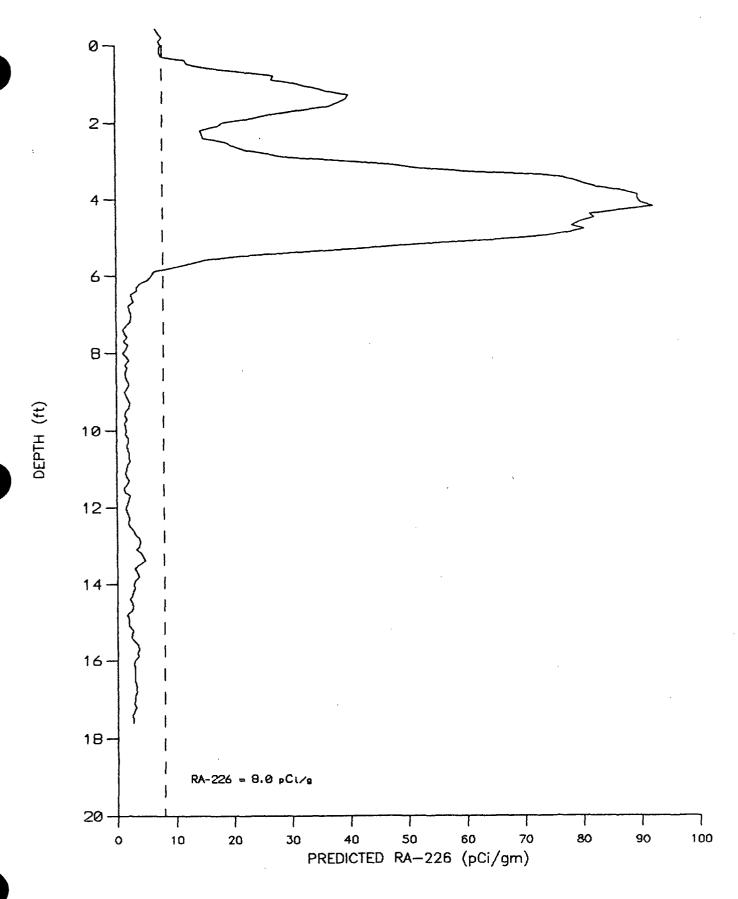


FIGURE A.3-31. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-15.

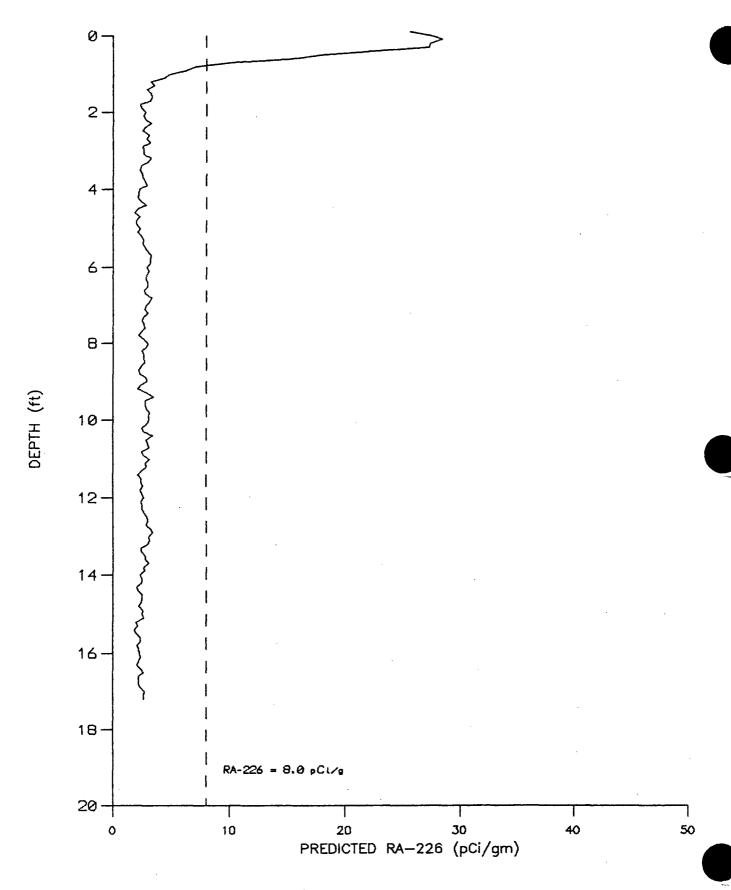


FIGURE A.3-32. PREDICTED RA-226 (pC1/g) IN TEST HOLE OP-16.

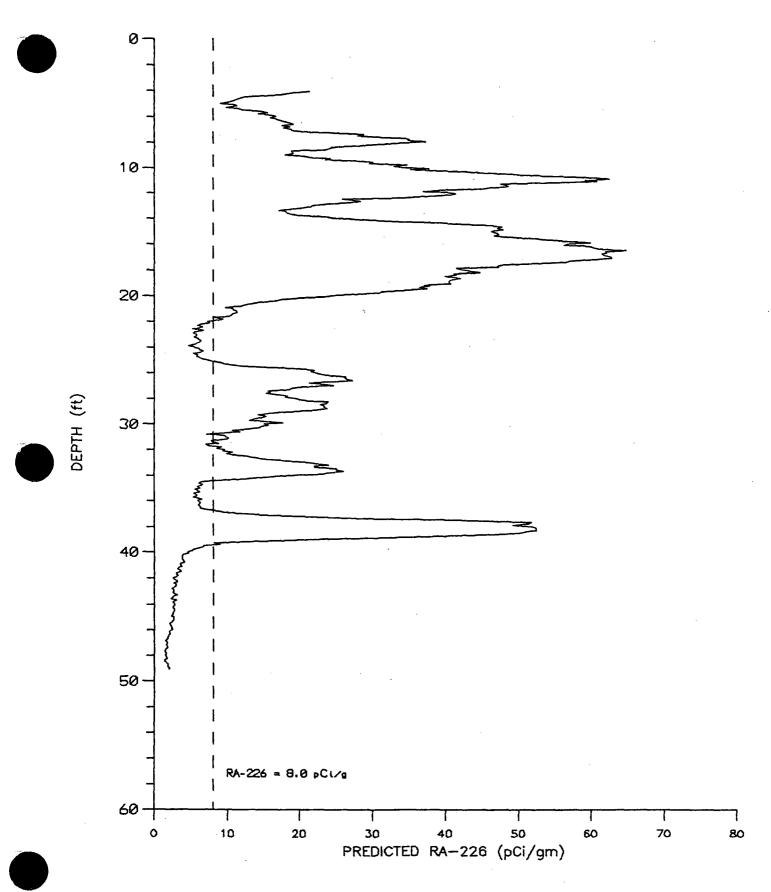


FIGURE A.3-33. PREDICTED RA-226 (pC1/g) IN TEST HOLE LGH-1.

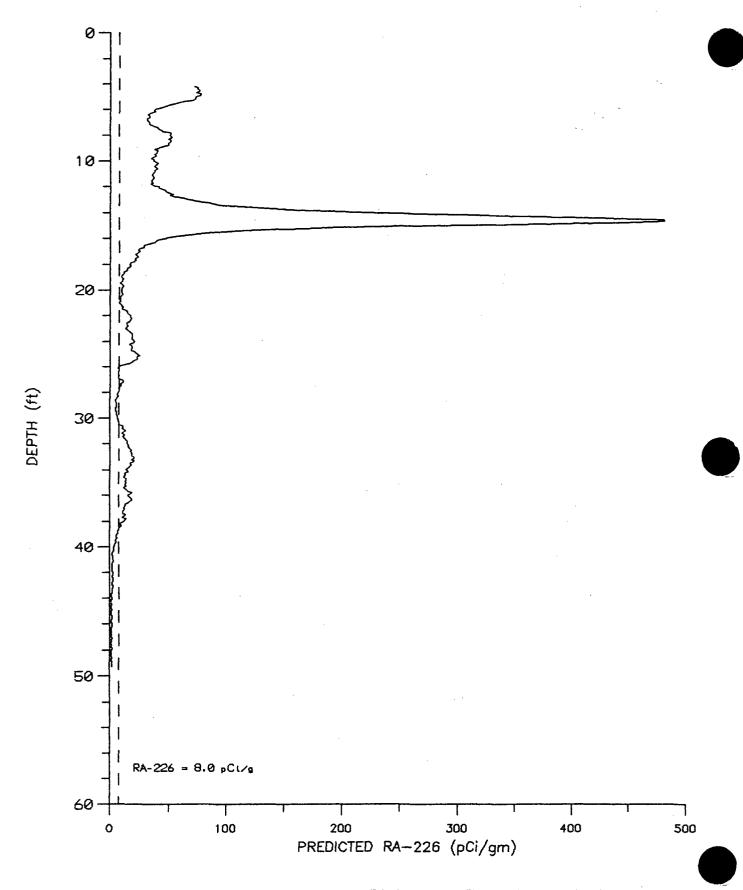


FIGURE A.3-34. PREDICTED RA-226 (PC1/g) IN TEST HOLE LGW-2.

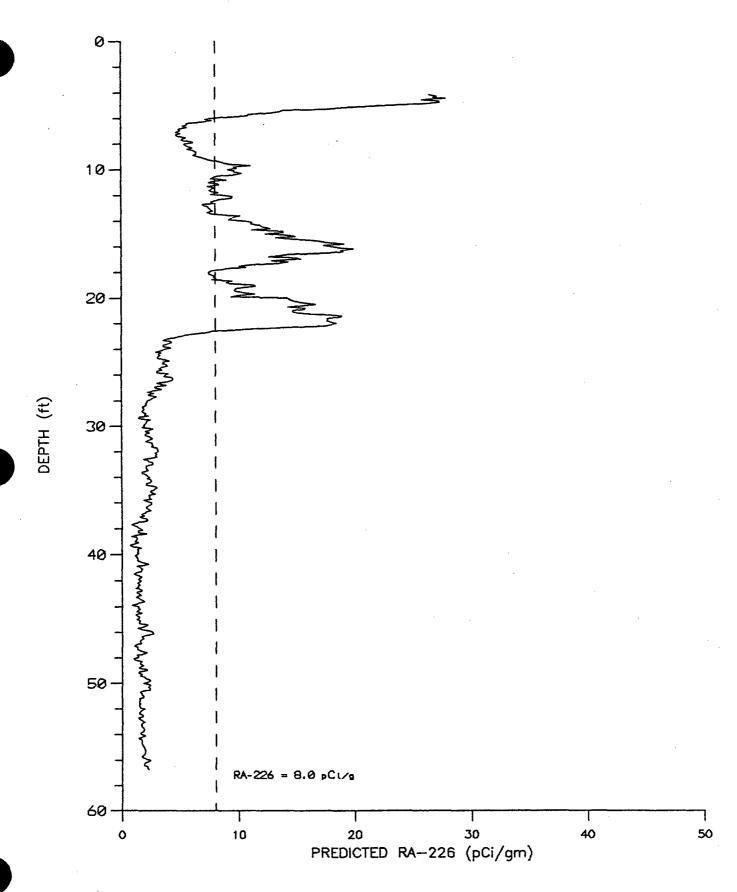


FIGURE A.3-36. PREDICTED RA-226 (pC1/g) IN TEST HOLE LGW-3.

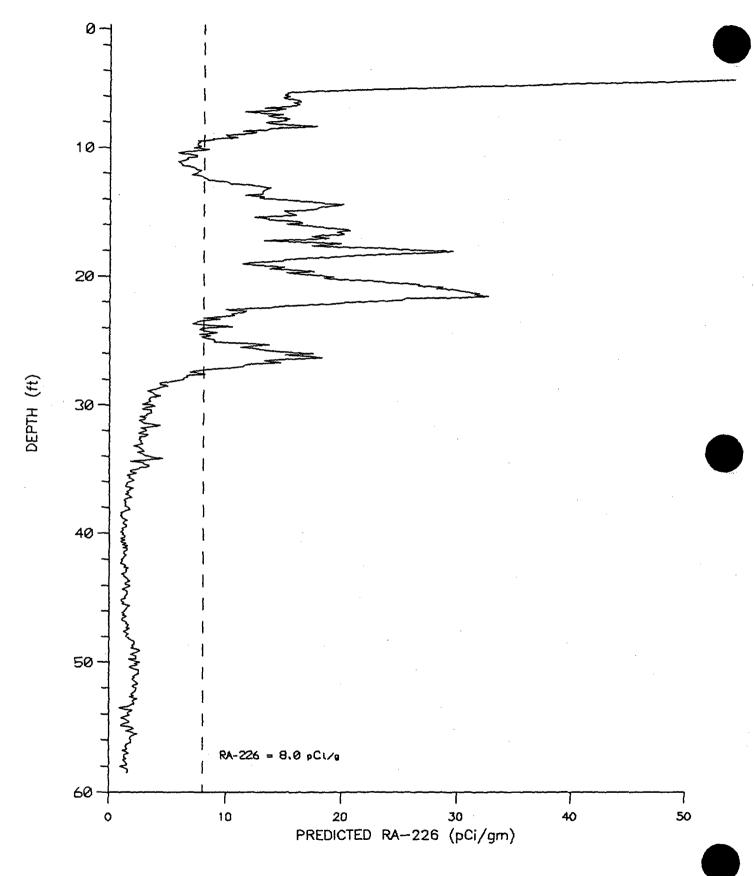


FIGURE A.3-36. PREDICTED RA-226 (pC1/g) IN TEST HOLE LGW-4.

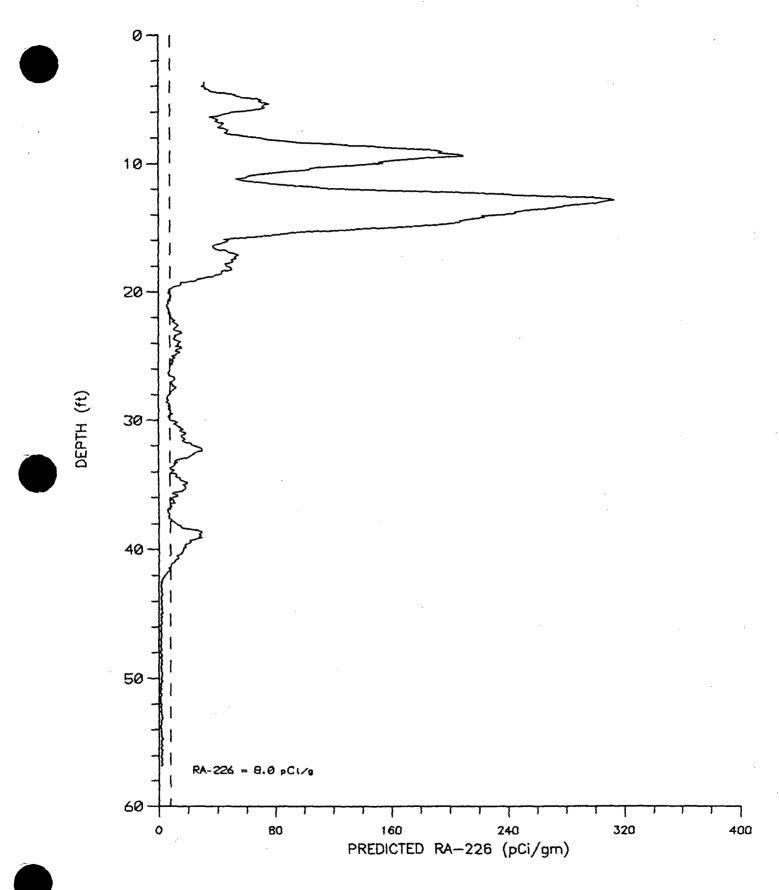


FIGURE A.3-37. PREDICTED RA-226 (pC1/g) IN TEST HOLE LGW-5.

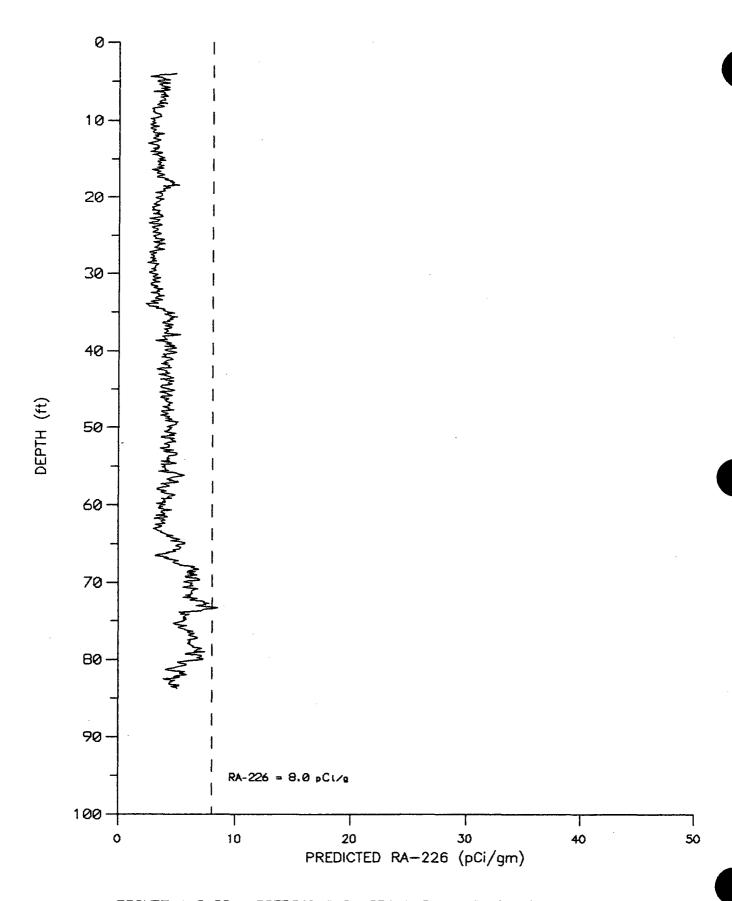


FIGURE A.3-38. PREDICTED RA-226 (pC1/g) IN TEST HOLE STH-1A.

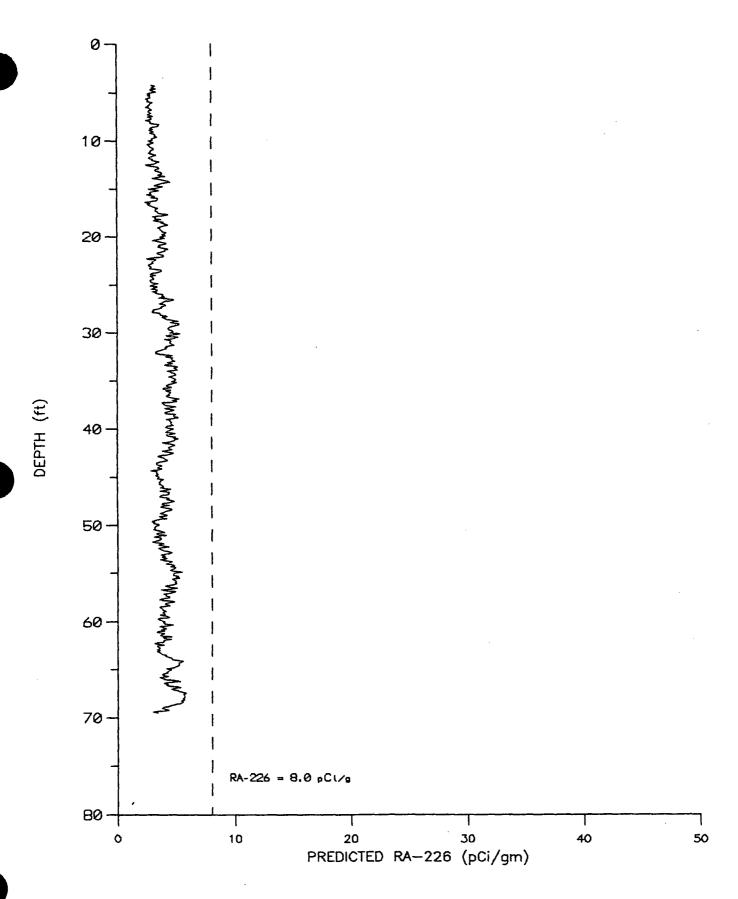


FIGURE A.3-39. PREDICTED RA-226 (pC1/g) IN TEST HOLE STH-3.

APPENDIX B BACKHOE PIT INFORMATION

APPENDIX B

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B.1 APPENDIX B TEXT

This appendix contains gamma profiles for the backhoe pits dug during this study. The gamma values presented are hand-held gamma meter readings taken in the field by placing the probe at the specified interval against the pit wall. Intervals of six inches or one foot were used to define the profile. The hand-held gamma meter - laboratory Ra-226 relationships are described in Section 2.4. Table 2-1 gives the radium results for the pit samples. The values in the tables in this Appendix are presented in $\mu R/hr$ times $\emptyset.1$.

TABLE B-1. RADIOLOGICAL DATA FOR BACKHOE PITS P4-1A, P4-1B, P4-1C, P4-2A, P4-2B, P4-3A, P4-3B, & P4-3C.

Depth From Surface	Pit P4-1A Gamma	Pit P4-1B Gamma	Pit P4-1C Gamma	
Ø"-6" 6"-12" 12"-18" 18"-24" 24"-3Ø" 3Ø"-36" 36"-42" 42"-48" 48"-54" 54"-6Ø" 6Ø"-66"	1.2 1.4 1.4 1.4 1.6 1.8 1.8 1.4 1.2	6 6.8 7 4 2.2 3 2.6 2.8 2.4 2.6 2.2 2.6	6 9 2.2 1.8 2 1.8 2 1.4	
Depth From Surface	Pit P4-2A Gamma	Pit P4-2B Gamma		
Ø"-6" 6"-12" 12"-18" 18"-24" 24"-3Ø" 3Ø"-36" 36"-42" 42"-48" 48"-54" 54"-6Ø" 6Ø"-66" 66"-72" 72"-78"	2.8 2.8 2.6 2.4 2.2 2.4 2.4 2.4 2.2 1.2	3.2 3 3.2 3.2 3.7 2.4 2.2 2		
Depth From Surface	Pit P4-3A Gamma	Depth From Surface	Pit P4-3B Gamma	Pit P4-3C Gamma
Ø"-6"	2.5	Ø"-12"	28	6
6"-12" 12"-18"	2.4 2.4	12"-24"	14	3.2
18"-24" 24"-3Ø"	2.3 2.2	24"-36"	6.5	2.2
3Ø"-36" 36"-42" 42"-48"	1.8 1.6 1.6	36"-48"	5.5	
42"-48 48"-54" 54"-6Ø" 6Ø"-66"	1.6 1.4 1.5	48''-6Ø''	5.9	

TABLE B-2. RADIOLOGICAL DATA FOR BACKHOE PITS P4-4A, P4-4B, P4-4C, P4-4D, P4-5A, P4-5B, P4-5C, & P4-5D.

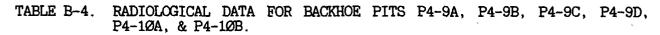
Depth From Surface	Pit P4-4A Gamma	Pit P4-4B Gamma	Pit P4-4C Gamma	Pit P4-4D Gamma
0"-6" 6"-12" 12"-18" 18"-24" 24"-30" 30"-36" 36"-42" 42"-48" 48"-54" 54"-60" 60"-66" 66"-72" 72"-78" 78"-84" 84"-90" 90"-96" 102"-108" 108"-114"	9 9 18 28 31 34 44 48 58 80 78 52 42 20 16 14 12 10	14 20 25 34 38 36 40 40 40 40 40 56 58 54 58 54	14 18 24 3Ø 37 38 42 52 58 96 12Ø 2ØØ 22Ø 56 25 2Ø 2Ø 2Ø	7 8 16 24 28 42 8Ø 4Ø 18 12 1Ø
114"-12Ø" Depth From	Pi+ P4-54	58 Pi+ P4-5R	Pi+ P4-5C	Denth From

Depth From Surface	Pit P4-5A Gamma	Pit P4-5B Gamma	Pit P4-5C Gamma	Depth From Surface	Pit P4-5D Gamma
Ø"-12"	1.6	6	4.8	Ø"-6"	11
12"-24"	1.4	12	24	6"-12" 12"-18"	14 16
24"-36"	1.6	4.4	44	18"-24" 24"-30"	24 4Ø
36"-48"	6.5	1.6	36	3Ø"-36" 36"-42"	62 95
48"-60"	3.8	1.4	9	42"-48" 48"-54"	15Ø 6Ø
6Ø''-72	1.8	1.4	6.5	54"-60" 60"-66"	18 12
72"-84"	1.2	1.2	5.5	66"-72" 72"-78"	8.5 <u>8</u>
84"-96"	1.4			78"-84" 84"-90" 90"-96"	7 6 1.2

TABLE B-3. RADIOLOGICAL DATA FOR BACKHOE PITS P4-6A, P4-6B, P4-7A, P4-7B, P4-8A, & P4-8B.

Field Gamma Readings in µR/hr x 10-1

Depth From Surface	Pit P4-6A Gamma	Pit P4-6B Gamma	
Ø"-12" 12"-24" 24"-36" 36"-48" 48"-6Ø" 6Ø"-72" 72"-84" 84"-96"	12 3Ø 28 32 32 32 36 1Ø	12 28 32 38 42 42 44 44	
Depth From Surface	Pit P4-7A Gamma	Pit P4-7B Gamma	
Ø"-12" 12"-24" 24"-36" 36"-48" 48"-6Ø" 6Ø"-72" 72"-84" 84"-96"	9 27 28 3Ø 5Ø 19 5.2 3.4	14 3Ø 42 3Ø 3Ø 3Ø 32	
Depth From Surface	Pit P4-8A Gamma	Depth From Surface	Pit P4-8B Gamma
Ø"-6" 6"-12"	2Ø 22	Ø"-12"	6
12"-18" 18"-24"	26 32	12"-24"	13
24"-3Ø" 3Ø"-36"	58 11	24"-36"	32
36"-42" 42"-48" 48"-6Ø"	6 4.4 32	36"-48"	34



Depth From Surface	Pit P4-9A Gamma	Pit P4-9B Gamma	Pit P4-9C Gamma	Depth From Surface	Pit P4-9D Gamma
Ø"-6" 6"-12"	2 2	2.8 2.4	4.5 6.8	Ø"'-12"	42
12"-18" 18"-24"	$\begin{array}{c} \textbf{1.6} \\ \textbf{1.4} \end{array}$	2 1.6	9.1 15	12"-24"	200
24''-3Ø'' 3Ø''-36''	1.2 1.2	1.4 1.2	18 24	24"-36"	1 <i>0</i> /0 48
36"-42" 42"-48"	1 1	1.2 1.2	18 14	36"-48"	34 28
48"-54" 54"-6Ø" 6Ø"-66"	1.2	1.1 1 1	6 4.4 4	48"-6Ø"	32
66"-72" 72"-78" 78"-84"		ĩ	3.2 3.2 2.8		

Depth From	Pit P4-10A	Pit P4-1ØB
Surface	Gamma	Gamma
Ø"-6" 6"-12" 12"-18" 18"-24" 24"-3Ø" 3Ø"-36" 36"-42" 42"-48" 48"-54" 54"-6Ø" 6Ø"-66" 66"-72" 72"-78" 78"-84"	2Ø 28 3Ø 38 26 42 44 22 12 10 11 12 12	3.6 3.4 2.8 2.4 1.8 1.8 1.6 1.4 1.2

TABLE B-5. RADIOLOGICAL DATA FOR BACKHOE PITS P4-11A, P4-11B, P4-11C, P4-12A, P4-12B, P4-12C, & P4-12D.

Field Gamma Readings in μ R/hr x 10-1

Depth From	Pit P4-11A	Pit P4-11B	Pit P4-11C
Surface	Gamma	Gamma	Gamma
Ø"-6" 6"-12" 12"-18" 18"-24" 24"-3Ø" 3Ø"-36" 36"-42" 42"-48" 48"-54" 54"-6Ø" 6Ø"-66" 66"-72" 72"-78" 78"-84" 84"-9Ø" 9Ø"-96"	4.5 6.5 2Ø 24 32 42 5 3.2 2.4 2.2 1.8 1.7 1.8 1.6 1.8	9 16 18 14 8 3.8 3.2 2.6 2.5 2.2 2.5	3.6 5 4 2 1.4 1.4

Depth From	Pit P4-12A	Pit P4-12B	Pit P4-12C	Pit P4-12D
Surface	Gamma	Gamma	Gamma	Gamma
Ø"-6" 6"-12" 12"-18" 18"-24" 24"-3Ø" 3Ø"-36" 36"-42" 42"-48" 48"-54" 54"-6Ø" 6Ø"-66" 66"-72" 72"-78" 78"-84" 84"-9Ø" 9Ø"-96"	2.4 2.1 3 4.2 2.2 1.8 1.8 1.4 1.2 1.2	3 3.1 4.3 2.5 2.6 2 1.7 1.4 1.6 1	5 6.2 7.8 8.4 9 9 11 18 22 1Ø 8 5.2 4.2 5	1.4 1.2 1.3 1.2 1.2 1.2 1.2 1.2 1.2 1.2

TABLE B-6. RADIOLOGICAL DATA FOR BACKHOE PITS P4-13A, P4-13B, P4-13C, P4-14A, P4-14B, P3-1, P3-2, P3-3 & P3-4.

Depth From Surface	Pit P4-13A Gamma	Pit P4-13B Gamma	Pit P4-13C Gamma	
Ø"-6" 6"-12" 12"-18" 18"-24" 24"-3Ø" 3Ø"-36" 36"-42" 42"-48" 48"-54" 54"-6Ø" 6Ø"-66"	2.2 2.4 2.2 2.2 2.2 2.2 2.2 2.2 2.2	1.2 1.6 2 1 1.2 1	1.6 1.6 2.4 2.3 2.1 1.1 1.1	
Depth From Surface	P4-14A Gamma	Pit P4-14B Gamma		
Ø"-6" 6"-12" 12"-18" 18"-24" 24"-3Ø" 3Ø"-36" 36"-42" 42"-48" 48"-54" 54"-6Ø" 6Ø"-66"	2 2.2 2.2 2.4 2.5 2.4 2.8 3 3.4 2.8	2.3 2 1.6 1.5 1.2 1.3 1.4 1.3		
Depth From Surface	Pit P3-1 Gamma	Pit P3-2 Gamma	Pit P3-3 Gamma	Pit P3-4 Gamma
Ø"-6" 6"-12" 12"-18" 18"-24" 24"-3Ø" 3Ø"-36" 36"-42" 42"-48"	2.6 2.7 2.4 2 2.8 2.8 2.6 1.4	9.5 14 18 2Ø 14 1Ø 8 6.5	12 17 18 9 6 5	4.4 4.8 5.2 5.2 5.2

TABLE B-7. RADIOLOGICAL DATA FOR BACKHOE PITS P5-1A, P5-1B, P5-2, O-1, & O-2.

Depth From	Pit P5-1A	Pit P5-1B	Pit P5-2
Surface	Gamma	Gamma	Gamma
Ø"-6" 6"-12" 12"-18" 18"-24" 24"-3Ø" 36"-42" 42"-48" 48"-54" 54"-6Ø" 6Ø"-66" 66"-72" 72"-78"	5.8 7.5 12 16 18 18 18 18 20 18 18 16 20	5.2 3 12 14 14 14 16 18 2Ø 2Ø 2Ø 2Ø 24	2.2 2.2 2.2 2.2 2.2 1.7 1.6 1.4 1.5 1.4

Depth From Surface	Pit 0-1 Gamma	Depth From Surface	Pit O-2 Gamma
Ø"-6"	4.4		
6"-12"	6	Ø"-12"	3.4
12"~18"	5.5		
18"-24"	5.5	12"-24"	4.6
24"-36"	5.8	24"-36"	5.8
36"-48"	7	36"-48"	8.5
48''-6Ø''	1Ø	48"-60"	12
60"-72"	8.5	6Ø"-72"	14
72"-84"	9	72"-84"	16
84"-96"	9	84"-96"	40
96"-1Ø8"	7.5	96"-1Ø8"	20
1Ø8"-12Ø"		108"-120"	28
120"-132"		120"-132"	24

APPENDIX C LABORATORY REPORTS - MATERIALS PROPERTIES

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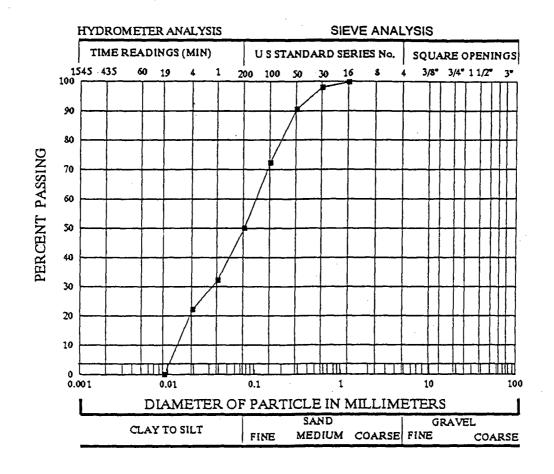
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SUMMARY OF LABORATORY TEST RESULTS PATHFINDER - SHIRLEY BASIN TAILINGS

TABLE C.1-1. SUMMARY OF TAILINGS PHYSICAL PROPERTIES FROM DRILLING SAMPLES.

			NATURAL	ATTERI	BERG LIMITS	GRADATION ANALYSIS		WATER		
BORING	DEPTH	NATURAL	DRY	LIQUID	PLASTICITY		#4	İ	SOLUBLE	SOIL CLASSIFICATION
	(ft)	MOISTURE	DENSITY	LIMIT	INDEX	+ #4	+ #200	- #200	SULFATE	
		(%)	(pcf)	(%)	(%)	(%)	(%)	(%)	(%)	
										SANDY LEAN CLAY/CLAYEY
TW4-1B	25-27	36	85	43	20	0	50	50		SAND (CL/SC)
								ļ		
		1]							POORLY GRADED SAND WITH
TW4-4B	5-7	20	93	22	NP	0	92	8		SILTY (SP-SM)
		ļ		 	ļ	 		10	 	
TW4-5B	5.5-8	16	91	30	NP	0	84	16		SILTY SAND (SM)
			 		 	 	<u> </u>	 	<u> </u>	DOODLY CD A DED SAND WITH
TW4-1C	3-5	20	94	23	NP	0	95	5		POORLY GRADED SAND WITH
144-10	3-5	20	94	23	NF	-	93		 	SILT (SP –SM)
TW4-2C	40-42	80	55	103	62	0	23	77	 	FAT CLAY WITH SAND (CH)
		 				1				
TW4-3C	42-43	62	62	84	55	0	0	100		FAT CLAY (CH)
		<u> </u>	<u> </u>	<u></u>					<u> </u>	
TW5-2B	3	27	89	NV	NP	0	94	6		POORLY GRADED SAND WITH SILT (SP – SM)
		 	ļ	 		 	ļ	 		
<u> </u>	 	 				 			 	
				ļ		ļ	ļ			
		 	 	 	 	 	 	}	<u> </u>	
L	<u> </u>		<u> </u>	L	<u> </u>		L	L	<u> </u>	<u> </u>



PARTICLE	FRACTIONOF
SIZE	SAMPLE(%)
GRAVEL	0
SAND	<i>5</i> 0
FINES (SILT AND CLAY)	50
ATTERBERGLI	MITS
LIQUID LIMIT (%)	43
PLASTICITY INDEX (%)	20
• • •	•

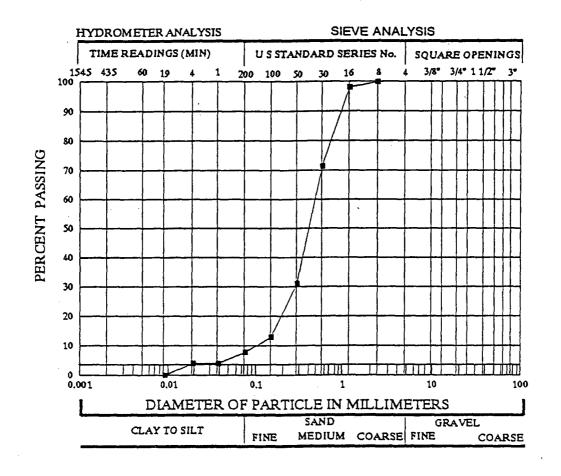
SAMPLE: TW4-1B DEPTH (ft): 25 to 27

SOIL CLASSIFICATION: SANDY LEAN CLAY/CLAYEY SAND (CL/SC)

PATHFINDER – SHIRLEY BASIN TAILINGS HYDRO-ENGINEERING

I	HCN 9~12		
1	,	HUNTINGDON	
	93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS

FIGURE C.1-1. GRADATION AND ATTERBERG LIMIT RESULTS FOR SAMPLE TW4-1B (25'-27').



PARTICLE	FRACTIONOF
SIZE	SAMPLE(%)
GRAVEL	0
SAND	92
FINES (SILT AND CLAY)	. 8
ATTERBERGLI	MITS
LIQUID LIMIT (%)	22
PLASTICITY INDEX(%)	NP

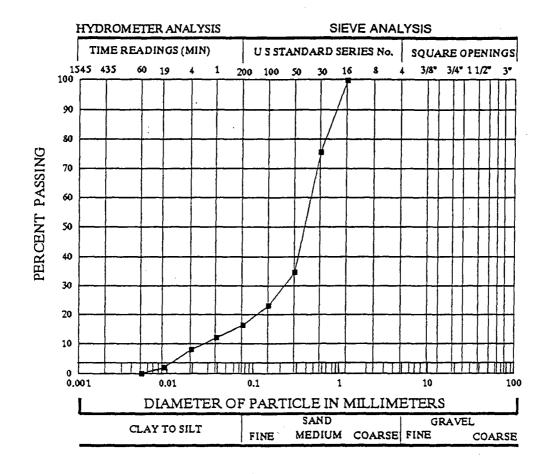
SAMPLE: TW4-4B DEPTH (ft): 5 to 7

SOIL CLASSIFICATION: POORLY GRADED SAND WITH SILT (SP-SM)

PATHFINDER - SHIRLEY BASIN TAILINGS HYDRO-ENGINEERING

1	HCN 9-12		
		HUNTINGDON	
	93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS

FIGURE C.1-2. GRADATION AND ATTERBERG LIMIT RESULTS FOR SAMPLE TW4-4B (5'-7').



PARTICLE	FRACTIONOF	
SIZE	SAMPLE (%)	
GRAVEL	0	
SAND	84	
FINES (SILT AND CLAY)	16	

ATTERBERG LIMITS

LIQUID LIMIT (%) 30
PLASTICITY INDEX (%) NP

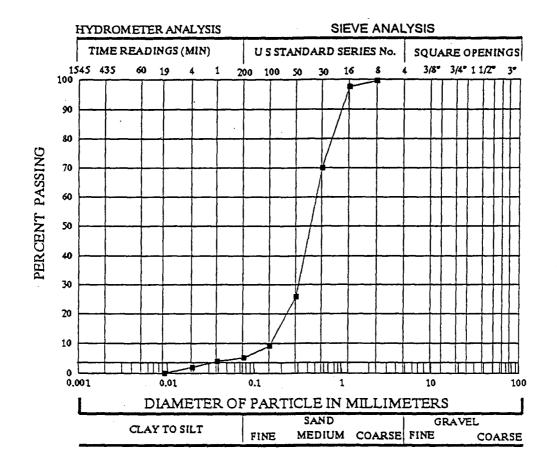
SAMPLE: TW4-5B DEPTH (ft): 5.5 to 8

SOIL CLASSIFICATION: SILTY SAND (SM)

PATHFINDER - SHIRLEY BASIN TAILINGS HYDRO-ENGINEERING

HUNTINGDON
93-4511 CHEN-NORTHERN, INC. GRADATION ANALYSIS

FIGURE C.1-3. GRADATION AND ATTERBERG LIMIT RESULTS FOR SAMPLE TW4-5B (5.5'-8').



PARTICLE	FRACTIONOF
SIZE	SAMPLE(%)
GRAVEL	0
SAND	95
FINES (SILT AND CLAY)	5
ATTERRERGI	IMITS

LIQUID LIMIT (%)

PLASTICITY INDEX(%) NP

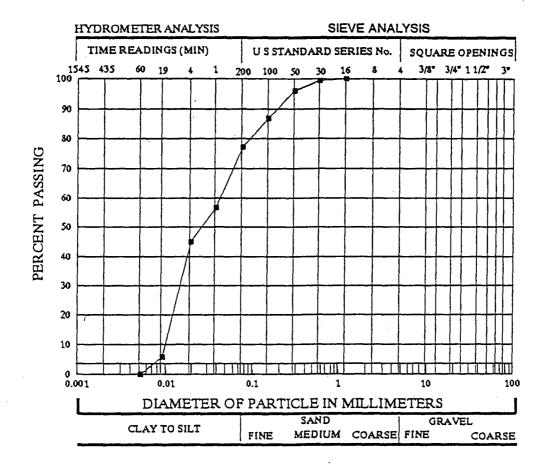
SAMPLE: TW4-1C DEPTH (ft): 3 to 5

SOIL CLASSIFICATION: POORLY GRADED SAND WITH SILT (SP-SM)

PATHFINDER – SHIRLEY BASIN TAILINGS HYDRO – ENGINEERING

HUNTINGDON
93-4511 CHEN-NORTHERN INC. GRADATION ANALYSIS

FIGURE C.1-4. GRADATION AND ATTERBERG LIMIT RESULTS FOR SAMPLE TW4-1C (3'-5').



PARTICLE	FRACTIONOF
SIZE	SAMPLE(%)
GRAVEL	. 0
SAND	23
FINES (SILT AND CLAY)	77
ATTERBERG LII	MITS
LIQUID LIMIT (%)	103
PLASTICITY INDEX(%)	62

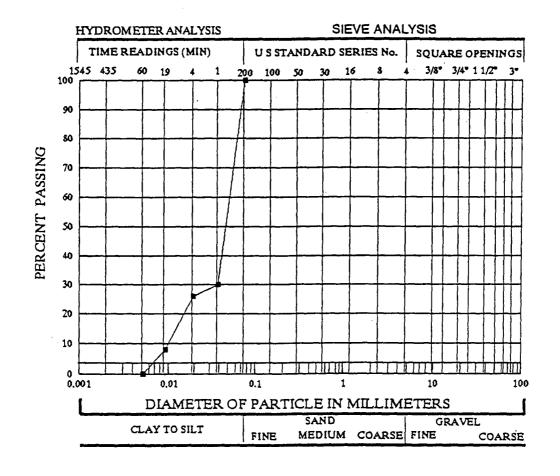
SAMPLE: TW4-2C DEPTH (ft): 40 to 42

SOIL CLASSIFICATION: FAT CLAY WITH SAND (CH)

PATHFINDER - SHIRLEY BASIN TAILINGS HYDRO-ENGINEERING

HC	-W A→15		
		HUNTINGDON	
9:	3-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS

FIGURE C.1-5. GRADATION AND ATTERBERG LIMIT RESULTS FOR SAMPLE TW4-2C (40'-42').



PARTICLE	FRACTIONOF	
SIZE	SAMPLE(%)	
GRAVEL	0.	
SAND	0	
FINES (SILT AND CLAY)	100	
· · · · · · · · · · · · · · · · · · ·		
ATTERRERGI	IMITS	

LIQUID LIMIT (%) 84
PLASTICITY INDEX (%) 55

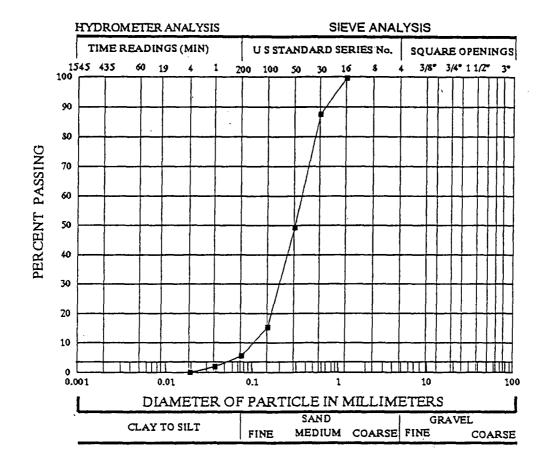
SAMPLE: TW4-3C DEPTH (ft): 42 to 43

SOIL CLASSIFICATION: FAT CLAY (CH)

PATHFINDER - SHIRLEY BASIN TAILINGS HYDRO-ENGINEERING

HCN 9~12		
[HUNTINGDON	
93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS

FIGURE C.1-6. GRADATION AND ATTERBERG LIMIT RESULTS FOR SAMPLE TW-3C (42'-43').



PARTICLE	FRACTIONOF
SIZE	SAMPLE(%)
GRAVEL.	0
SAND	94
FINES (SILT AND CLAY)	6
ATTERBERG LI	MITS
LIQUID LIMIT (%)	NV

SAMPLE: TW5-28 DEPTH (ft): 3

NP

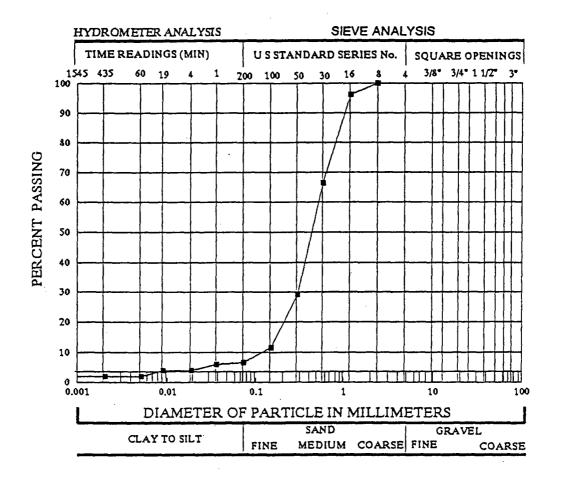
SOIL CLASSIFICATION: POORLY GRADED SAND WITH SILT (SP-SM)

PATHFINDER - SHIRLEY BASIN TAILINGS HYDRO-ENGINEERING

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PLASTICITY INDEX(%)

FIGURE C.1-7. GRADATION AND ATTERBERG LIMIT RESULTS FOR SAMPLE TW5-2B (3').



PARTICLE SIZE	FRACTION OF SAMPLE (%)	
GRAVEL	0	
SAND	94	
FINES (SILT AND CLAY)	6	
ATTERBERG L	IMITS	
PLASTICITY INDEX	NP	
SAMPLE TO	W 5-3	
DEPTH (ft)	: 6'-8'	
SOIL CLASSIF	ICATION:	

HYDRO-ENGINEERING

SAND WITH SILT (SP-SM)

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93-4511 CHEN-NORTHERN, INC. GRADATION ANALYSIS

FIGURE C.1-8. GRADATION AND ATTERBERG LIMIT RESULTS FOR SAMPLE TW5-3 (6'-8').

HUNTINGDON

CHEN-NORTHERN, INC.
TIME-CONSOLIDATION, SWELL-SETTLEMENT TEST

JOB NUMBER: タスームテリ	BORING: TINE SOC.
JUB NUMBER. 43-4511	BOHING: 11774 - DC
JOB NAME: Hydro Enoineering (Southfinder-	DEPTH: AO -4つ1
VISUAL SOIL DESCRIPTION: JShirley Bain-Tallings	BLOW CT: —
7	DATE: 2-2-33
	BY: ())
	1

MACHINE NO: ___

p		,	·													
LOAD (psf):	5	LOAD.	(psf):	LOAD ((psf):	<u>a</u>	LOAD (psf): Д								
DATE:	3-8	-93	DATE:	2-7	-93	DATE:	3-10	-43	DATE: 3-11-93							
BY:			BY:			BY:	•		BY:							
стоск	ELAPSED	DIAL	стоск	ELAPSED	DIAL	стоск	ELAPSED	DIAL	стоск	ELAPSED	DIAL					
TIME	TIME	READING	TIME	TIME	READING	TIME	TIME	READING	TIME	TIME	READING					
	(MIN)			(MIN)			(MIN)			(MIN)						
7:35	0	1825	7:50	0 -	1423	7:28	0	1890	7:52	0	1157					
	0.1	1795		0.1	1= 25		0.1	15193		0.1	142					
	0.25	1792		0.25	13 ~9		0.25	1587		0.25	1120					
	0.5	1790		0.5	1374		0.5	والإقدا		0.5	, f əÚ					
731	1	דצרו	7·51	1	.1370	7:29	1	1973	7:53	1	1/27					
7:37	2	.1725	53	2	1371	.30	2	15164	54	2	1134					
7:39	4	mzo	54	4	1320	35	4	1255	57,	4	1025					
7.43	8	1774	5%	8	1363	3),	8	1549	2.00	8	10 7-6					
7.50	15	17/1/	8:05	15	.13.59	7:43	15	13 45	2.197	15	10:11					
2.05	30	17.77	8,20	30	13 -5	7:58	30	18410	8:22	30	1075					
3:35	1 hr.	1724	P.50	1 hr.	1349	8: 28	1 br.	138	8.50	1 hr.	101,8					
7:35	2 hrs.	1621	9:50	2 hrs.	1744	9:38	2 hrs.	1246	7.52	2 hrs.	10/2					
11:25	4 hrs.	11271	11:50	4 hrs.	13=7	11:28	4 hrs.	133%	11:52	4 hrs.	1255					
-3:25	8 hrs.	1482	7:50	8 hrs.	1317	3 -2/	8 hrs.	1227	<u>,२:ऽन्</u>	8 hrs.	। ञन्					
77.25	24 hrs.	1423	رايد . در .	24 hrs.	1290	7:27	24 hrs./	1157	7:53	24 hrs.	0984)					
REMAR	KS:		REMAR	,		REMAR	KS:		REMAR	KS:						
117.2	: n 1 d's															
Í	= 0.0	1		D.004	14	⊋ ₹	0.50	2-	ج) =	<u>`````</u>						
				,												

FIGURE C.1-9. CONSOLIDATION TEST WORKSHEET FOR SAMPLE TW4-2C (40'-42').

HUNTINGDON

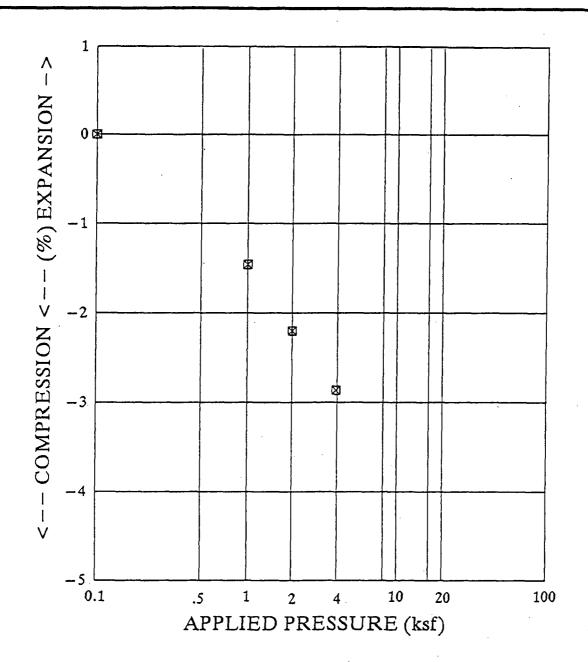
CHEN-NORTHERN, INC.
TIME-CONSOLIDATION, SWELL-SETTLEMENT TEST

JOB NUMBER: 93-4/5//	BORING: TU4-18
JOB NAME: Huden Engineering (Pathinder	DEPTH: <u>35-37</u> '
VISUAL SOIL DESCRIPTION - Shiring Tailing)	BLOW CT: —
	DATE: 3-8-93
Sand promother (Charles trans) (California	BY: 30+

MACHINE NO: 3

1040			LOAD	·		LOAD	·		I DAD (not):									
	psf):		LOAD ((psf):		LOAD (psf): 4									
DATE:	3-8-	93	DATE:	2	1-93	DATE:	3-11	73	DATE: ? -11-93									
BY:			BY:			BY:	· y 		BY:									
стоск	ELAPSED	DIAL	СТОСК	ELAPSED	DIAL	аоск	ELAPSED	DIAL	СТОСК	ELAPSED	DIAL							
TIME	TIME	READING	TIME	TIME	READING	TIME	TIME	READING	TIME	TIME	READING							
ė.	(MIN)			(MIN)		,	(MIN)			(MIN)	<u> </u>							
7:41	0 (1971	2:01	0	1237	7:34	0	1533	2:00	0	119%							
	0.1	1913		0.1	1410:		0.1	1577		0.1	1195							
	0.25	1909		0.25	1808		0.25	1575		0.25	1198							
	0.5	1905		0.5	1705		0.5	157~3		0.5	1177							
42	1	1891	201	1	1794	7:35	1	1543	2:01	1	1120							
4/3	2	1890	<u> </u>	2	1791	36	2 .	1525		2	1105							
45-	4	1887	4	4	1725	38	4	15-14-	4/	4	1095							
49	8	1880.	1	8	1780	4/2	8	1502	*	8	102%							
7:5%	15	1875-	- ، رح	15	1778	7:49	15	1464	2:15	15	1070							
2:11	30	18/1/2	200	30	1775	2.04	30	1415-	1:20	30	1053							
.7.4)	1 hr.	1811	2.30	1 hr.	17771	7:24	1 hr.	1207	7:00	1 hr.	112413							
9:41	2 hrs.	1245	10:00	2 hrs.	1745	9:34	2 hrs.	1388	11):00	2 hrs.	1014							
11:41	4 hrs.	1224	13:00	4 hrs.	17.13	11:34	4 hrs.	1343	13:00	4 hrs.	.0994							
7:41	8 hrs.	1875	460	8 hrs.	1225-	3.34	8 hrs.	1203	2.00	8 hrs.	0941							
7:41	24 hrs./	1827	2.33	24 hrs. (1537	7:34	24 hrs.	11198	2100	24 hrs.	(6840)							
REMAR	KS: 17.	·h.15	REMAR	KS:		REMAR	KS:		REMAR	KS:								
ļ	<u> </u>																	
ł	- 0.00	1	1 =	000	3 ⊃	C '-	71.754	2	4=	0.00	< ∤							
																		

FIGURE C.1-10. CONSOLIDATION TEST WORKSHEET FOR SAMPLE TW4-1B (25'-27').



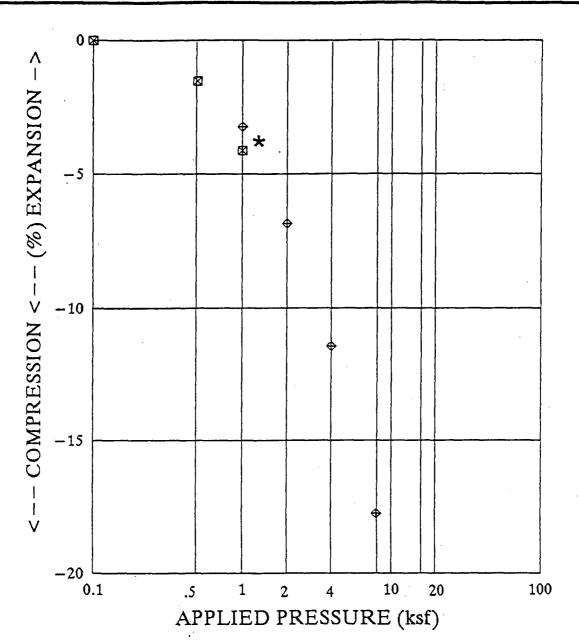
WATER CONTENT (%): 16 DRY DENSITY (pcf): 91 TESTED LOAD INCREMENTS (ksf): 1, 2, 4,

TEST SAMPLE IDENTIFICATION: SAMPLE TW4-5B DEPTH (ft): 5.5 to 8

> SOIL CLASSIFICATION: SILTY SAND (SM)

PATHFINDER – SHIRLEY BASIN TAILINGS HYDRO-ENGINEERING

HUNTINGDON
93-4511 CHEN-NORTHERN.INC. SWELL-SETTLEMENT GRAPHICS



*EXPANSION UNDER CONSTANT LOAD AFTER WETTING

WATER CONTENT (%): 62 DRY DENSITY (pcf): 62 TESTED LOAD INCREMENTS (ksf): 1, wetting, 2, 4, 8

TEST SAMPLE IDENTIFICATION:

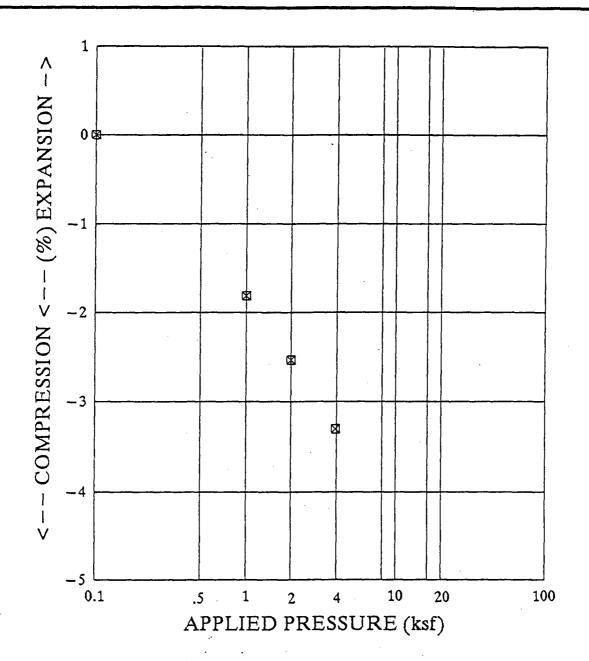
SAMPLE: TW4-3C DEPTH (ft): 42 to 43

SOIL CLASSIFICATION: FAT CLAY (CH)

PATHFINDER – SHIRLEY BASIN TAILINGS HYDRO-ENGINEERING

HUNTINGDON
93-4511 CHEN-NORTHERN.INC. SWELL-SETTLEMENT GRAPHICS

FIGURE C.1-12. CONSOLIDATION RESULTS FOR SAMPLE TW4-3C (42'-43').



WATER CONTENT (%): 20 DRY DENSITY (pcf): 94
TESTED LOAD INCREMENTS (ksf): 1, 2, 4,

TEST SAMPLE IDENTIFICATION: SAMPLE TW4-1C DEPTH (ft): 3 to 5

SOIL CLASSIFICATION: POORLY GRADED SAND WITH SILT (SP-SM)

PATHFINDER - SHIRLEY BASIN TAILINGS HYDRO-ENGINEERING

HUNTINGDON
93-4511 CHEN-NORTHERN.INC. SWELL-SETTLEMENT GRAPHICS

HUNTINGDON CHEN-NORTHERN, INC.

SUMMARY OF LABORATORY TEST RESULTS HYDRO-ENGINEERING

TABLE C.2-1. SUMMARY OF DRILLING PROGRAM FOR COVER PHYSICAL PROPERTIES TESTING.

			<u> </u>	BERGLIMITS	GRADA	TION AN	ALYSIS	·			•
BORING	DEPTH	NATURAL	rionid	PLASTICITY		- #4		SPECIFIC	ORGANIC	PERMEABILITY	SOIL CLASSIFICATION
	(ħ)	MOISTURE	LIMIT	INDEX	+ #4	+ #200	- #200	GRAVITY	CONTENT	(cm/sec)	ļ
		(%)	(%)	(%)	(%)	(%)	(%)		(%)		
STH-2	60-67	18	54	34	0	17	83		7.2		FAT CLAY WITH SAND (CH)
STH-4	30-37	15 .	61	40	0	8	92		6.2		FAY CLAY (CH)
STH-5	40-47	9	58	35	0	7	93		6.2	2.6x10-8	FAT CLAY (CH)
STH-6	20-27	6	45	25	0	17	83		5.9		LEAN CLAY WITH SAND (CL)
STH-7	40-47		57	37	0	45	55	2.36	4.2	1.54x10-9	SANDY FAT CLAY (CH)
STH-11	20-27	13	64	. 37	0	4	96		10.4		FAT CLAY (CH)
NTH-1	60-67		65	38	0	15	85	2.31	8.0	1.96x10-8	FAT CLAY WITH SAND (CH)
NTH-3	140-147	6	42	27	10	61	29	2.36			SANDY LEAN CLAY (CL)
NTH-5		16	65	42	0	10	90		10.0		FAT CLAY (CH)
	120-127	10	55	39	7	45	48				SANDY FAT CLAY (CH)
· · · · · · · · · · · · · · · · · · ·											
		L	<u>L</u>	<u> </u>	<u> </u>						

HUNTINGDON CHEN-NORTHERN, INC.

SUMMARY OF LABORATORY TEST RESULTS HYDRO-ENGINEERING

TABLE C.2-2. SUMMARY OF CP SERIES COVER MATERIAL PHYSICAL PROPERTIES TESTING.

			· · · · · · · · · · · · · · · · · · ·	ATTEDI	BERG LIMITS	GRADA	TION AN	AI VCIC		
DODING	DEBLA	NATURAL	ORGANIC	<u> </u>	PLASTICITY	CICKIDA	- #4	AL 1313	SPECIFIC	SOIL CLASSIFICATION
DOMINO		MOISTURE	MATTER	LIMIT	INDEX	1	1	#200	GRAVITY	BOIL CLIEBINICATION
	(ft)		1	1		1	(%)	(%)	GKAVIII	
		(%)	(%)	(%)	(%)	(%)	(%)	(76)		
CP-1	10	14	4.14	56	29	0	25	75	2.63	SANDY CLAY (CL)
CP-2	9	16	3.72	50	26	0	25	75	2.61	SANDY CLAY (CL)
CP-3	9	15	2.76	52	27	0	23	77		SANDY CLAY (CL)
01 - 0		13	2.70	52			20	<u>'</u>	2.07	JANDI CLAI (CL)
		 		 		 				
	<u> </u>	<u> </u>				 	 			<u> </u>
	 	 		 		 			 	
					-					
	<u> </u>			-		 	 			
	 	<u> </u>		 		 	 	 		
	 	<u> </u>		1						
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HUNTINGDON CHEN-NORTHERN, INC.

SUMMARY OF LABORATORY TEST RESULTS HYDRO-ENGINEERING

TABLE C.2-3. SUMMARY OF ND SERIES COVER MATERIAL PHYSICAL PROPERTIES TESTING.

				ATTER	BERG LIMITS	GRADA	TION AN	ALYSIS		
BORING	DEPTH	NATURAL	ORGANIC	LIQUID	PLASTICITY		- #4		SPECIFIC	SOIL CLASSIFICATION
	(ft)	MOISTURE	MATTER	LIMIT	INDEX	+ #4	+ #200	- #200	GRAVITY	
		(%)	(%)	(%)	(%)	(%)	(%)	(%)		
ND-1		30	3.6	59	33	1	22	77	2.68	I.EAN CLAY WITH SAND (CL)
ND-2		25	2.7 ·	64	36	0	27	73	2.69	LEAN CLAY WITH SAND (CL)
ND-3		35	3.7	75	40	1	23	76	2.60	LEAN CLAY WITH SAND (CL)
ND-4		36	3.1	69	36	0	25	75	2.66	LEAN CLAY WITH SAND (CL)
ND-5		23	3,3	61	33	0	14	86	2.66	LEAN CLAY (CL)
ND-6		23	2.6	65	34	1	27	72	2.63	LEAN CLAY WITH SAND (CL)

TABLE C.2-4. SUMMARY OF CP SERIES COVER MATERIAL PERMEABILITY TESTING.

HUNTINGDON CHEN-NORTHERN, INC.

SUMMARY OF PERMEABILITY TESTING HYDRO-ENGINEERING

Sample ID	Maximum Dry Density (pcf)	Optimum Water Content (%)	Remolded Density (pcf)	Percent of Maximum Dry Density (%)	Coefficient of Permeability (cm/sec)
CP-1	92.8	26.5	88.3	95	7.6x10 ⁻⁹
CP-3	94.9	27.5	90.2	95	7.7x10 ⁻⁸
CP-2	96,4	24.3	91.8	95	3.9x10 ⁻⁸

TABLE C.2-5. SUMMARY OF SET SERIES COVER MATERIAL PERMEABILITY TESTING.

INBERG-MILLER ENGINEERS

1120 EAST "C" STREET

CASPER. WYOMING 82601-2195

307-377-0806

August 27, 1991

5390-CM

Hydro Engineering 770 East Magnolia Casper, WY 82604

ATTENTION: LARRY STEELE

RE: LABORATORY TESTING

Gentlemen:

As requested, we have performed laboratory testing on samples submitted to our laboratory on June 13, 1991. The tests performed included moisture-density analysis, permeability, hydrometer, organic content, and specific gravity.

The results of the testing are as follows:

	Permeability(cm/sec)	Organic Content (%)	Specific <u>Gravity</u>
Sec #1	2.6x10 ⁻⁷ (90x) 5.3x10 ⁻⁹ (95x) 3.6x10 ⁻⁹ (100x)	8.8	2.680
Set #2	2.3x10 ⁻⁶ (90x) 3.6x10 ⁻⁸ (95x) 6.7x10 ⁻⁹ (100x)	8.0	2.672
Set #3	8.2x10 ⁻⁹ (90%) 3.5x10 ⁻⁹ (95%) 2.0x10 ⁻⁹ (100%)	7.1	2.702

The numbers in parentheses indicate percent of maximum density as determined by Standard Proctor. The soil for each sample was remolded to the percentage of compaction shown before each permeability test was conducted.

The moisture-density and hydrometer analyses are plotted on the attached forms.

Please contact us if you have any questions regarding these testing results or if we can be of additional assistance.

Sincerely,

INBERG-MILLER ENGINEERS

James D. Filkins, E.I.T.

Civil Engineer

JDF:cag:ltr

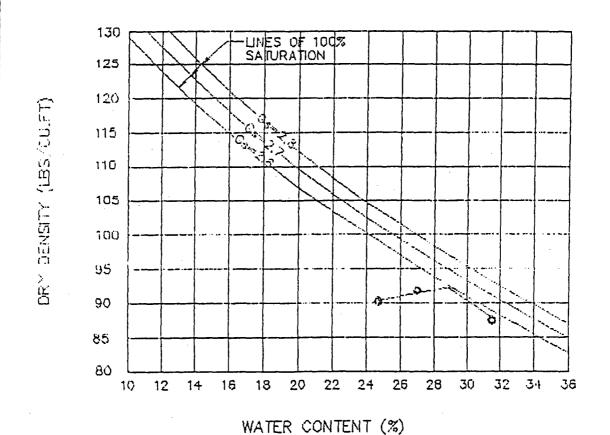
Enclosures as stated

MOISTURE-DENSITY ANALYSIS

PROJECT: SOIL TESTING TEST DATE: 6-24-81

JOB NO: 5390-CM TESTED BY: SRE

CLIENT: HYDRO ENGINEERING TEST METHOD: ASTM D-698
METHOD "A"



SOIL DESCRIPTION: PALE BROWN CLAY SHALE, LITTLE COAL % PASSING #200 SIEVE:	SAMPLED BY:CLIENT SET #1
LIQUID LIMIT:	OPTIMUM WATER CONTENT: 25.8 %
PLASTICITY INDEX:	MAX, DRY DENSITY: 92.5 (LBS/CU.FT.)

INBERG - MILLER ENGINEERS

PARTICLE SIZE ANALYSIS

	JC	ROJECT: Soil Testing OB NO: 5390-CM UENT: Hydro Engineering															TEST DATE: 6/26/91 TESTED BY: TDT TEST METHOD: ASTM D422										·														
		US STANDARD SIEVE OPENINGS (inches) (numbers) 3 2 1 1/2 4 8 16												HYDROMETER 40 100 200																											
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SOIL DESCRIPTION: Pale Brown, Clay	SOURCE: Client Set #1
Shale, Little Coal	·
SAMPLE NO: Set #1	REVIEWED BY: 571/
SAMPLED BY: Client	DATE: SPECIAL

INBERG - MILLER ENGINEERS

FIGURE C.2-2. GRADATION RESULTS FOR SAMPLE SET #1.

MOISTURE-DENSITY ANALYSIS

TEST DATE: 6-24-91

PROJECT: SOIL TESTING

JOB NO:	:5	390-	CM					TESTE	ED 8'	r: <u>. </u>	SRE			
CLIENT:		IYDRO) ENC	SINEER	RING			TEST	METH	HOD:	ASTM METI	D-6	A 98	
DRY DENSITY (LBS/CU.FT)	130 125 120 115 110 105 100 95 90 85 80			4 1	SA	ES OFFURA					ASTM METI		98 A	4 35
					1A/A	TER	COM	TFN	ين 'ينو') ۲	١				

SHALE, LITTLE COAL	SAMPLED BY: CLIENT
% PASSING #200 SIEVE:	SOURCE: CLIENT SET #2
LIQUID LIMIT%	OPTIMUM WATER CONTENT: 23.5%
PLASTICITY INDEX:#	MAX. DRY DENSITY: 93.0 (LEG/930.FT.)

INBERG - MILLER ENGINEERS

FIGURE C.2-3. STANDARD PROCTOR RESULTS FOR SAMPLE SET #2.

PARTICLE SIZE ANALYSIS

	PROJECT: Soil Testing JOB NO: 5390-CM CLIENT: Hvdro Engineering											TESTED BY: TDT																							
	US STANDARD SIEVE OPENINGS (Inches) (numbers)																•		HYD				ER	}											
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		SIEVE DESIGNATION DETERMINED PERCENT FINER SPECIFIED PERCENT FINER																																	

SOIL DESCRIPTION: Pale Brown. Clav	SOURCE: Client Set #2
Shale, Little Coal	•

SAMPLE NO: Set #2 REVIEWED BY: SAMPLED BY: DATE: Glassia

INBERG - MILLER ENGINEERS

FIGURE C.2-4. GRADATION RESULTS FOR SAMPLE SET #2.

MOISTURE-DENSITY ANALYSIS

PROJECT:	SOIL TESTING	TEST DATE:	6-24-91
JOB NO:	5390-CM	TESTED BY:	SRE.
CLIEHT:	HYDRO ENGINEERING	TEST METHOD:	ASTM D-698 METHOD "A"
130 125 120 115 110 105 109 95 90 85 80		100% N	METHIOD "A"
	WATER C	ONTENT (%)	

SOIL DESCRIPTION: PALE BROWN CLAY SHALE, LITTLE COAL	SAMPLED BY: CLIENT
% PASSING #200 SIEVE:	SOURCE: CLIENT SET #3
LIQUID LIMIT: % PLASTICITY INDEX: %	OFTIMUM WATER CONTENT: 26.3 % MAX. DRY DENSITY: 95.0 (LBS/CU.FT.)

INBERG - MILLER ENGINEERS

PARTICLE SIZE ANALYSIS

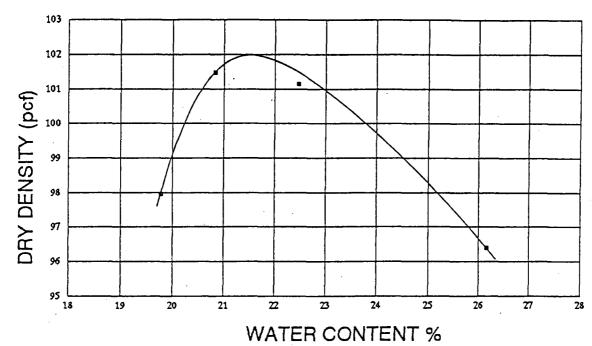
	PROJECT: Soil Testing JOB NO: 5390-CM												TEST DATE:8/26/91							 -		
												ום סב					DT					
Cl	JENT: _	Hydro		TEST METHOD: ASTM D42						42	2											
	US STANDARD SIEVE OPENINGS (Inches) (numbers)																HYDROMETER					
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	SIEY	E DESI	GNA	TION	DE	TER	MINE	P	ERI	CEN	I	INER			SE	ECI	FED	PE	RC	EN	ΙĐ	YER

SOIL DESCRIPTION: Pale Brown, Clay Shale, Little Coal	SOURCE: Client Set #3
SAMPLE NO: Set #3	REVIEWED BY: 5711
SAMPLED BY: Client	DATE: Etasker

INBERG - MILLER ENGINEERS

FIGURE C.2-6. GRADATION RESULTS FOR SAMPLE SET #3.





ASTM 698-78, METHOD A

OPTIMUM WATER CONTENT 21.5% MAXIMUM DRY DENSITY 102.0 pcf

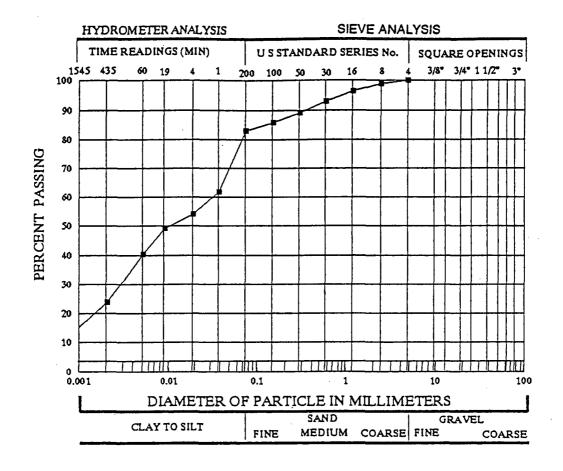
PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	0
SAND	. 17
FINES (SILT AND CLAY)	83
ATTERBERG L	IMITS
LIQUID LIMIT	54
PLASTICITY INDEX	34

SAMPLE IDENTIFICATION: BORING STH-2 DEPTH (ft): 60 to 67

> SOIL CLASSIFICATION: FAT CLAY WITH SAND (CH)

HYDRO-ENGINEERING

HUNTINGDON MOISTURE - DENSITY RELATIONSHIP Fig. 1 CHEN-NORTHERN, INC. 93-4511A



PARTICLE	FRACTIONOF
SIZE	SAMPLE(%)
CDASATT	•
GRAVEL	0
SAND	17 .
FINES (SILT AND CLAY)	. 83
ATTERBERG	LIMITS
LIMIT LIMIT	54
PLASTICITY INDEX	54 34

BORING STH-2 DEPTH (ft): 60 to 67

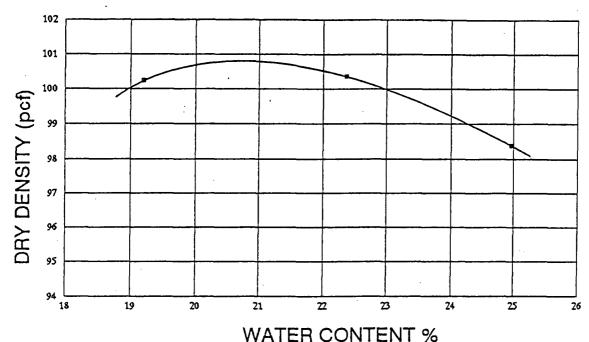
SOIL CLASSIFICATION: FAT CLAY WITH SAND (CH)

HYDRO-ENGINEERING

HUNTINGDON
93-4511A CHEN-NORTHERN, INC. GRADATION ANALYSIS Fig. 2

FIGURE C.2-8. GRADATION RESULTS FOR SAMPLE STH-2 (60'-67').





ASTM 698-78, METHOD A

OPTIMUM WATER CONTENT 20.8% MAXIMUM DRY DENSITY 100.7 pcf

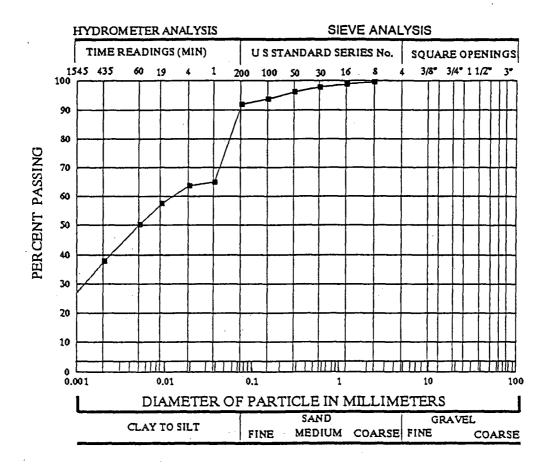
PARTICLE SIZE	FRACTION OF SAMPLE (%)
GRAVEL SAND FINES (SILT AND CLAY)	0 8 92
ATTERBERG L	JMITS 61
PLASTICITY INDEX	40

SAMPLE IDENTIFICATION: BORING STH-4 DEPTH (ft): 30 to 37

> **SOIL CLASSIFICATION:** FAT CLAY (CH)

HYDRO-ENGINEERING

HUNTINGDON MOISTURE-DENSITY RELATIONSHIP Fig. 3 93-4511A CHEN-NORTHERN, INC.



PARTICLE	PRACTION OF
SIZE	SAMPLE (%)
GRAVEL	. 0
SAND	8
FINES (SILT AND CLAY	92
ATTERBER	G LIMITS
LIQUID LIMIT	61
PLASTICITY INDEX	40
SAMPLE IDE	NTIFICATION:

FRACTIONOF

PARTICI F

BORING STH-4 DEPTH (ft): 30 to 37 SOIL CLASSIFICATION:

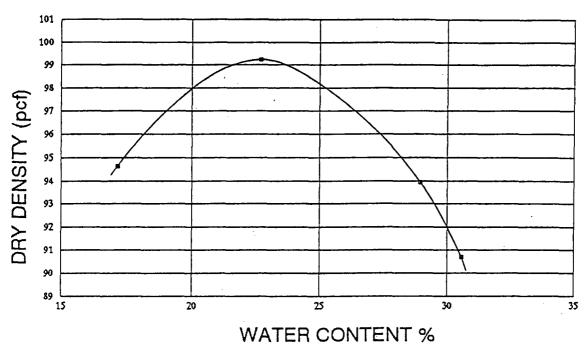
FAT CLAY (CH)

HYDRO-ENGINEERING

HUNTINGDON
93-4511A CHEN-NORTHERN, INC. GRADATION ANALYSIS Fig. 4

FIGURE C.2-10. GRADATION RESULTS FOR SAMPLE STH-4 (30'-37').





ASTM 698-78, METHOD A

OPTIMUM WATER CONTENT 22.6% MAXIMUM DRY DENSITY 99.2 pcf

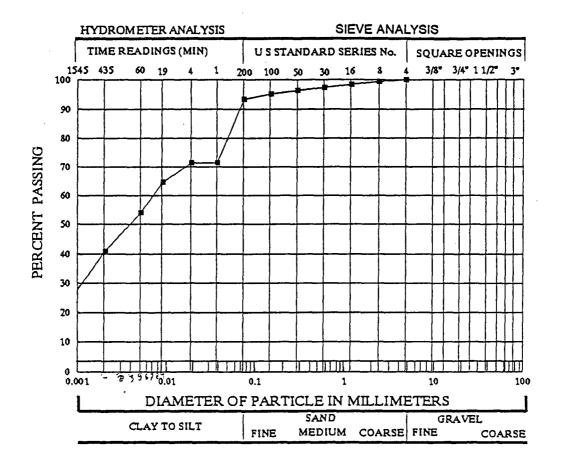
PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	0
SAND	7
FINES (SILT AND CLAY)	93
	11.4170
ATTERBERG L	IMITS
LIQUID LIMIT	. 58
PLASTICITY INDEX	35

SAMPLE IDENTIFICATION:

BORING STH-5 DEPTH (ft): 40 to 47

SOIL CLASSIFICATION: FAT CLAY (CH)

HUNTINGDON 93-4511A CHEN-NORTHERN, INC. MOISTURE-DENSITY RELATIONSHIP Fig. 5			
TO LOUIS AND A TO LOU	HUNTINGDON	,	
	93-4511A CHEN-NORTHERN, INC.	MOISTURE - DENSITY RELATIONSHIP	Fig. 5



1,1111000	110101101101
SIZE	SAMPLE(%)
GRAVEL	. 0
SAND	7
FINES (SILT AND CLAY)	93
ATTERBERG L	IMITS
LIMIT LIMIT	58
PLASTICITY INDEX	35

FRACTIONOF

PARTICLE

SAMPLE IDENTIFICATION: BORING STH-5 DEPTH (ft): 40 to 47

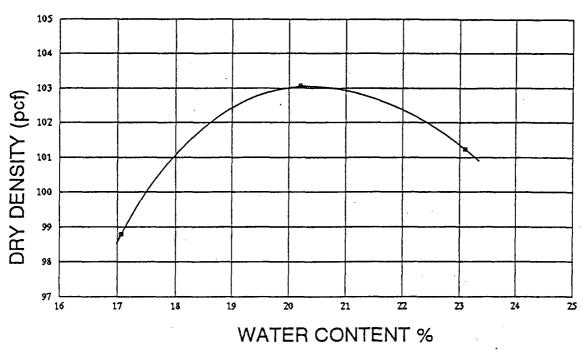
> SOIL CLASSIFICATION: FAT CLAY (CH)

HYDRO-ENGINEERING

HUNTINGDON
93-4511A CHEN-NORTHERN, INC. GRADATION ANALYSIS Fig. 6

FIGURE C.2-12. GRADATION RESULTS FOR SAMPLE STH-5 (40'-47').





ASTM 698-78, METHOD A

OPTIMUM WATER CONTENT 20.2% MAXIMUM DRY DENSITY 103.1 pcf

PARTICLE SIZE	FRACTION OF SAMPLE (%)	
OIZC	OPTIVIT ELL (70)	
GRAVEL	0	
SAND	17	
FINES (SILT AND CLAY)	83	

ATTERBERG LIMITS

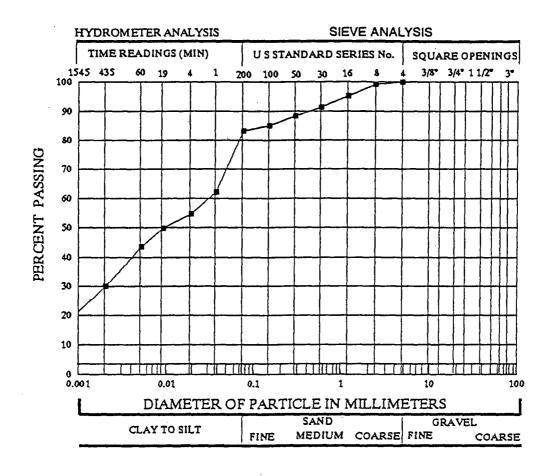
LIQUID LIMIT 45 PLASTICITY INDEX 25

SAMPLE IDENTIFICATION:

BORING STH-6 DEPTH (ft): 20 to 27

SOIL CLASSIFICATION: LEAN CLAY WITH SAND (CL)

- 3				
		HUNTINGDON]
1	93-4511A	CHEN-NORTHERN, INC.	MOISTURE - DENSITY RELATIONSHIP	Fig. 7



PARTICLE	FRACTIONOF
SIZE	SAMPLE(%)
GRAVEL	0.
SAND	17
FINES (SILT AND CLAY)	83
ATTERBERG	LIMITS
LIQUID LIMIT	45
PLASTICITY INDEX	25

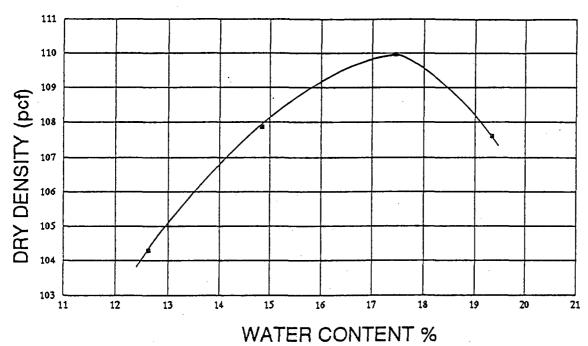
SAMPLE IDENTIFICATION:
BORING STH-6 DEPTH (ft): 20 to 27

SOIL CLASSIFICATION: LEAN CLAY WITH SAND (CL)

	HCN 9-12			
		HUNTINGDON		
ł	93-4511A	CHEN-NORTHERN, INC.	GRADATION ANALYSIS	Fig. 8

FIGURE C.2-14. GRADATION RESULTS FOR SAMPLE STH-6 (20'-27').





ASTM 698-78, METHOD A

OPTIMUM WATER CONTENT 17.5% MAXIMUM DRY DENSITY 110.0 pcf

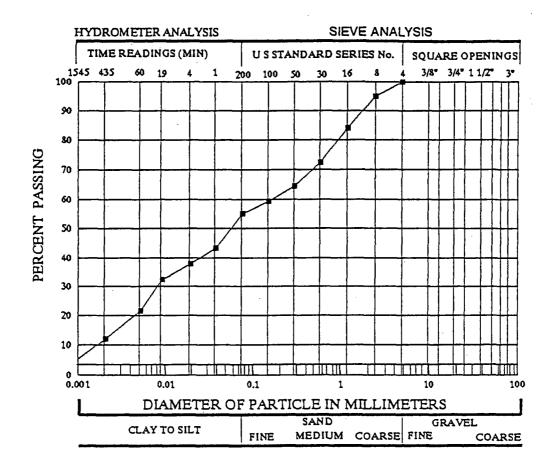
PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	. 0
SAND	45
FINES (SILT AND CLAY)	55
ATTERREDOL	IMITO
ATTERBERG L	TIMIT 2
LIQUID LIMIT	57
PLASTICITY INDEX	37

SAMPLE IDENTIFICATION:

BORING STH-7 DEPTH (ft): 40 to 47

SOIL CLASSIFICATION: SANDY FAT CLAY (CH)

		المتعرب والمناز والمناور والمناور والمناز والمناز والمناز والمناز والمناز والمناز والمناز والمناز والمناز والمناز	
	HUNTINGDON		
93-4511A	CHEN-NORTHERN, INC.	MOISTURE-DENSITY RELATIONSHIP	Fig. 9



PARTICLE	FRACTIONOF
SIZE	SAMPLE (%)
GRAVEL	0
SAND	45
FINES (SILT AND CLAY)	55
	·
ATTERBERG:	LIMITS
LIQUID LIMIT	57
PLASTICITY INDEX	37
	•

SAMPLE IDENTIFICATION:
BORING STH-7 DEPTH (ft): 40 to 47

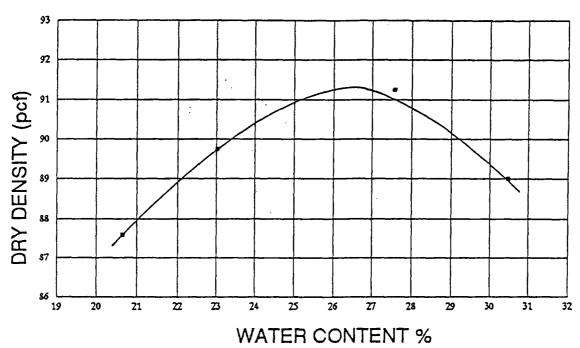
SOIL CLASSIFICATION: SANDY FAT CLAY (CH)

HYDRO-ENGINEERING

HUNTINGDON
93-4511A CHEN-NORTHERN, INC. GRADATION ANALYSIS Fig. 10

FIGURE C.2-16. GRADATION RESULTS FOR SAMPLE STH-7 (40'-47').





ASTM 698-78, METHOD A

OPTIMUM WATER CONTENT 26.7% MAXIMUM DRY DENSITY 91.3 pcf

PARTICLE SIZE	FRACTION OF SAMPLE (%)	
GRAVEL	0	
SAND	4	
FINES (SILT AND CLAY)	96	

ATTERBERG LIMITS

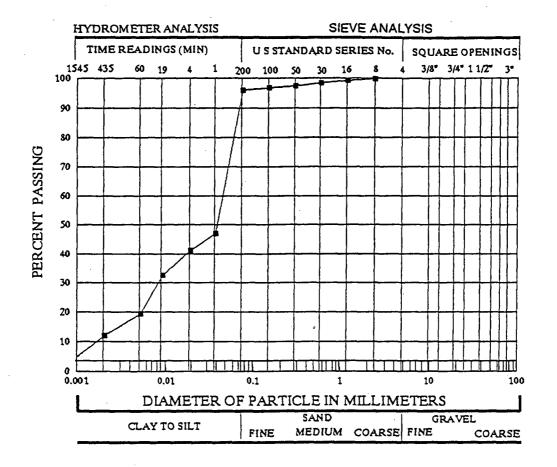
LIQUID LIMIT 64 PLASTICITY INDEX 37

SAMPLE IDENTIFICATION:

BORING STH-11 DEPTH (ft): 20 to 27

SOIL CLASSIFICATION: FAT CLAY (CH)

	HUNTINGDON		1
93-4511A	CHEN-NORTHERN, INC.	MOISTURE - DENSITY RELATIONSHIP	Fig. 11



PARTICLE	FRACTIONOF
SIZE	SAMPLE(%)
GRAVEL	0
SAND	4
FINES (SILT AND CLAY)	96
ATTERBERG LIMITS	
LIQUID LIMIT	. 64
PLASTICITY INDEX	37
SAMPLE IDENTIFICATION:	
SAMPLE IDENTIFICATION.	

SOIL CLASSIFICATION: FAT CLAY (CH)

BORING STH-11 DEPTH (ft): 20 to 27

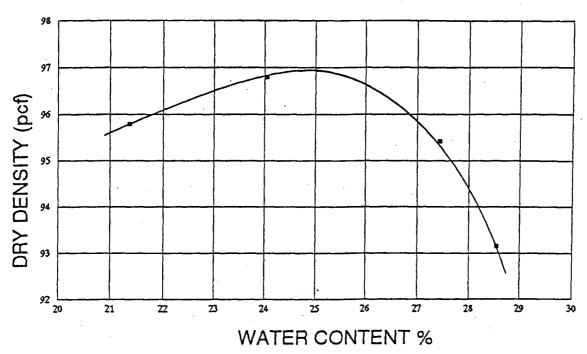
HYDRO-ENGINEERING

HUNTINGDON

93-4511A CHEN-NORTHERN, INC. GRADATION ANALYSIS Fig. 12

FIGURE C.2-18. GRADATION RESULTS FOR SAMPLE STH-11 (20'-27').





ASTM 698-78, METHOD A

OPTIMUM WATER CONTENT 24.8% MAXIMUM DRY DENSITY 96.8 pcf

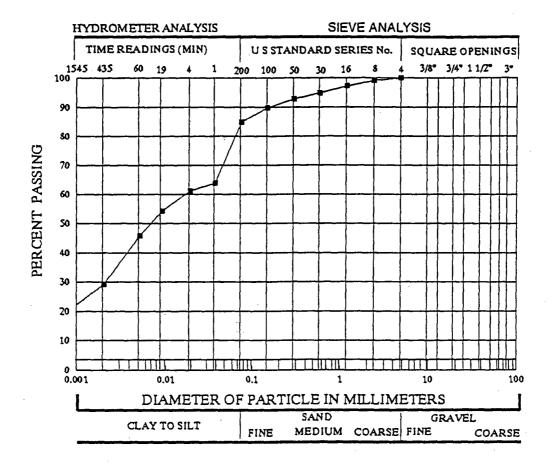
PARTICLE SIZE	FRACTION OF SAMPLE (%)	
GRAVEL	. 0	
SAND	15	
FINES (SILT AND CLAY)	85	
ATTERBERG LIF	MITS	
LIQUID LIMIT	65	

SAMPLE IDENTIFICATION: BORING NTH-1 DEPTH (ft): 60 to 67

PLASTICITY INDEX

SOIL CLASSIFICATION: FAT CLAY WITH SAND (CH)

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H	UNTINGDON		ĺ
1	HEN-NORTHERN, INC.	MOISTURE - DENSITY RELATIONSHIP	Fig. 13
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FRACTION OF SAMPLE (%)
0
15
CLAY) 85
DED CLIMITS
X 38
EIDENTIFICATION:
15 CLAY) 85 BERG LIMITS 65 EX 38

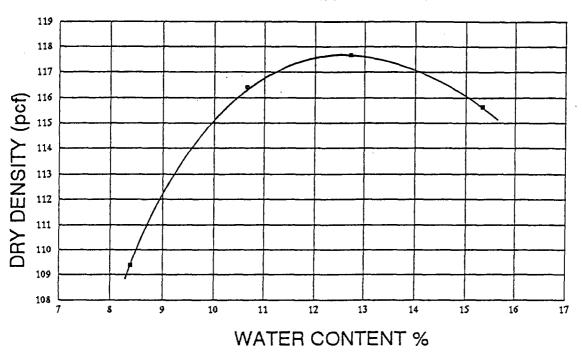
SOIL CLASSIFICATION: FAT CLAY WITH SAND (CH)

HYDRO-ENGINEERING

HUNTINGDON
93-4511A CHEN-NORTHERN, INC. GRADATION ANALYSIS Fig. 14

FIGURE C.2-20. GRADATION RESULTS FOR SAMPLE NTH-1 (60'-67').





ASTM 698-78, METHOD A

OPTIMUM WATER CONTENT 12.7% MAXIMUM DRY DENSITY 117.7 pcf

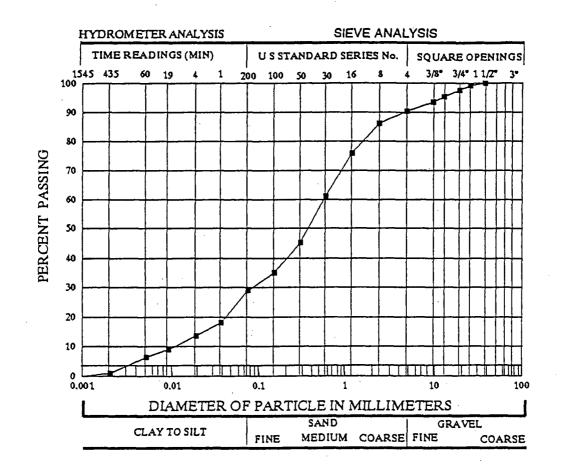
PARTICLE SIZE	FRACTION OF SAMPLE (%)
GRAVEL SAND FINES (SILT AND CLAY)	10 61 29
ATTERBERG LII	MITS
LIQUID LIMIT	42
PLASTICITY INDEX	27

SAMPLE IDENTIFICATION:

BORING NTH-3 DEPTH (ft): 140 to 147

SOIL CLASSIFICATION: SANDY LEAN CLAY (CL)

	HUNTINGDON		
93-4511A	CHEN-NORTHERN, INC.	MOISTURE-DENSITY RELATIONSHIP	Fig. 15



PARTICLE	FRACTIONOF
SIZE	SAMPLE(%)
GRAVEL.	10
SAND	. 61
FINES (SILT AND CLAY)	29
ATTERBERGL	
LIQUID LIMIT	42
PLASTICITY INDEX	27
SAMPLE IDENT	FICATION:
ORING NTH-3 DEF	PTH (ft): 140 to 147

SOIL CLASSIFICATION: SANDY LEAN CLAY (CL)

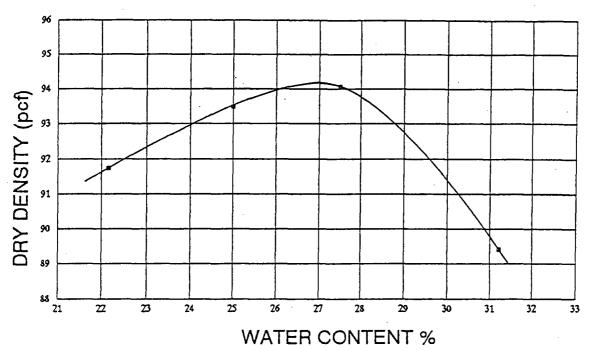
HYDRO-ENGINEERING

HCN 9-12

HUNTINGDON
93-4511A CHEN-NORTHERN, INC. GRADATION ANALYSIS Fig. 16

FIGURE C.2-22. GRADATION RESULTS FOR SAMPLE NTH-3 (140'-147').





ASTM 698-78, METHOD A

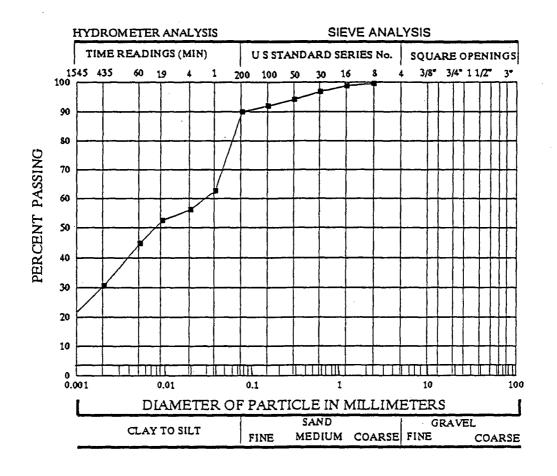
OPTIMUM WATER CONTENT 27.0% MAXIMUM DRY DENSITY 94.1 pcf

PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	0
SAND	10
FINES (SILT AND CLAY)	. 90
A77500500 LI	MITO
ATTERBERG LI	MII2
LIQUID LIMIT	65
PLASTICITY INDEX	42

SAMPLE IDENTIFICATION: BORING NTH-5 DEPTH (ft): 20 to 27

> SOIL CLASSIFICATION: FAT CLAY (CH)

HU	NTINGDON	·	
93-4511A CHE	N-NORTHERN, INC.	MOISTURE - DENSITY RELATIONSHIP	Fig. 17



PARTICLE	FRACTIONOF
SIZE	SAMPLE(%)
GRAVEL	· 0
SAND	10
FINES (SILT AND CLAY)	90
ATTERBERG L	IMITS
LIQUID LIMIT	65
PLASTICITY INDEX	42
SAMPLE IDENTI	FICATION:

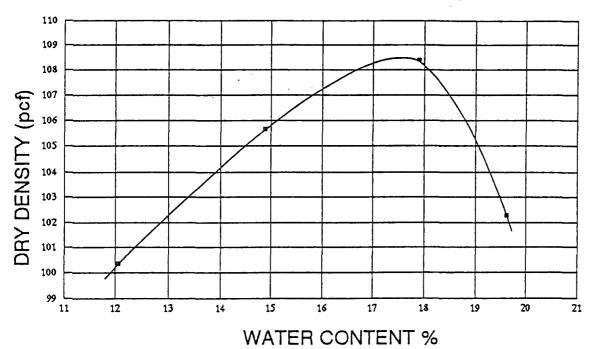
BORING NTH-5 DEPTH (ft): 20 to 27

SOIL CLASSIFICATION: FAT CLAY (CH)

ļ	HCN 9-12			
ł		HUNTINGDON		
I	93-4511A	CHEN-NORTHERN, INC.	GRADATION ANALYSIS	Fig. 18

FIGURE C.2-24. GRADATION REUSLTS FOR SAMPLE NTH-5 (20'-27').





ASTM 698-78, METHOD A

OPTIMUM WATER CONTENT 17.3% MAXIMUM DRY DENSITY 108.5 pcf

PARTICLE	FRACTION OF	
SIZE	SAMPLE (%)	
GRAVEL	7	
SAND	45	
FINES (SILT AND CLAY)	48	
ATTERBERG	LIMITS	
LIQUID LIMIT	55	
PLASTICITY INDEX	39	

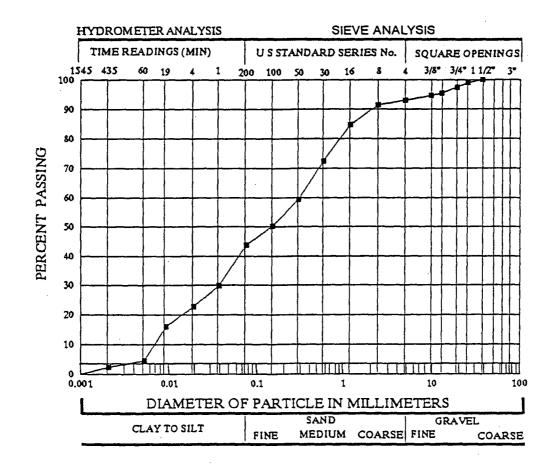
SAMPLE IDENTIFICATION:

BORING NTH-5 DEPTH (ft): 120 to 127

SOIL CLASSIFICATION: SANDY FAT CLAY (CH)

HYDRO-ENGINEERING

HUNTINGDON MOISTURE - DENSITY RELATIONSHIP CHEN-NORTHERN, INC. 93-4511A



PARTICLE	FRACTIONOF
SIZE	SAMPLE(%)
GRAVEL	7
SAND	45
FINES (SILT AND CLAY)	48
ATTERBERO	LIMITS
LIQUID LIMIT	55
PLASTICITY INDEX	39
SAMPLE IDEN	ITIFICATION:

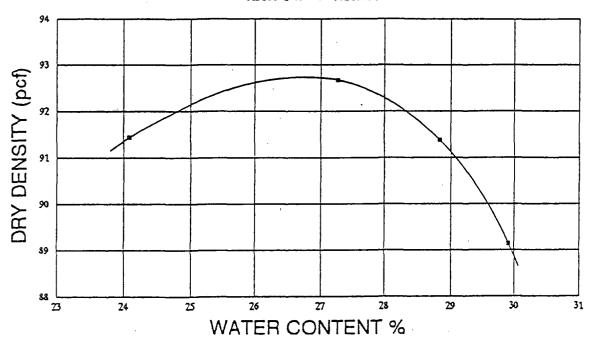
SAMPLE IDENTIFICATION:
BORING NTH-5 DEPTH (ft): 120 to 127

SOIL CLASSIFICATION: SANDY FAT CLAY (CH)

HCN 9-12				
		HUNTINGDON		
١	93-4511A	CHEN-NORTHERN, INC.	GRADATION ANALYSIS	Fig. 20

FIGURE C.2-26. GRADATION RESULTS FOR SAMPLE NTH-5 (120'-127').

ASTM D 698-78 METHOD A



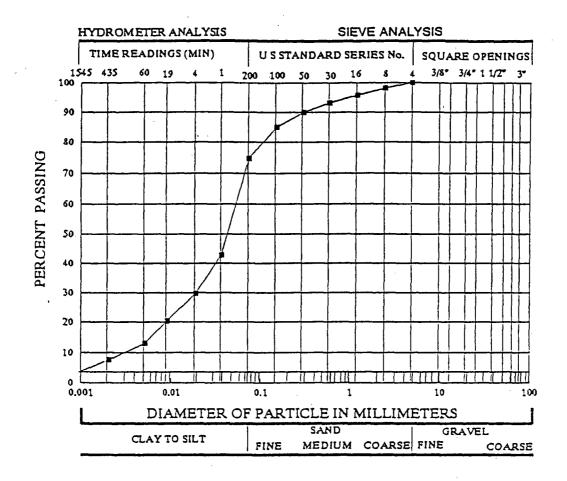
OPTIMUM WATER CONTENT 26.5% MAXIMUM DRY DENSITY 92.8 pcf

PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	0
SAND	25
FINES (SILT AND CLAY)	75
ATTERBERG	LIMITS
LIQUID LIMIT	56
PLASTICITY INDEX	29

SAMPLE CP-1 DEPTH (ft): 10

SOIL CLASSIFICATION: SANDY CLAY (CL)

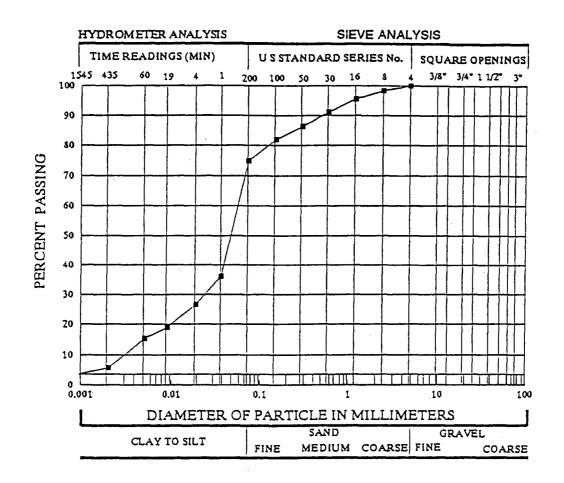
		والمرابع والمرابع والمرابع والمرابع والمرابع والمرابع والمرابع والمرابع والمرابع والمرابع والمرابع والمرابع والمرابع	
	HUNTINGDON		
93-4511	CHEN-NORTHERN. INC.	MOISTURE - DENSITY RELATIONSHIP	Fig. 1



PARTICLE	FRACTIO	N OF	
SIZE SAMPLE		(%)	
GRAVEL	,	0	
SAND		25	
FINES (SILT AND CLAY)		75	
ATTERBERG	LIMITS		
LIQUID LIMIT		56	
PLASTICITY INDEX		29	
SAMPLE CP-1	DEPTH (ft):	10	
SOIL CLASS SANDY C			
HYDRO-ENG	GINEERING		

	HUNTINGDON		1
93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS	Fig. 1A

FIGURE C.2-28. GRADATION RESULTS FOR SAMPLE CP-1 (10').



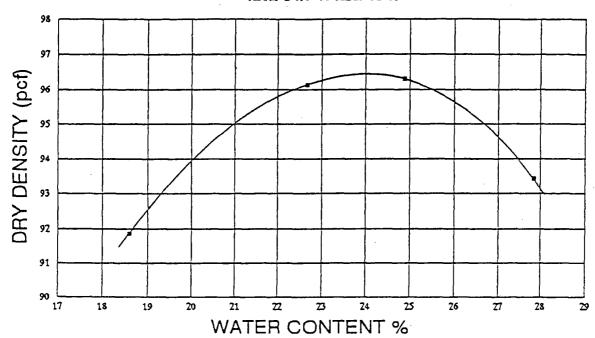
PARTICLE SIZE	FRACTION OF SAMPLE (%)	
	OFERT EE (70)	
GRAVEL	0	
SAND	25	
FINES (SILT AND CLAY)		
ATTERBERG LII	MITS	
LIQUID LIMIT	50	
PLASTICITY INDEX	26	
SAMPLE CP-2 D	EPTH (ft): 9	

SOIL CLASSIFICATION: SANDY CLAY (CL)

-					
		HUNTINGDON			
	93~4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS	Fig. 2A	

FIGURE C.2-29. STANDARD PROCTOR RESULTS FOR SAMPLE CP-2 (9').

ASTM D 698-78 METHOD A



OPTIMUM WATER CONTENT 24.3%

MAXIMUM DRY DENSITY 96.4 pcf

PARTICLE SIZE	FRACTION OF SAMPLE (%)
GRAVEL SAND FINES (SILT AND CLAY)	0 25 75
ATTERBERG	LIMITS 50
PLASTICITY INDEX	26

SAMPLE CP-2 DEPTH (ft): 9

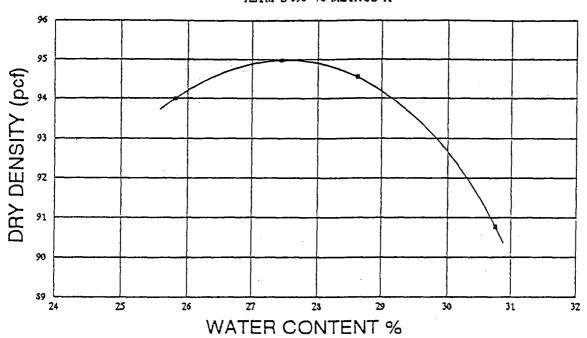
SOIL CLASSIFICATION: SANDY CLAY (CL)

HYDRO-ENGINEERING

	HUNTINGDON		•
00 4544		MOJETUDE DENCITY DELATIONICUM	ria a
93-4511	CHEN-NORTHERN, INC.	MOISTURE - DENSITY RELATIONSHIP	Fig. 2

FIGURE C.2-30. GRADATION RESULTS FOR SAMPLE CP-2 (9').

ASTM D 698-78 METHOD A



OPTIMUM WATER CONTENT 27.5%

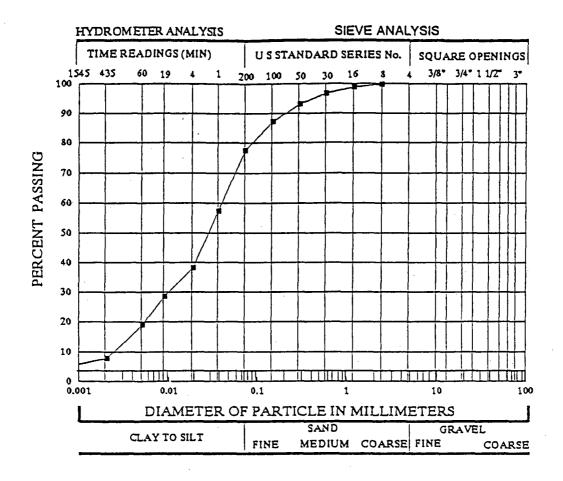
MAXIMUM DRY DENSITY 94.9 pcf

PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	0
SAND	23
FINES (SILT AND CLAY)	77
ATTERBERG	LIMITS
LIQUID LIMIT	52
PLASTICITY INDEX	27

SAMPLE CP-3 DEPTH (ft): 9

SOIL CLASSIFICATION: SANDY CLAY (CL)

	HUNTINGDON		
93-4511	CHEN-NORTHERN, INC.	MOISTURE - DENSITY RELATIONSHIP	Fig. 3



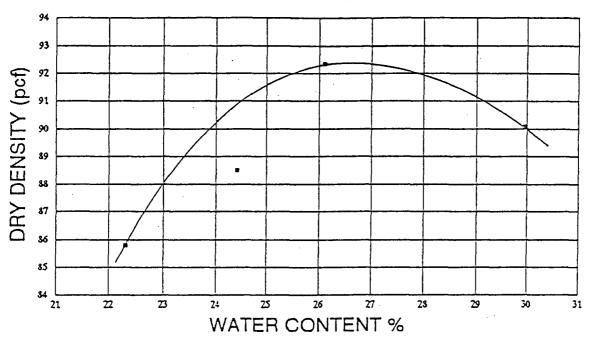
PARTICLE SIZE	FRACTION OF SAMPLE (%)
GRAVEL SAND FINES (SILT AND CLAY)	0 23 77
ATTERBERG LI	MITS
LIQUID LIMIT PLASTICITY INDEX	52 27
	EPTH (ft): 9

SOIL CLASSIFICATION: SANDY CLAY (CL)

ı				
		HUNTINGDON		
I	93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS	Fig. 3A

FIGURE C.2-32. GRADATION RESULTS FOR SAMPLE CP-3 (9').

ASTM D 698-78 METHOD A



OPTIMUM WATER CONTENT 26.3% MAXIMUM DRY DENSITY 92.4 pcf

PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
	,
GRAVEL	1
SAND	22
FINES (SILT AND CLAY)	77
ATTERBERG L	IMITS
LIQUID LIMIT	59
PLASTICITY INDEX	33

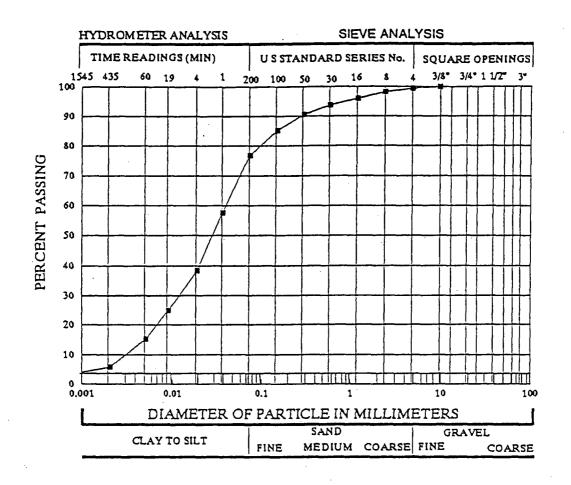
SAMPLE ND-1

SOIL CLASSIFICATION: LEAN CLAY WITH SAND (CL)

HYDRO-ENGINEERING

HUNTINGDON		l
93-4511 CHEN-NORTHERN. INC.	MOISTURE - DENSITY RELATIONSHIP	Fig. 4

FIGURE C.2-33. STANDARD PROCTOR RESULTS FOR SAMPLE ND-1.



PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	. 1
SAND FINES (SILT AND CLAY)	22 77
TINES (SILI AND CLAT)	
ATTERBERG I	
LIQUID LIMIT	. 59
PLASTICITY INDEX	33

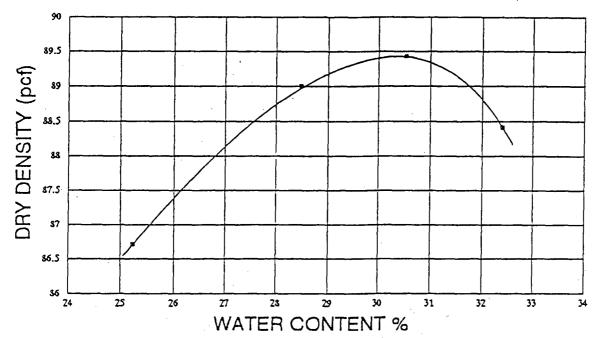
SAMPLE ND-1

SOIL CLASSIFICATION: LEAN CLAY WITH SAND (CL)

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Ì		HUNTINGDON	i e e e e e e e e e e e e e e e e e e e	i 1
Į		11011111100011) 1
1	93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS	Fig. 4A
1	30 - 4011	CHE.T-HORTHER.T. L.YC.	GIABATION ANALIGIS	112.71

FIGURE C.2-34. GRADATION RESULTS FOR SAMPLE ND-1.

ASTM D 698-78 METHOD A



OPTIMUM WATER CONTENT 30.3%

MAXIMUM DRY DENSITY 89.4 pcf

PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	. 0
SAND	27
FINES (SILT AND CLAY)	73
ATTERBERG LI	MITS

LIQUID LIMIT	64
PLASTICITY INDEX	36

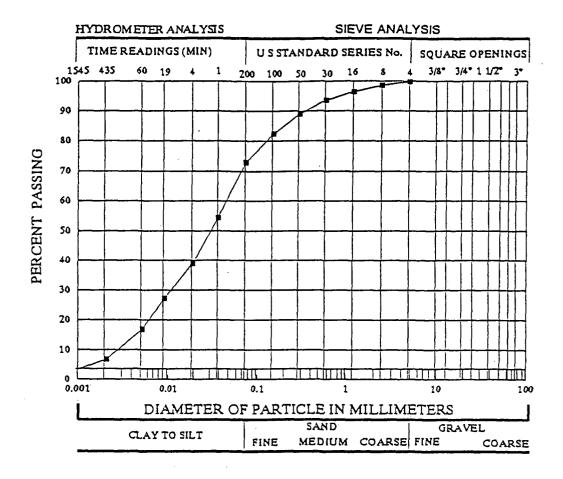
SAMPLE ND-2

SOIL CLASSIFICATION: LEAN CLAY WITH SAND (CL)

HYDRO-ENGINEERING

	LUINITINICOONI) ;
t .	HUNTINGDON	· · · · · · · · · · · · · · · · · · ·	
	· · · - ·	A CONTRACT OF A CONTRACT A CONTRACT OF A CON	
93-4511	CHEN-NORTHERN, INC.	MOISTURE – DENSITY RELATIONSHIP	Fig. 5
	CLIEL HOLLIER HILL	11101010101	

FIGURE C.2-35. STANDARD PROCTOR RESULTS FOR SAMPLE ND-2.



PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	0
SAND	27
FINES (SILT AND CLAY)	73
	73 /7000
ATTERBERG L	IWILZ
LIQUIDLIMIT	64
PLASTICITY INDEX	36

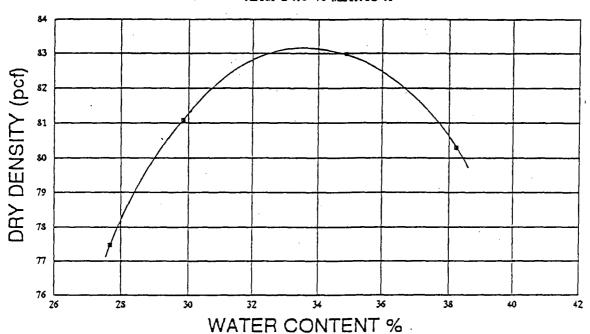
SAMPLE ND-2

SOIL CLASSIFICATION: LEAN CLAY WITH SAND (CL)

	HUNTINGDON		
93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS	Fig. 5A

FIGURE C.2-36. GRADATION RESULTS FOR SAMPLE ND-2.

ASTM D 698-78 METHOD A



OPTIMUM WATER CONTENT 33.5% MAXIMUM DRY DENSITY 83.2 pcf

PARTICLE	FRACTION OF	
SIZE	SAMPLE (%)	
	· .	
GRAVEL	1 _	
SAND	23	
FINES (SILT AND CLAY)	76	
ATTERBERG	PIMITS	
LIQUIDLIMIT	75	
PLASTICITY INDEX	40	

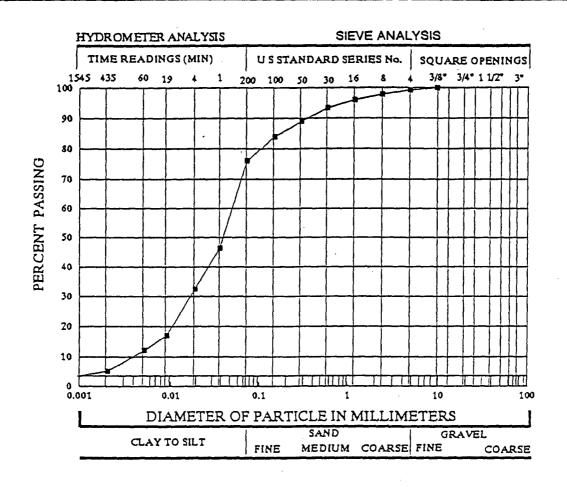
SAMPLE ND-3

SOIL CLASSIFICATION: LEAN CLAY WITH SAND (CL)

HYDRO-ENGINEERING

<u> </u>		
HUNTINGDON		j j
93-4511 CHEN-NORTHERN, INC.	MOISTURE - DENSITY RELATIONSHIP	Fig. 6

FIGURE C.2-37. STANDARD PROCTOR RESULTS FOR SAMPLE ND-3.



PARTICLE SIZE	FRACTION OF SAMPLE (%)
GRAVEL SAND FINES (SILT AND CLAY)	1 23 76
ATTERBERG L LIQUID LIMIT PLASTICITY INDEX	IMITS 75 40

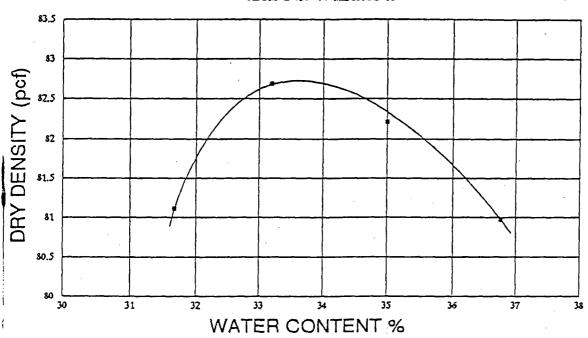
SAMPLE ND-3

SOIL CLASSIFICATION: LEAN CLAY WITH SAND (CL)

	HUNTINGDON		} }
93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS	Fig. 6A

FIGURE C.2-38. GRADATION RESULTS FOR SAMPLE ND-3.

ASTM D 698-78 METHOD A



OPTIMUM WATER CONTENT 33.6%

MAXIMUM DRY DENSITY 82.7 pcf

PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	0
SAND	25
FINES (SILT AND CLAY)	75
ATTERBERG LI	MITS
LIQUID LIMIT	69
DI ACTICITY INDEY	26

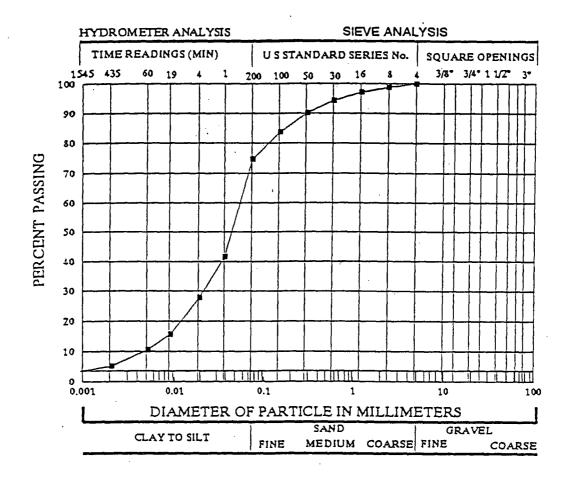
SAMPLE ND-4

SOIL CLASSIFICATION: LEAN CLAY WITH SAND (CL)

HYDRO-ENGINEERING

	HUNTINGDON		
93-4511	CHEN-NORTHERN, INC.	MOISTURE - DENSITY RELATIONSHIP	Fig. 7

FIGURE C.2-39. STANDARD PROCTOR RESULTS FOR SAMPLE ND-4.



PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	0
SAND	25
FINES (SILT AND CLAY)	75
ATTERBERG 1	LIMITS
LIQUID LIMIT	69
PLASTICITY INDEX	36

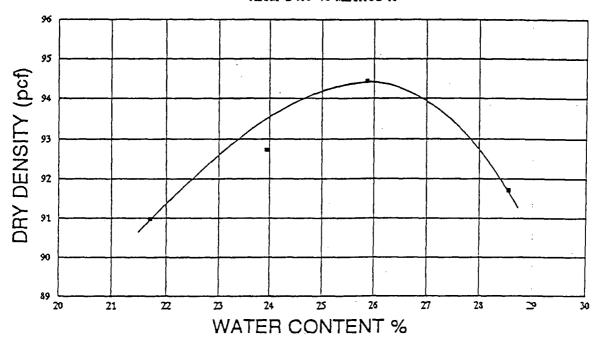
SAMPLE ND-4

SOIL CLASSIFICATION: LEAN CLAY WITH SAND (CL)

	HUNTINGDON		
93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS	Fig. 7A

FIGURE C.2-40. GRADATION RESULTS FOR SAMPLE ND-4.

ASTM D 698-78 METHOD A



OPTIMUM WATER CONTENT 25.8%

MAXIMUM DRY DENSITY 94.5 pcf

PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	0
SAND	14
FINES (SILT AND CLAY)	86
ATTERBERG L	IMITS
LIQUID LIMIT	61
PLASTICITY INDEX	33

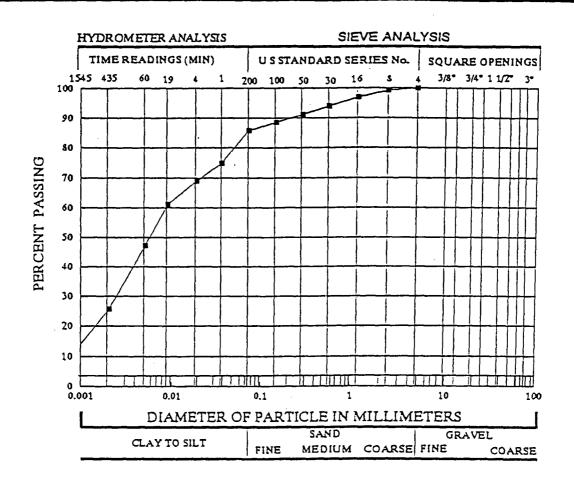
SAMPLE ND-5

SOIL CLASSIFICATION: LEAN CLAY (CL)

HYDRO-ENGINEERING

	HUNTINGDON		
93-4511	CHEN-NORTHERN, INC.	MOISTURE - DENSITY RELATIONSHIP	Fig. 8

FIGURE C.2-41. STANDARD PROCTOR RESULTS FOR SAMPLE ND-5.



PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
	•
GRAVEL	0
SAND	14
FINES (SILT AND CLAY)	86
ATTERBERG I	INITC
• • • • • • • • • • • • • • • • • • • •	
LIQUIDLIMIT	61
PLASTICITY INDEX	33

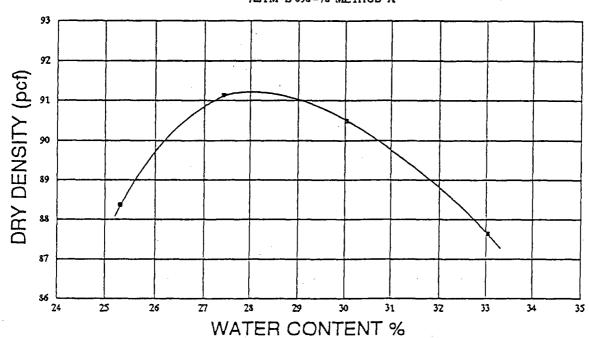
SAMPLE ND-5

SOIL CLASSIFICATION: LEAN CLAY (CL)

L				ز المساحب
١	j	HUNTINGDON		1 !
1	ı			i i
l	93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS	Fig. 8A

FIGURE C.2-42. GRADATION RESULTS FOR SAMPLE ND-5.

ASTM D 698-78 METHOD A



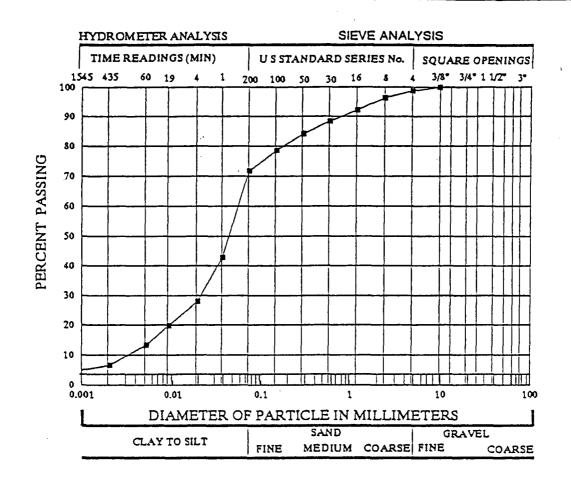
OPTIMUM WATER CONTENT 28.0% MAXIMUM DRY DENSITY 91.2 pcf

PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	1
SAND	27
FINES (SILT AND CLAY)	72
ATTERBERG I	LIMITS
LIQUID LIMIT	65
PLASTICITY INDEX	34

SAMPLE ND-6

SOIL CLASSIFICATION: LEAN CLAY WITH SAND (CL)

HUNTINGDON		
93-4511 CHEN-NORTHERN, INC.	MOISTURE - DENSITY RELATIONSHIP	Fig. 9



PARTICLE SIZE	FRACTION OF SAMPLE (%)
GRAVEL SAND FINES (SILT AND CLAY)	1 27 72
ATTERBERG L LIQUID LIMIT PLASTICITY INDEX	.IMITS 65 34

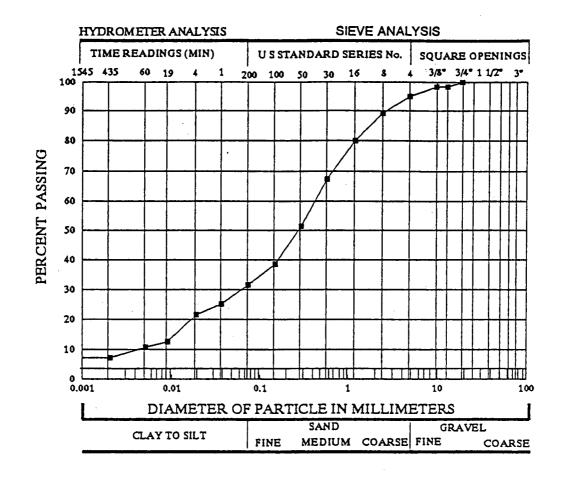
SAMPLE ND-6

SOIL CLASSIFICATION: LEAN CLAY WITH SAND (CL)

HYDRO-ENGINEERING

			
1	HUNTINGDON		1 1
	1.0111100011		1
93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS	Fig. 9A

FIGURE C.2-44. GRADATION RESULTS FOR SAMPLE ND-6.



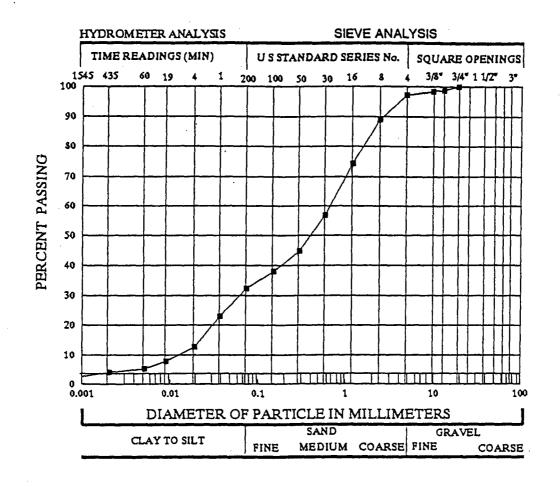
PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	5
SAND	63
FINES (SILT AND CLAY)	32
ATTERBERG I	
LIQUIDLIMIT	50
PLASTICITY INDEX	31

SAMPLE TS-1

SOIL CLASSIFICATION: SILTY SAND (SM)

	HUNTINGDON	
93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS

FIGURE C.3-1. GRADATION RESULTS FOR TOPSOIL SAMPLE TS-1.



PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	. 3
SAND	65
FINES (SILT AND CLAY)	32
ATTERBERG I	LIMITS
LIQUID LIMIT	37
PLASTICITY INDEX	21

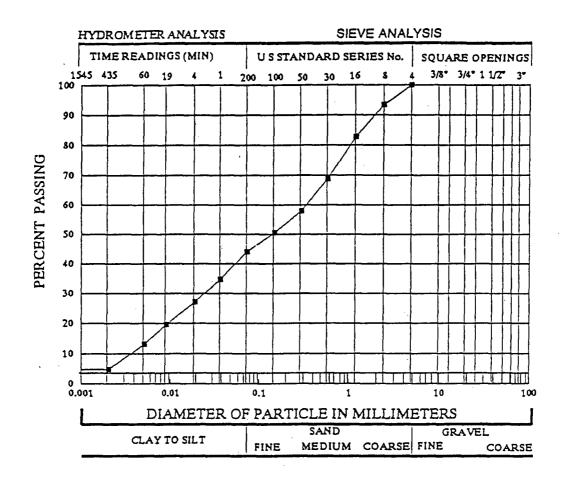
SOIL CLASSIFICATION: SILTY SAND (SM)

SAMPLE TS-2

HYDRO-ENGINEERING

	HUNTINGDON	
93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS

FIGURE C.3-2. GRADATION RESULTS FOR TOPSOIL SAMPLE TS-2.



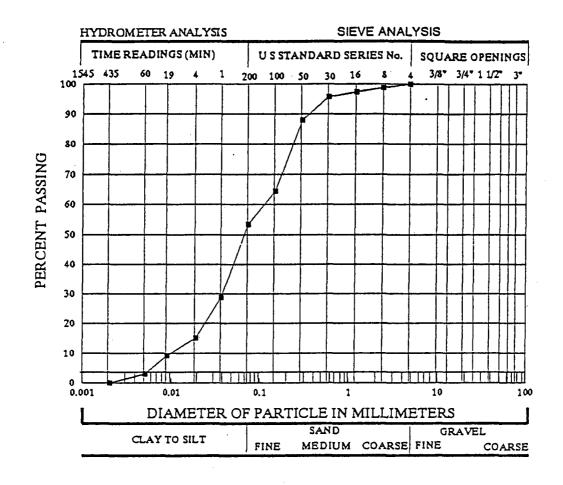
PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	0
SAND	56
FINES (SILT AND CLAY)	44
ATTERBERG L	IMITS
LIQUID LIMIT	48
— -	
PLASTICITY INDEX	25

SAMPLE TS-3

SOIL CLASSIFICATION: CLAYEY SAND (SC)

	HUNTINGDON		
93-4511	CHEN-NORTHERN, INC.	GRADATION ANALYSIS	Fig. 4

FIGURE C.3-3. GRADATION RESULTS FOR TOPSOIL SAMPLE TS-3.



PARTICLE	FRACTION OF
SIZE	SAMPLE (%)
GRAVEL	. 0
SAND	47
FINES (SILT AND CLAY)	53
	·
ATTERBERG I	LIMITS
LIQUID LIMIT	52
PLASTICITY INDEX	19

SOIL CLASSIFICATION: SANDY LEAN CLAY (CL)

SAMPLE TS-4

	HUNTINGDON		
93-4511	CHEN-NORTHERN. INC.	GRADATION ANALYSIS	Fig. 5

FIGURE C.3-4. GRADATION RESULTS FOR TOPSOIL SAMPLE TS-4.

TABLE C.4-1. SUMMARY OF ROCK DURABILITY TESTING.

LABORATORY TESTING SUMMARY HYDRO ENGINEERING

	GRANITE	GRANITE	GRANITE	SANDSTONE
TEST TYPE	ROCK A	ROCK B	ROCK C	ROCK D
Bulk Specific Gravity ASTM C 127-88	2.64	2.64	2.65	2.59
Apparent Specific Gravity ASTM C 127-88	2.65	2.65	2.66	2.76
Bulk SSD Specific Gravity ASTM C 127-88	2.64	2.64	2.65	2.65
Absorption, % ASTM C 127-88	0.12	0.14	0.08	2.47
L.A. Abrasion, 100 Revolutions ASTM C 535-87 Grading 1	2.2	2.8	2.8	4.2
Sulfate Soundness (5 days) ASTM C 88-83	0.04	0.11	0.08	0.18

THEODORE P. PASTER, Ph.D.

Consultant 11425 East Cimmarron Drive Englewood, Colorado 80111 (303) 771-8219

October 15, 1992

Attn:

Thomas G. Michel Hydro-Engineering 770 East Magnolia Casper, WY. 82604 FAX: (307) 266-6597

RE: Petrographic Analyses of Two Rock Samples to be used as Riprap Protection on a Uranium Tailings Impoundment.

CONCLUSIONS

TABLE 1 gives a summary of the petrographic ratings of the two samples according to the NRC document excerpts supplied to this investigator:

TABLE 1 Petrographic Rock Sample NRC Ratings

Rock Type	Bulk Composition	Secondary Minerals and Weathering
Granite	Good*, Group 1	Good**, No clays, no weathering rinds.
Sandstone	Not listed	Fair or Poor, clay must be tested further.

* = Table 6.1, NRC document supplied.
** = Table 6.4, NRC document supplied.

There is no question that the granite, sample no. SH-1, has a good rating.

The sandstone, sample no. SH-2, requires further testing if it is even considered for use. A list of the tests follows with a clay mineral identification at the top of the list because the NRC rules clearly state that a smectite-bearing rock is unacceptable:

List of Further Sandstone Tests

- 1) Identification of clay. (Which is 2.9 \pm 1.1% of the sample.)
- 2) Bulk specific gravity. (The sample contains 5.7 ± 1.5% air.)
- 3) Absorbtion. (The sample may absorb up to 2% by weight of water because of its nearly
- 6% porosity.)
- 4) Freeze-thaw weight loss. (Water saturation may be high due to the 6% porosity.

The quality of the sandstone is highly questionable and not recommended for further testing unless an alternate source is unavailable.

Respectfully submitted:

MINERAL PERCENTAGES IN PROPOSED RIPRAP SAMPLES

TABLE 2 Mineral Percentage of Granite Sample HE-1 (By 2mm spaced point counting.)

Mineral		•
Phase	counts	o,
Quartz	142	30.5 ± 4.2
Microcline	137	29.4 ± 4.2
Plagioclase	160	34.3 ± 4.4
Biotite	26	5.6 ± 1.5
Zircon	0	<0.2
Magnetite	1	0.2
Apatite	0	<0.2
Totals	466	100.0

TABLE 3
Mineral Percentages of Sandstone
Sample HE-2
(By 0.5 mm spaced point counting.)

Mineral		
Phase	counts	<u> </u>
Quartz	596	68.6 ± 3.1
Hematite	187	21.5 ± 2.7
Voids	50	5.7 ± 1.5
Clay	25	2.9 ± 1.1
Chert	10	1.1 ± 0.6
Other	2	0.2
Totals	870	100.0

PETROGRAPHIC DESCRIPTIONS

HE-1; Nearly Fresh, Medium-grained Granite with Trace Microfractures.

Granite (99+%):		
30.5% Quartz (Q)	1.4-8mm	Strained, amoeboid anhedra with sutured boundaries. Smaller, minor, 0.12mm, rounded blebs encapsulated or included in feldspars.
29.4% Microcline	0.6-4mm	Fresh anhedra have sutured boundaries with surrounding minerals which are predominately Q and finer-grained biotite, feldspars and Q. Contain small euhedral inclusions of sodic plagioclase.
34.3% Plagioclase (Pl, An ₂₅)	1-5mm	Oligoclase. Tabular subhedra with irregular edges are larger and interstitial to other minerals. Minor, smaller, 0.04-0.5mm, sub-rounded crystals are included in microcline. Slightly composition-zoned as evidenced by selective concentric alteration. Alteration is about 15% disseminated fine-grained sericite > epidote + Q ± chlorite.
5.6% Biotite (Bt)	0.2-2.5mm long	Ragged brown subhedral books usually in clumps interstitial to other silicates. Altered to green variety in some areas of thin section.
0.2% Magnetite (Mt)	0.04-0.6mm	Subhedra associated with Bt and zircon interstitial to larger silicates. Rarely partly replaced by sphene.
tr Zircon(?)	O.3-1mm	Brown, growth-zoned euhedra associated with Bt and Mt along boundaries of larger silicate crystals.
tr Apatite	0.08mm	Stubby prisms in Pl.
tr Hematite	<0.04mm	Red flakes and stain concentrated in altered Pl and along feldspar grain boundaries. Gives pink color to rock.

Microfractures (tr):

Discontinuous, sub-parallel fractures spaced about 1.5 mm and about 1 mm in length. Thickness <0.01mm and appears to contain sericite.

The alteration mentioned in this sample is hydrothermal and not due to weathering. It is not deleterious with regards to the intended use of the rock.

No measurable weathering rind was found on this sample.

TABLE C.4-2. PETROGRAPHIC ANALYSIS (continued).

HE-2; Quartz Sandstone with Hematite Cement.

Clastic Grains (6	4%):	
68.6% Quartz	0.12-0.5mm	Rounded to sub-rounded well-sorted grains.
1.1% Chert + Chalcedony	0.12-0.4mm	Rounded to sub-rounded grains.
Matrix (30.3%):		
21.5% Hematite	-	Deep red-brown to red-orange 5u coating on all clastic grains and occasionally fills 0.03mm interstitial areas. may be mixed with minor clay. Cements Q grains together.
5.7% Voids	0.01-0.4mm	Unfilled areas between clastic grains.
2.9% Clay	-	Fills some interstitial areas up to 0.2mm in some areas of section - may reflect bedding.
0.2% Other	0.08mm	Dark green cement grades to hematite which may be chamosite.

This rock has no recognizable weathering rind. It is thin if present. The clay in this rock did not come from weathering of the rock itself. It appears to be derived from outside the rock which suggests some degree of permeability in the rock.

The clay type may be smectite though even if it is, the rock appears to be too porous for the clay to cause any expansive damage. For the same reason (porosity) freeze-thaw damage in the rock is doubtful.

The void content of the rock is 6% by volume which, if filled with water, could cause a weight increase of 2%.

TABLE C.5-1. CRUMB TEST RESULTS SAMPLE STR-5 (40'-47').

Initial Moisture Condition - Allowed to air dry for 48 hours.

Date	Time	Blapsed Time	ДН	Conductivity unho/cm	Temp.	Grading & Comments
9/30/93	14:45	Ø min.	7.15	6	25°	Sample inserted
	14:47	2 min.	7.70		19.8*	Grade 2 Slight reaction
	15:00	15 min.	7.65		19.5*	Grade 2 Slight reaction
	15:45	60 min.	7.6		19*	Grade 2 Slight reaction
10/1/93	14:45	24 hrs.	7.35	230	19.5°	Grade 1 No reaction

TABLE C.5-2. CROWN TEST RESULTS SAMPLE STH-4 (30'-37').

Initial Moisture Condition - Allowed to air dry for 48 hours.

<u>Date</u>	fine	Elapsed Time	Hq	Conductivityumho/cm	Temp.	Grading & Comments
9/30/93	14:15	9 min.	7.15	6	25°	Sample inserted
	14:17	2 min.	7.75		26°	Grade 2 Slight reaction
	14:39	15 min.	8.9	•	19.8°	Grade 2 Slight reaction
	15:15	60 min.	7.71		19°	Grade 2 Slight reaction
10/1/93	14:15	24 hrs.	7.35	225	19°	Grade 1 No reaction

TABLE C.5-3. CROWN TEST RESULTS SAMPLE STH-2 (60'-67').

Initial Moisture Condition - Allowed to air dry for 48 hours.

<u>Date</u>	_Time_	Blapsed Time	pH	Conductivityumho/cm	Temp.	Grading & Comments
9/30/93	14:00	Ø min.	7.15	6	25°	Sample inserted
	14:02	2 min.	7.75		24*	Grade 1 No reaction
	14:15	15 min.	7.55		26°	Grade 1 No reaction
	15: <i>00</i>	60 min.	7.5	·	19.5*	Grade 1 No reaction
10/1/93	14:00	24 hrs.	7.35	290	19.5°	Grade 1 No reaction

TABLE C.5-4. CRUMB TEST RESULTS SAMPLE STH-6 (20'-27').

Initial Moisture Condition - Allowed to air dry for 48 hours.

Date	Tine	Rlapsed Time	<u>pH</u>	Conductivityumho/cm	Temp.	Grading & Comments
9/30/93	14:30	Ø min.	7.15	6	25°	Sample inserted
	14:32	2 min.	7.70		19.8*	Grade 3 Moderate reaction
	14:45	15 min.	7.82		19.8°	Grade 3 Moderate reaction
	15:30	60 min.	1.1		19°	Grade 2 Slight reaction
10/1/93	14:39	24 hrs.	7.4	185	19°	Grade 2 Slight reaction

APPENDIX D

LABORATORY REPORTS -RADIOLOGICAL AND RADON EMANATION PROPERTIES

APPENDIX D

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D.1 APPENDIX D TEXT

This appendix contains copies of laboratory analysis results provided by the analyzing laboratory. Emanation coefficient results are also included. Emanation coefficients were analyzed by Energy Laboratories Inc. of Casper, Wyoming under the direction of Roger Garling. The procedure used is that described in Nielson and others (1982), and was summarized by lab personnel as follows:

Samples were initially placed in a vacuum for removal of Rn-222. Initial counts were made after allowing unsupported Rn-222 daughters to decay out of the sealed sample. Final counts were preformed after allowing sufficient time for the Ra-226, Rn-222 and daughters to reach secular equilibrium.



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SOIL ANALYSIS REPORT - PATHFINDER MINES CORP

Project: Shirley Basin

SOIL ANALYSIS REPORT - PATHFINDER MINES CORPORATION

Project: Shirley Basin
Report Date: 04/01/93

LAB I.D.	SAMPLE/ I.D.	DEPTH, FT.	U-Nat pCi/g	Ra226 (Chemical) pCi/g	Net CPM 1 A°	Net CPM 2 A∞	(A∞ - A°)/A∞ 3
93-7679	TW4-2C	40'-42'	0.6	1557 ± 3.0	3299	3312	0.0041
93-7680	TW4-1B	25'-27'	0.6	728 ± 2.1	2827	2874	0.0166
93-7681	TW4-5B	5-1/2'-8'	0.6	172 ± 1.0	383	475	0.1934
93-7682	TW5-2B	31	0.6	20 ± 0.3	89	96	0.0699
93-7683	TW5-1B	4'-6'	0.6	47 ± 0.5	192	214	0.1005
93-7684	TW4-1B	3'-5'	0.7	63.3 ± 0.6	474	553	0.1423
93-7685	TW4-4B	5'-7'	0.7	58 ± 0.6	419	444	0.0552
93-7686	TW4-1C	3'-5'	0.6	53.6 ± 0.6	449	513	0.1246

¹ A° - Net CPM after de-emanation - (Initial Activity) ² A $_{\infty}$ - Net CPM after full ingrowth - (Final Activity)

Report Approved By: A.O. Lindung

^{3 (}A∞ - A°)/A∞ Radon Emanation Coefficient



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SOIL ANALYSIS REPORT - PATHFINDER MINES CORP

Project: Shirley Basin

SOIL ANALYSIS REPORT - PATHFINDER MINES CORPORATION

Project: Shirley Basin Report Date: 06/10/93

LAB I.D.	SAMPLE/ DEPTH, FT. I.D.	U-Nat pCi/g	Ra226 (Chemical) pCi/g	Net CPM 1 A°	Net CPM 2 A∞	(Am - A°)/Am 3
93-7679	TW4-2C 40'-42' Recheck - 06-10-93	0.6	1557 ± 3.0	3299	3312	0.0041
	20% Moisture - R.E.C	•		2664	2918	0.0870
93-7680		0.6	728 ± 2.1	2827	2874	0.0166
Recheck - 06-10-93 20% Moisture - R.E.C			1924	2267	0.1513	
	TW4-5B 5-1/2'-8' Recheck - 06-10-93	0.6	172 ± 1.0	383	475	0.1934
	6% Moisture - R.E.C.	,		330	367	0.0990
93-7682	TW5-2B 3' Recheck - 06-10-93	0.6	20 ± 0.3	89	96	0.0699
	6% Moisture - R.E.C.	٠		64.9	64.9	0.0000
93-7683 93-7684 93-7685 93-7686	TW5-1B 4'-6' TW4-1B 3'-5' TW4-4B 5'-7' TW4-1C 3'-5'	0.7 0.7	47 ± 0.5 63.3 ± 0.6 58 ± 0.6 53.6 ± 0.6	474	214 553 444 513	0.1005 0.1423 0.0552 0.1246

¹ A° - Net CPM after de-emanation - (Initial Activity) ² A_{∞} - Net CPM after full ingrowth - (Final Activity)

R.E.C. = Radon Emanation Coefficient

Report Approved By: R.a. Leading kmk

^{3 (}Aw - A°)/Aw Radon Emanation Coefficient



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SOIL ANALYSIS REPORT - PATHFINDER MINES CORPORATION

Project: Shirley Basin

Report Date: 04/19/93

LAB I.D.	SAMPLE/ DEPTH, I.D.	FT. Ra226 (Chemical) pCi/g	Net CPM 1 A°	Net CPM 2	(A∞ - A°)/A∞ 3
93-8403	TW5-3 6'- 8'	54.7 ± 0.6	165	169	0.0254
93-8404	TW5-1B 6'- 8'	51.8 ± 0.6	136	152	0.1084

Report Approved By: R.a. harlung kmk

¹ A° - Net CPM after de-emanation - (Initial Activity) 2 $A \infty$ - Net CPM after full ingrowth - (Final Activity) 3 $(A \infty - A^{\circ})/A \infty$ Radon Emanation Coefficient



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SOIL ANALYSIS REPORT - PATHFINDER MINES - SHIRLEY BASIN

Report Date: 08/17/93

LAB I.D.	SAMPLE I.D.	Ra226 (Chemical)	Net CPM ¹ A°	Net CPM ² A∞	$(A - A^{\circ})/A^{3}$	% Moisture
93-24405	BP-1	214 ± 1.6				
93-24406	BP-2	34.6 ± 0.7				
93-24407	BP-3	66.0 ± 1.2				
93-24408	BP-4	102 ± 1.1	252.2	257.9	0.0221	24.14
93-24409	BP-5	55.0 ± 0.8				
93-24410	BP-6	438 ± 2.3				
93-24411	BP-7	206 ± 1.6				
93-24412	BP-8	481 ± 2.5				
93-24413	BP-9	394 ± 2.2	867.5	1039.9	0.1658	19.68
93-24414	BP-10	414 ± 2.3				
93-24415	BP-11	737 ± 3.0	1683.5	1689.2	0.0034	19.15
93-24416	BP-12	565 ± 2.6				

Report Approved By: kmk 8324405.pmc

 $^{^{1}}$ A° - Net CPM after de-emanation 2 A $_{\odot}$ - Net CPM after full ingrowth 3 (A $_{\odot}$ - A°)/A $_{\odot}$ Radon Emanation Coefficient @ 20% Moisture Content

TABLE D-5. SOIL ANALYSIS FOR PIT SAMPLES P4-12B (18"-24"), P4-12B (78"-84"), P4-12C (24"-30"), P4-10A (0"-6"), P4-10A (78"-84"), P4-6A (60"), P4-8A (24"-30"), P4-5D (42"-48"), P4-12B (36"-42"), P4-12C (6"-12"), P4-12C (48"-54"), P4-12A (36"-42"), P4-8B (48"), P4-8A (0"-6"), P4-5D (36"-42"), P4-4D (36"-42"), P4-12B (48"-54"), P4-12B (72"-78"), P4-12B (72"-78"), P4-12B (48"-54").



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PATHFINDER MINES CORPORATION - SHIRLEY BASIN SOIL ANALYSIS REPORT

Date Submitted: 11-24-92 Date Received: 11-24-92 Report Date: 01-19-92

Analyst: DB

LAB I.D.	Sample I.D.	Sample Corr. Depth	U-Nat pCi/g	Ra226 pCi/g	Ra226 Frec.+/-		A∞ Final Activity	Radon Emanation (A - A°)/A
92-46510	P4-12B	18-24"		1.7	0.1	neer reg		(AG - A)/AG
92-46511	P4-12B	78-84"		0.3	0.1			
92-46512	P4-12C	24-30"		30.9	0.6			
92-46513	P4-10A	0-6"		29.9	0.6			
92-46514	P4-10A	78-84"		1.9	0.1			
92-46515	P4-6A	60"		1.0	0.1			
92-46516	P4-8A	24-30"		40.0	0.7			•
92-46517	P4-5D	42-48"		4.7	0.1			
92-46518	P4-12B	36-42"		9.6	0.4			
92-46519	P4-12C	6-12"		4.2	0.1			
92-46520	P4-12C	48-54"		23.8	0.5			_
92-46521	P4-12A	36-42"	•	93.5	1.0			
92-46522	P4-8B	48"		105	1.1			
92-46523	P4-8A	0-6"		65.8	0.9			
92-46524	P4-5D	36-42"		1157	3.4			
92-46525	P4-4D	36-42"		68.7	0.9			
92-46526	P4-12B	48~54"	20.4	2.1	0.1			
92-46527	P4-12B	72~78"	2.3	4.2	0.2			
92-46528	0-1	90-96" 7.40	43.5	15.0	0.4	38.2	45.1	0.153
92-46529	0-2	108-114" 2.97	158	194	1.4	144	173	0.166
92-46530	0-2	66~72"		32.2	0.6	115	130	0.113
92-46531	P4-12C	72~78"	11.9	1.8	0.1			
92-46532	P3-1	0-6"	1.2	14.9	0.4			
92-46533	P3-1	48-54"	1.6	1.9	0.1			

REPORT APPROVED BY: A.Q. Leaching kmk



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SOIL ANALYSIS - PATHFINDER MINES - SHIRLEY BASIN

Sample I.D.:	P4 8A	P4 8A	P4 8A
-	48-54	60-66	78-84
Sample Date:	12-92	12-92	12-92
Report Date:	01-10-93	01-10-93	01-10-93
Sample Number:	93-47270	92-47271	92-47272

RADIOMETRIC pCi/g:

Ra226		1.0	0.4	0.3
Ra Prec.	+/-	0.2	0.1	0.1

Report Approved By: A.Q. harling kmk



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SOIL ANALYSIS REPORT - PATHFINDER MINES - SHIRLEY BASIN MINE

Date Submitted: 06-18-93 Date Received: 06-18-93 Report Date: 07-12-93

LAB I.D.	Sample Date	Sample Identification	Ra226 Gamma pCi/g	Ra226 Prec.+/-
93-23065		STH-6 20-27'	2,05	0.13
93-23066		NTH-1 60-67'	2.99	0.15
93-23067		STH-5 40-47'	2.26	0.14
93-23068		G5 - 1	1.06	0.10
93-23069		G5 - 2	12.2	0.31
93-23070		G5 - 3	5.20	0.21
93-23071		G5 - 4	7.63	0.33
93-23072		G5 - 5	140	1.09
93-23073		G5 - 6	68.8	0.77
93-23074		G5 - 7	11.0	0.38
93-23075		G5 - 8	9.66	0.40
93-23076		G5 - 9	20.2	0.57
93-23077		G5 - 10	19.8	0.56
93-23078		G5 - 11	3.61	0.24
93-23079		G5 - 12	2.18	0.19
93-23080		G5 - 13	52.5	1.59
93-23081		G5 - 14	140	1.49

** G5 prefix corresponds to GS prefix throughout text.

Report Approved by: A. a. Leading

kmk =323065.pmc



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SOIL ANALYSIS REPORT - HYDRO ENGINEERING

Project: PMC - SB

Sample I.D.:	BGS-1	BGS-2	BGS-3	BGS-4	
Sample Date:	NSD	NSD	NSD	NSD	Det.Limit
Report Date:	07-19-93	07-19-93	07-19-93	07-19-93	
Sample Number:	93-23760	93-23761	93-23762	93-23763	
RADIOMETRIC pCi/g:					
Ra226	0.32	0.35	0.43	0.48	0.2
Ra Prec. +/-	0.04	0.04	0.04	0.04	

REPORT APPROVED BY: A.Q. Lacking

APPENDIX E

HYDROGRAPHS

APPENDIX E

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```
ID PATHFINDER, TAILINGS ROUTED TO 2/8 RESERVOIR THROUGH CONTROL
 ID BASED UPON PMF- RAINFALL OF 9.03 IN. COMPUTATION OF 1.0 MINUTE INCREMENTS
 ID DATE=4-18-96
 *FREE
 *DIAGRAM
IT 1.0,,,300,,
 IO 5,1
 IN 1,,
 PG HMR55
PC .0000,.0321,.0658,.1011,.1381,.1766,.2167,.2585,.3041,.3499,
PC .3959, .4423, .4900, .5400, .5939, .6535, .7211, .7994, .8913, 1.0003,
PC 1.1301,1.2848,1.4691,1.6878,1.9462,2.2499,2.6051,3.0181,3.4957,4.2583,
PC 5.1498, 5.7341, 6.1786, 6.5618, 6.8905, 7.1708, 7.4087, 7.6095, 7.7784, 7.9202,
PC 8.0390, 8.1390, 8.2237, 8.2963, 8.3597, 8.4162, 8.4679, 8.5166, 8.5635, 8.6096,
PC 8.6555,8.7012,8.7467,8.7876,8.8269,8.8647,8.9008,8.9353,8.9682,8.9995,
PC 9.0300
KK DM-2
KO 0,,,,21,1,100
BA .043
PR HMR55
LS 0,91
UD .1805
KK DM-3
KO 0,,,,21,1,100
BA .029
PR HMR55
LS 0,91
UD .1243
KK DM-4
KO 0,,,,21,1,100
BA .034
PR HMR55
LS 0,91
UD .1019
KK DM-6
KO 0,,,,21,1,100
BA .0078
PR HMR55
LS 0,91
UD .0788
KK DM-6A
KO 0,,,,21,1,100
BA .010
PR HMR55
LS 0,91
UD .0731
KK INDUST POND
KO 0, , , , 2\overline{1}, 1, 300
HC 5
KK IND_POND_CONTROL
KO 0, , , , 21, \overline{1}, 300
RS 1,STOR,0,0
SA 3.03 3.51 3.95 4.42 4.92 5.45 6.23 7.09 7.90
SQ 0 15.5 57 126 273 438 710 1002 1358
SE 0 1 2 3 4 5 6 7 8
KK DM-5
```

TABLE E-1. HEC-1 INPUT AND FLOW PATH FOR TAILINGS DISCHARGING TO AREA 2/8 AND SOUTH DRAINAGES (continued).

```
KO 0,,,,21,1,100
BA .035
 PR HMR55
LS 0,91
UD .0777
KK INDUST POND AREA
KO 0, , , , 2\overline{1}, 1, 3\overline{0}0
HC 2
KK BELOW IND POND
KO 0, , , , \overline{2}1, 1, \overline{3}00
RS 1,STOR,0,0
SA 0 0.77 1.31 2.88 3.86 5.10 6.52
SQ 0 21.7 68 143 280 478 763
SE 0 1.2 2.2 3.2 4.2 5.2 6.2
KK DM-1
KO 0,,,,21,1,100
BA .017
PR HMR55
LS 0,91
UD .0376
KK SOUTH_DRAIN
KO 0,,,,21,1,300
HC 2
KK DM-8
KO 0,,,,21,1,100
BA .007
PR HMR55
LS 0,91
UD .1295
KK DM-7
KO 0,,,,21,1,100
BA .012
PR HMR55
LS 0,91
UD .1425
KK DM-7+DM-8
KO 0,,,,21,1,300
HC 2
KK D4-5
KO 0,,,,21,1,100
BA .038
PR HMR55
LS 0,87
UD .1280
KK DM-7_D4-5
KO 0,,,,21,1,300
HC 2
KK ROUTE D4-5
KO 0, \dots, \overline{2}1, 1, 300
RS 1,STOR,0,0
SA 0 0.80 1.5
SQ 0 50 200
SE 0 0.5 1.0
KK D4-3
KO 0,,,,21,1,100
```

BA .045

TABLE E-1. HEC-1 INPUT AND FLOW PATH FOR TAILINGS DISCHARGING TO AREA 2/8 AND SOUTH DRAINAGES (continued).

```
PR HMR55
LS 0,86
UD .1954
KK INT COLL
KO 0,,,,21,1,300
HC 2
KK D5-2A
KO 0,,,,21,1,100
BA .026
PR HMR55
LS 0,91
UD .098
KK BASIN D5-2 CONTROL
KO 0,,,,21,1,180
RS 1,STOR,0,0
SA 0 0.82 3.20 3.62 3.78 3.95
SQ 0 2 4 9.9 25 114
SE 0 1 2 3 4 5
KK D5-2B
KO 0,,,,21,1,150
BA .008
PR HMR55
LS 0,87
UD .057
KK TOTAL D5-2
KO 0,,,,21,1,150
HC 2
KK D4-1
KO 0,,,,21,1,150
BA .039
PR HMR55
LS 0,91
UD .2774
KK D4-2
KO 0,,,,21,1,150
BA .0210
PR HMR55
LS . 0,85
UD .1021
KK D4-1&2
KO 0,,,,21,1,150
HC 2
KK D4-4
KO 0,,,,21,1,150
BA .044
PR HMR55
LS 0,91
UD .0497
KK D4-6
KO 0,,,,21,1,150
BA .051
PR HMR55
LS .0, 91
UD .0953
KK D3-1
```

KO 0,,,,21,1,150

TABLE E-1. HEC-1 INPUT AND FLOW PATH FOR TAILINGS DISCHARGING TO AREA 2/8 AND SOUTH DRAINAGES (continued).

```
BA .051
PR HMR55
LS 0,89
UD .1121
KK WEST_TAILS
KO 0,,,,21,1,300
HC 6
KK AREA-28-CONTROL
KO 0,,,21,1,300
RS 1,STOR,0,0
SA 1 4.09 7.26 10.06 13.25 16.83 19.41 22.40 25.23
SQ 0 2.25 14.1 41.7 90 163 265 400 570
SE 0 1 2 3 4 5 6 7 8
ZZ
```

TABLE E-1. HEC-1 INPUT AND FLOW PATH FOR TAILINGS DISCHARGING TO AREA 2/8 AND SOUTH DRAINAGES.

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT	(V)	ROUTING	. (>) .	DIVERSION	OR PUMP FLO	W
LINE NO.	(.)	CONNECTOR	(<)	RETURN OF	DIVERTED OR	PUMPED FLOW
15	DM-2					
21	:	DM-3				
	•	•				
27	:	•	DM-4			
33		•	•	DM-6		
	•	•	•			
39	•	•	•	•	DM-6A	
45	י י י שפונולוגד	OND	·		•	
43	_v	MD	• • • • • • • • • • • • • •		• • • • • • • • • • •	
48	V IND_POND_	CONTROL				
54	•	D) (5				
34	•	DM-5				
60		OND_AREA				
62	V V					
63	BELOW_IND	_POND				
69	•	DM-1				
		:				
75	SOUTH_DRA	LIN				
78	•	DM-8				
0.4	•	•	6			
84	•	•	DM-7			
90	•	DM-7+DM-8.	•			
	•	•				
93	•	•	D4-5			

TABLE E-1. HEC-1 INPUT AND FLOW PATH FOR TAILINGS DISCHARGING TO AREA 2/8 AND SOUTH DRAINAGES.

V V V V V V V V V V V V V V V V V V V	
102	
108	
114 : INT_COLL	
114 : INT_COLL	
117	
117	
v	
v	
v	
. v	
123 . BASIN D5-2 CONTROL	
•	
129	
•	
135 . TOTAL D5-2	
135 . 10TAL D5-2	
138 D4-1	
144	·
. D4-1&2	
153 . D4-4	
• • • • • • • • • • • • • • • • • • • •	
159	
165	3-1
•	•
171 . WEST TAILS	•
. WEST TAILS	• •
. v	
174 . AREA-28-CONTROL	

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```
ID PATHFINDER, TAILINGS ROUTED TO EAST THROUGH CONTROL
ID BASED UPON PMF- RAINFALL OF 9.03 IN. COMPUTATION OF 1.0 MINUTE INCREMENTS
ID DATE=9-1-93
*FREE
*DIAGRAM
IT 1.0,,,300,,
IO 5,1
IN 1,,
PG HMR55
PC .0000,.0321,.0658,.1011,.1381,.1766,.2167,.2585,.3041,.3499,
PC .3959, .4423, .4900, .5400, .5939, .6535, .7211, .7994, .8913, 1.0003,
PC 1.1301,1.2848,1.4691,1.6878,1.9462,2.2499,2.6051,3.0181,3.4957,4.2583, PC 5.1498,5.7341,6.1786,6.5618,6.8905,7.1708,7.4087,7.6095,7.7784,7.9202,
PC 8.0390,8.1390,8.2237,8.2963,8.3597,8.4162,8.4679,8.5166,8.5635,8.6096,
PC 8.6555,8.7012,8.7467,8.7876,8.8269,8.8647,8.9008,8.9353,8.9682,8.9995,
PC 9.0300
KK D5-1
KO 0,,,,21,1,100
BA .1359
PR HMR55
LS 0,85
UD .323
KK POND5-CONTROL
KO 0,,,,21,1,300
RS 1,STOR,0,0
SA 0 1.11 5.4 8.5 15.2 31.7
SQ 0 9.9 29.2 62 114 279
SE 0 2 3 4 5 7
KK D5-5
KO 0,,,,21,1,100
BA .0104
PR HMR55
LS 0,87
UD .055
KK POND5-NORTH
KO 0,,,,21,1,100
HC 2
KK D5-4
KO 0,,,,21,1,100
BA .0519
PR HMR55
LS 0,87
UD .0226
KK DM-9
KO 0,,,,21,1,100
BA .046
PR HMR55
LS 0,90
UD .0811
KK SOUTH-UPPER-CONTROL
KO 0,,,,21,1,300
RS 1,STOR,0,0
SA .76 1.2 1.79 2.13 4.22
SQ 0 9.9 63 114 185
SE 0 2 4 5 6
KK D5-3
```

TABLE E-2. HEC-1 INPUT AND FLOW PATH FOR TAILINGS DISCHARGING TO THE EAST (continued).

```
KO 0,,,,21,1,100
BA .0439
PR HMR55
LS 0,91
UD .234
KK D5-7
KO 0,,,,21,1,100
BA .0099
PR HMR55
LS 0,87
UD .090
KK POND5-SOUTH
KO 0,,,,21,1,100
KK POND5-SOUTH & MILL
KO 0,,,,21,1,100
HC 2
KK SOUTH-LOWER-CONTROL
KO 0,,,,21,1,300
RS 1,STOR,0,0
SA .102 .14 .27 .35 1.2 2.11 SQ 0 9.9 63 114 399 723
SE 0 2 4 5 8 10
KK DM-10
KO 0,,,,21,1,100
BA .0148
PR HMR55
LS 0,91
UD .072
KK CROSS-SECTION HC-12
KO 0,,,,21,1,100
HC 2
KK D5-6
KO 0,,,,21,1,100
BA .0114
PR HMR55
LS 0,87
UD .060
KK SOUTH-BOTTOM-CONTROL
KO 0,,,,21,1,300
RS 1,STOR,0,0
SA .14 .30 .99
SQ 0 29 114
SE 0 3 5
KK POND5-AREA3
KO 0,,,,21,1,100
HC 2
ZZ
```

TABLE E-2. HEC-1 INPUT AND FLOW PATH FOR TAILINGS DISCHARGING TO THE EAST (continued).

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(>) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<) RETURN OF DIVERTED OR PUMPED FLOW
15	D5-1 V	
21	V POND5-CONTROL	
27	. D5-5	
33	POND5-NORTH	
36	D5-4	
42		DM-9 V
48		V SOUTH-UPPER
54		D5-3
60		D5-7
66		POND5-SOUTH
69		POND5-SOUTH & MILL
72		V SOUTH-LOWER-CONTROL
78		. DM-10
84	· · · · ·	CROSS-SECTION HC-12.
87		D5-6
93		. V . V . SOUTH-BOTTOM-CONTROL
99	·	POND5-AREA 3

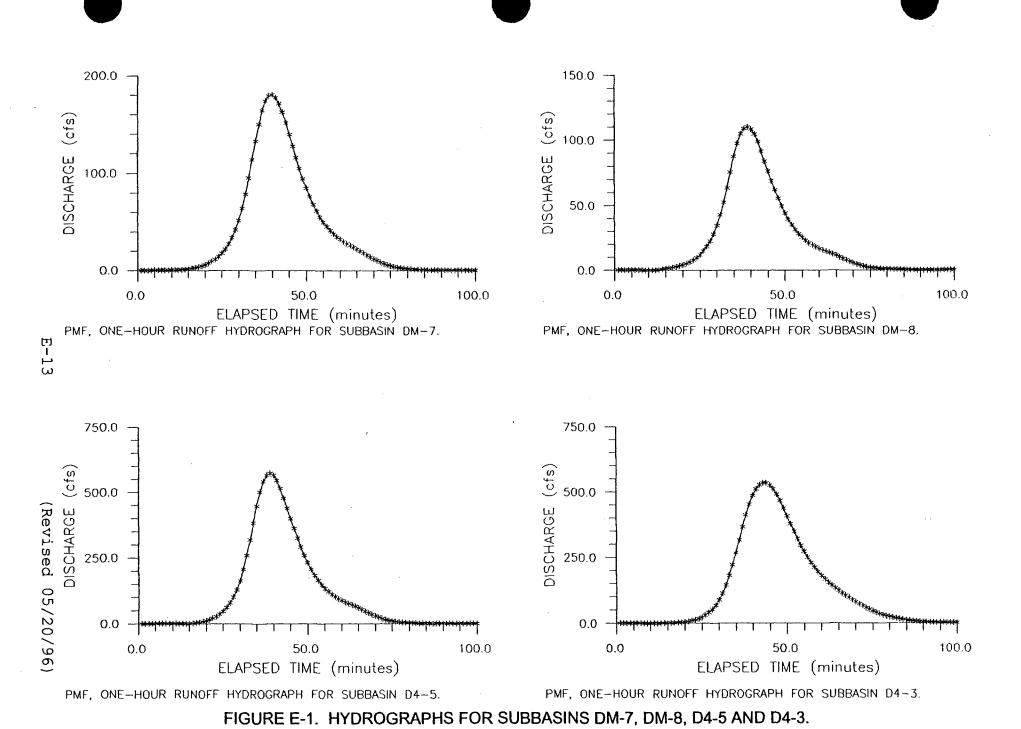
```
ID PATHFINDER, NORTH DRAINAGE ROUTED TO 2/8 RESERVOIR THROUGH CONTROL
 ID BASED UPON PMF- RAINFALL OF 9.03 IN. COMPUTATION OF 1.0 MINUTE INCREMENTS
ID DATE=4-18-96
*FREE
*DIAGRAM
IT 1.0,,,300,,
IO 5,1
IN 1,,
PG HMR55
PC .0000,.0321,.0658,.1011,.1381,.1766,.2167,.2585,.3041,.3499,
PC .3959,.4423,.4900,.5400,.5939,.6535,.7211,.7994,.8913,1.0003,
PC 1.1301,1.2848,1.4691,1.6878,1.9462,2.2499,2.6051,3.0181,3.4957,4.2583,
PC 5.1498,5.7341,6.1786,6.5618,6.8905,7.1708,7.4087,7.6095,7.7784,7.9202,
PC 8.0390,8.1390,8.2237,8.2963,8.3597,8.4162,8.4679,8.5166,8.5635,8.6096,
PC 8.6555, 8.7012, 8.7467, 8.7876, 8.8269, 8.8647, 8.9008, 8.9353, 8.9682, 8.9995,
PC 9.0300
KK N5
KO 0,,,,21,1,100
BA .036
PR HMR55
LS 0,91
UD .2236
KK N5-SURGE
KO 0,,,,21,1,200
RS 1,STOR,0,0
SA 0 0.69 2.39 4.78
SQ 0 70 200 500
SE 0 1 2 3
KK N7
KO 0,,,,21,1,150
BA .021
PR HMR55
LS 0,91
UD .1124
KK N6
KO 0,,,,21,1,100
BA .047
PR HMR55
LS 0,91
UD .1055
KK N6-SURGE
KO 0,,,,21,1,300
RS 1,STOR,0,0
SA 0 1.56 2.81
SQ 0 70 200
SE 0 1 2
KK N8
KO 0,,,,21,1,150
BA .060
PR HMR55
LS 0,91
UD .2769
KK UPPER NORTH
KO 0, \dots, \overline{2}1, 1, 300
HC 4
KK UP NORTH CON
KO 0,,,,21,1,300
```

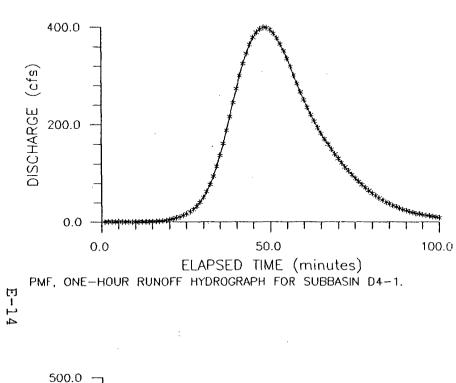
```
RS 1,STOR,0,0
SA 0 .20 .50 .89 1.32 1.89 2.58 3.40 4.89 SQ 0 2.25 14.1 41.7 90 163 265 400 570
SE 0 1 2 3 4 5 6 7 8
KK N9
KO 0,,,,21,1,100
BA .037
PR HMR55
LS 0,91
UD .1005
KK N11
KO 0,,,,21,1,100
BA .013
PR HMR55
LS 0,91
UD .0356
KK N9+N11+UPPER_NORTH
KO 0,,,,21,1,30\overline{0}
HC 3
KK N10
KO 0,,,,21,1,100
BA .003
PR HMR55
LS 0,91
UD .0260
KK N12
KO 0,,,,21,1,100
BA .018
PR HMR55
LS 0,91
UD .1235
KK N13
KO 0,,,,21,1,100
BA .013
PR HMR55
LS 0,91
UD .01
KK LOWER NORTH
KO 0, , , , \overline{2}1, 1, 300
HC 4
KK LOW_NORTH_CON
KO 0,,,,21,1,300
RS 1,STOR,0,0
SA 0 7.61 7.98 8.13 8.26
SQ 0 2.25 14.1 41.7 90
SE 0 1 2 3 4
zz
```

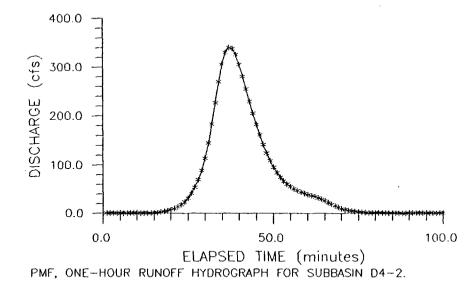
TABLE E-3. HEC-1 INPUT AND FLOW PATH FOR NORTH DRAINAGE. (continued).

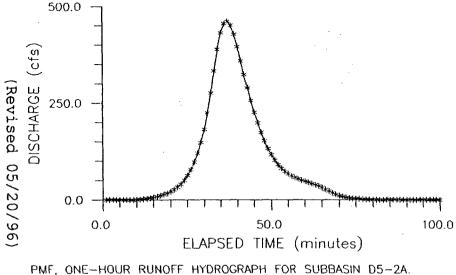
SCHEMATIC DIAGRAM OF STREAM NETWORK

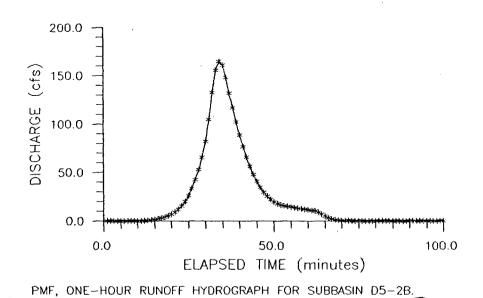
INPUT LINE	(V) ROUTING	(>) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<) RETURN OF DIVERTED OR PUMPED FLOW
15	ns V V	
21	N5-SURGE	·
27		7
33	:	. N6 . V
39	•	. V . N6-SURGE
45	: :	
51	UPPER_NORTH	· · · · · · · · · · · · · · · · · · ·
54	V V UP_NORTH_CON	
60		
66		N11
72	N9+N11+UPPER_NORTH	
75	. NIC	
81		N12
87	:	. N13
93	LOWER_NORTH	
96	V V LOW_NORTH_CON	





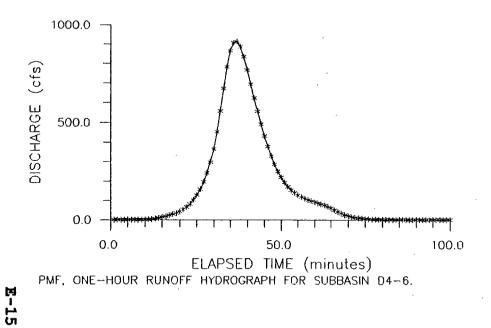


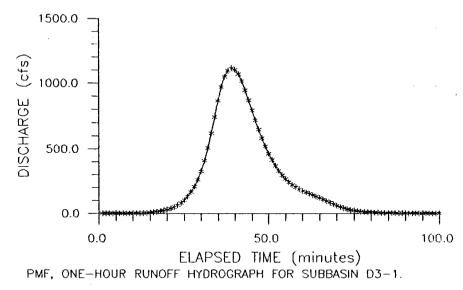


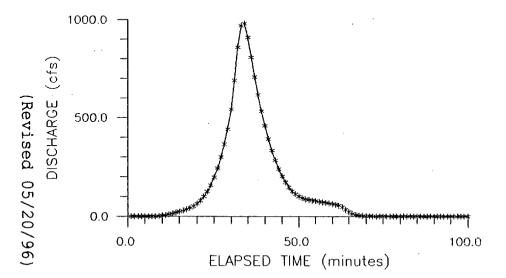


SINS D4-1, D4-2, D5-2A AND D5-2B.

FIGURE E-2. HYDROGRAPHS FOR S

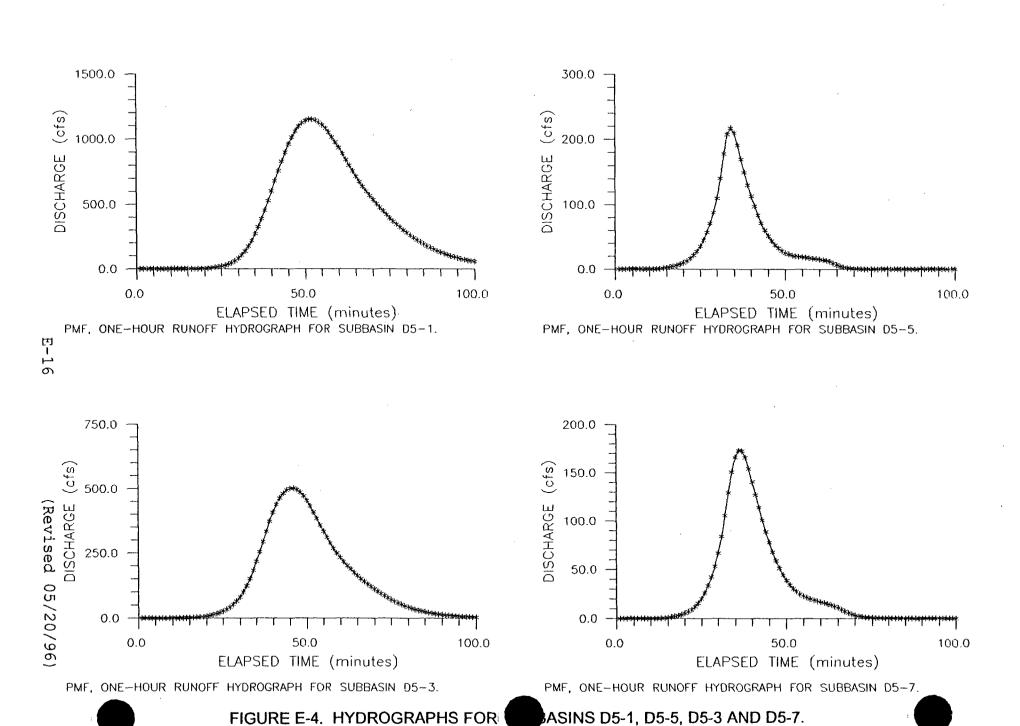


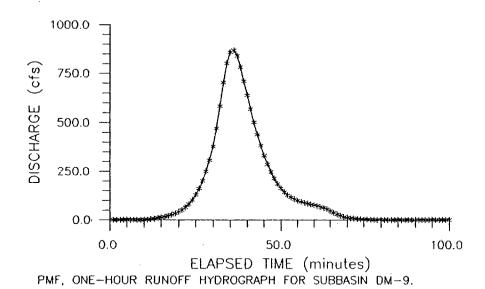


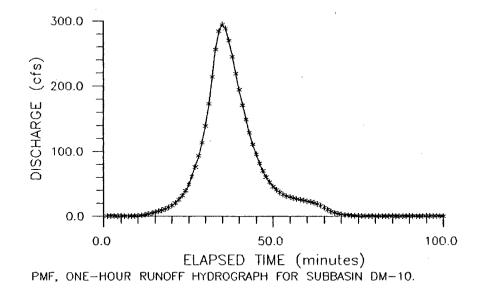


PMF, ONE-HOUR RUNOFF HYDROGRAPH FOR SUBBASIN D4-4.

FIGURE E-3. HYDROGRAPHS FOR SUBBASINS D4-6, D3-1 AND D4-4.







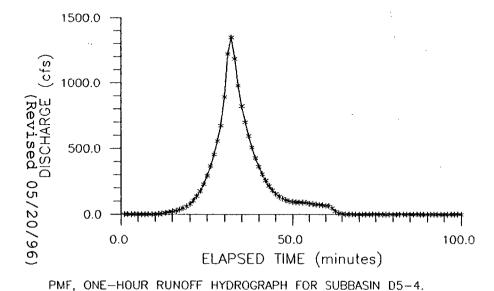
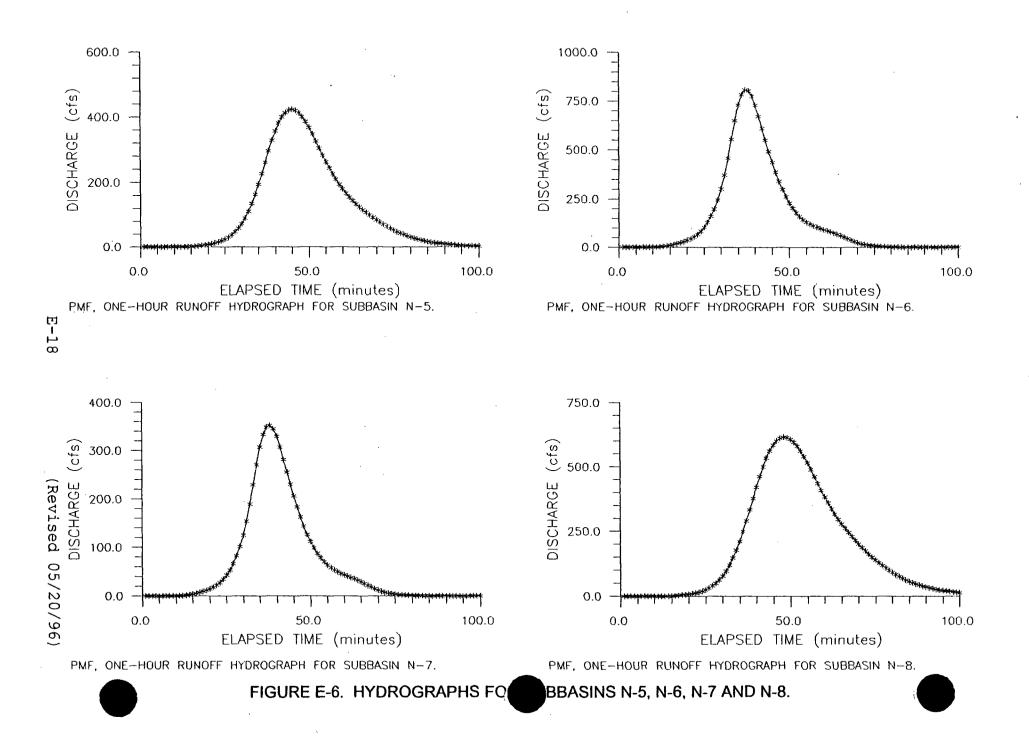
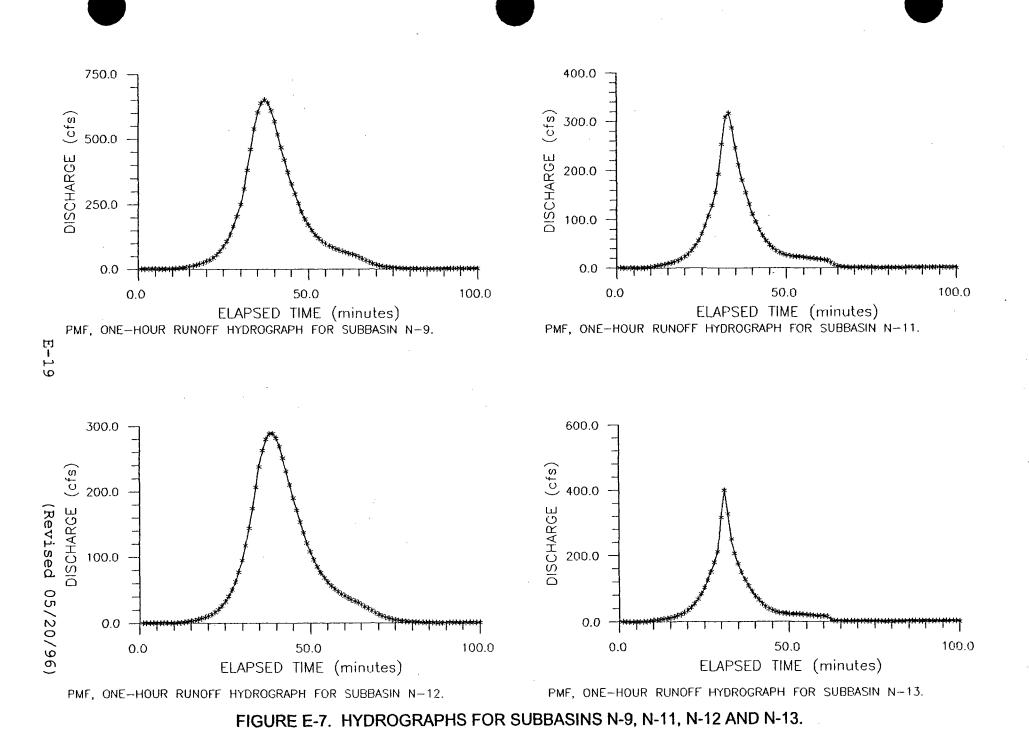
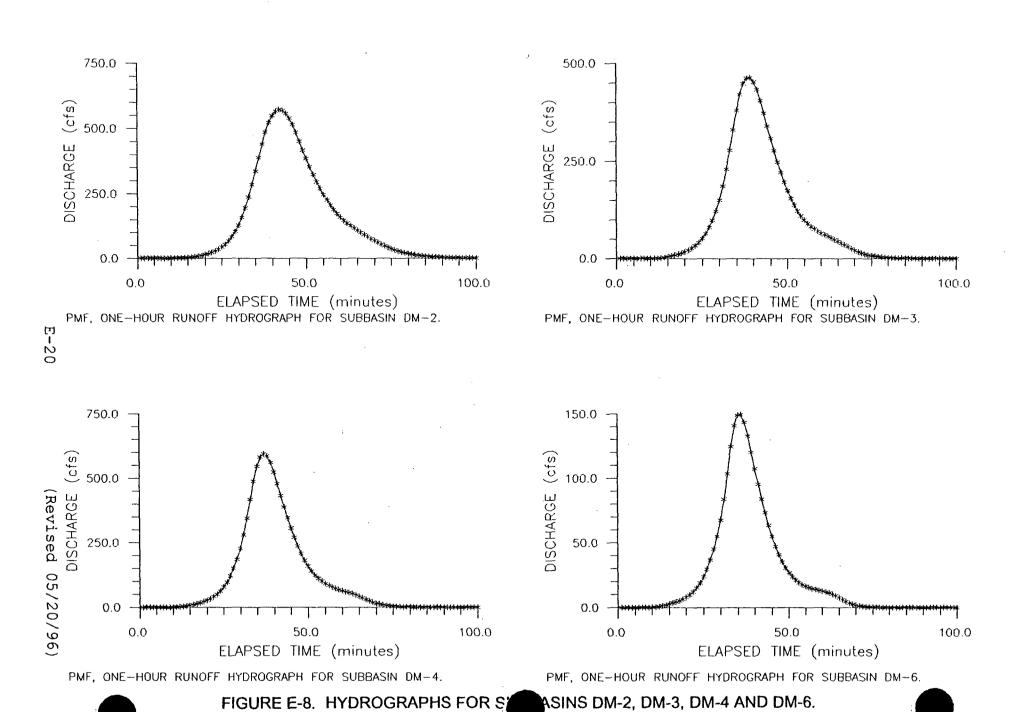
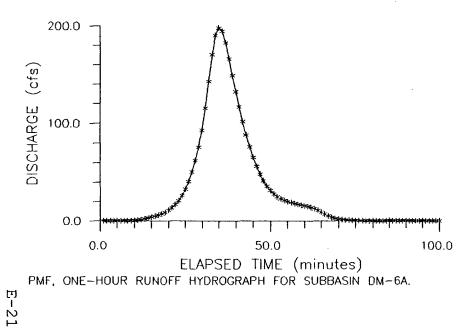


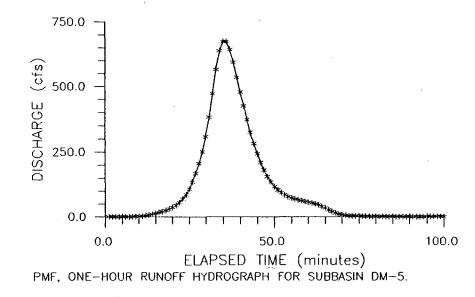
FIGURE E-5. HYDROGRAPHS FOR SUBBASINS DM-9, DM-10 AND D5-4.











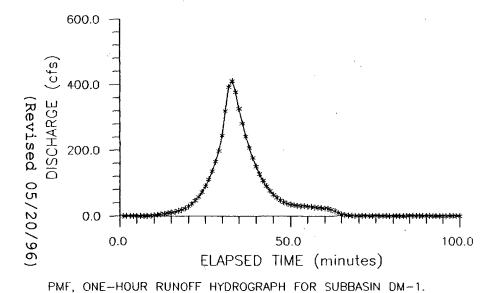
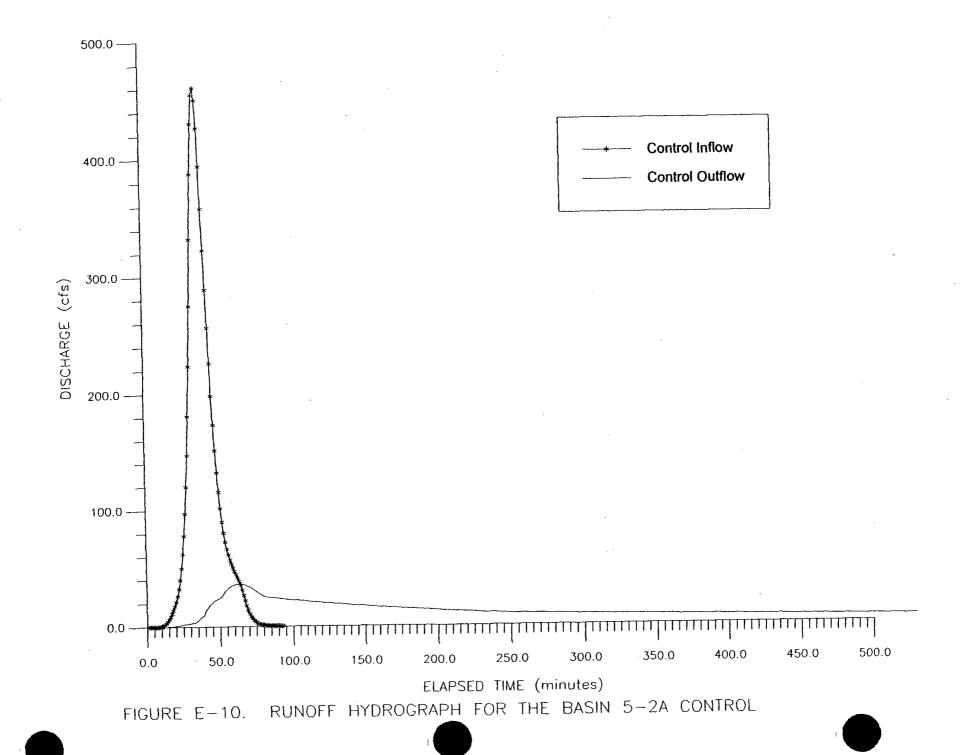
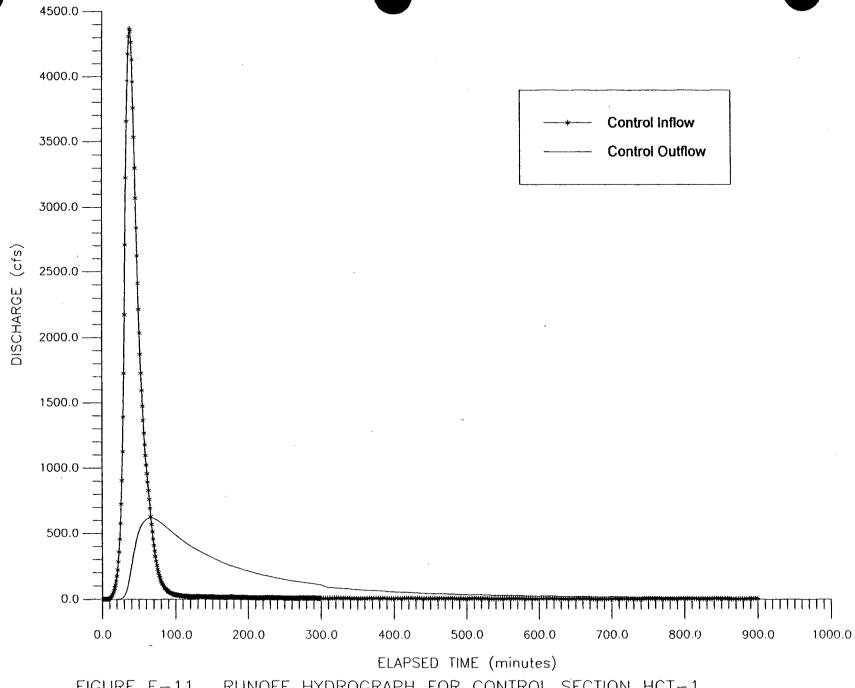
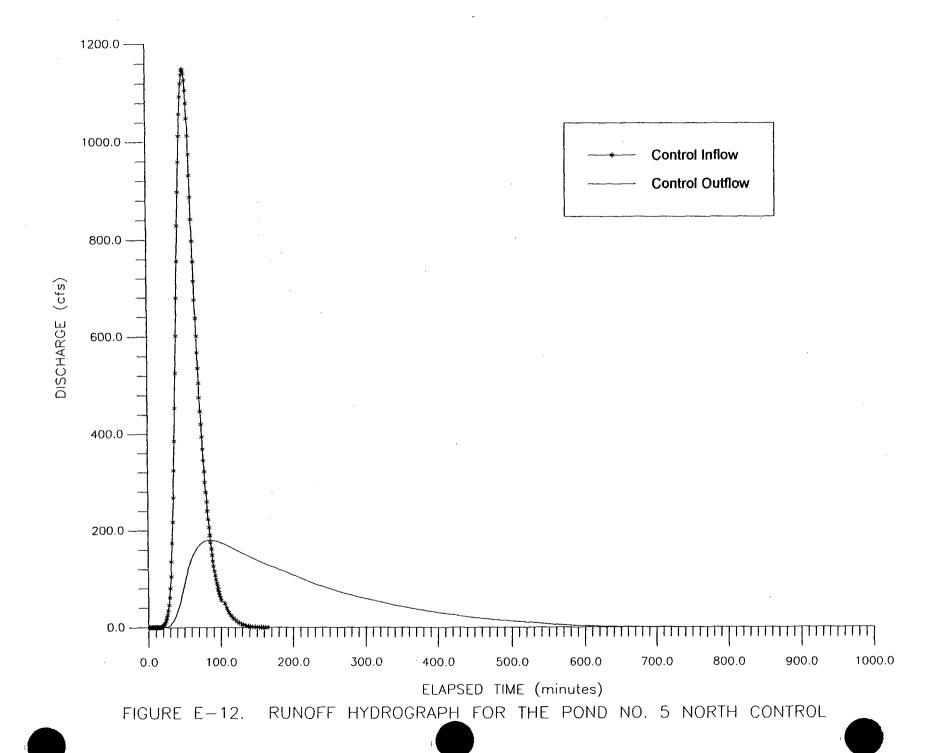


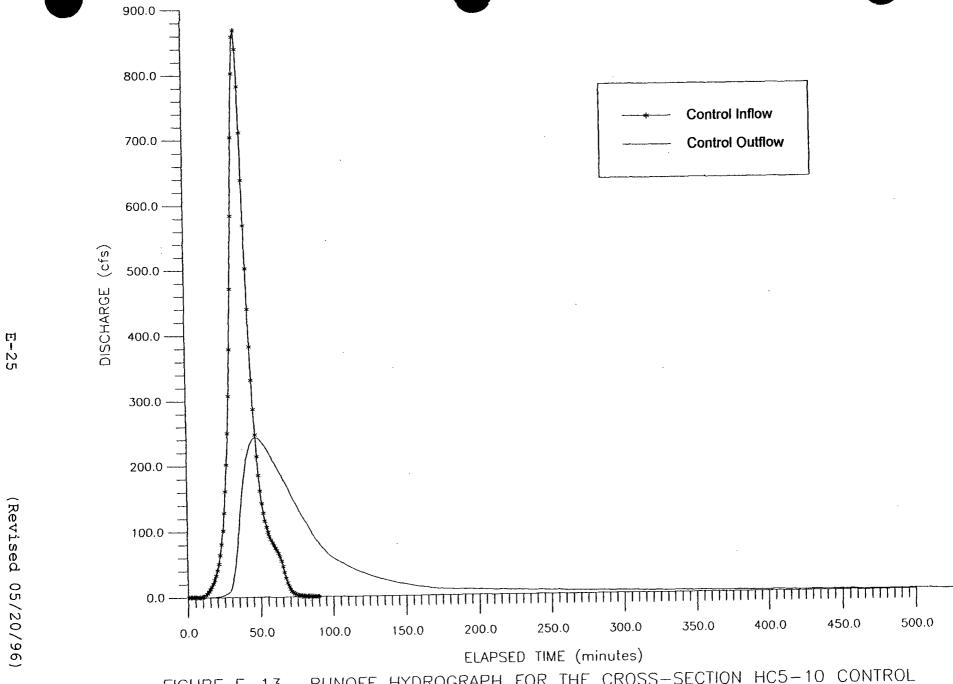
FIGURE E-9. HYDROGRAPHS FOR SUBBASINS DM6-6A, DM-5 AND DM-1.



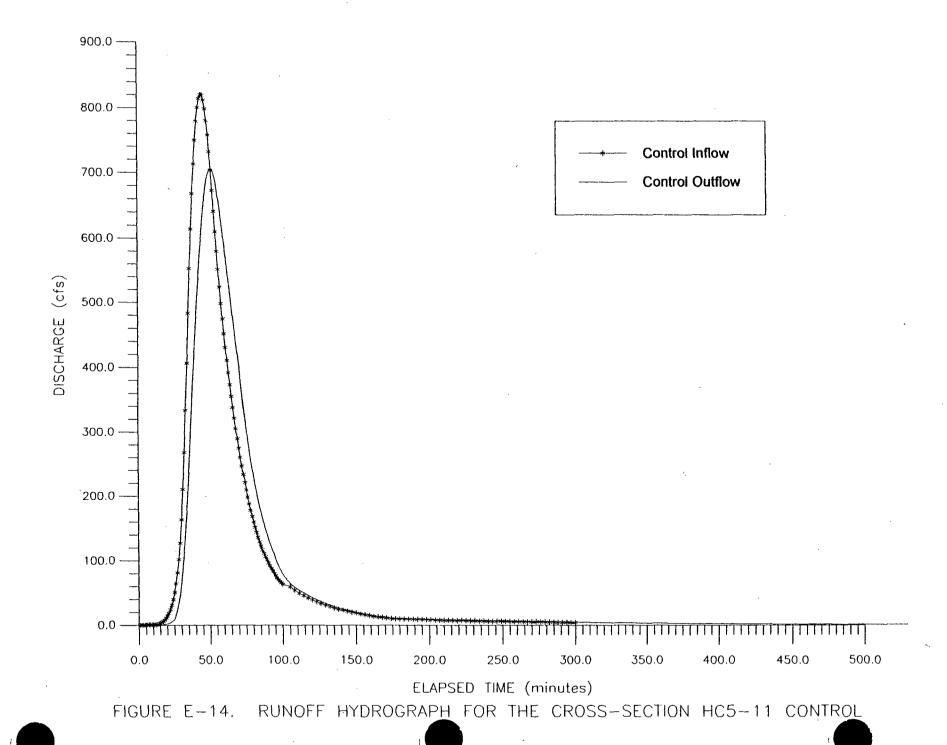


RUNOFF HYDROGRAPH FOR CONTROL SECTION HCT-1 FIGURE E-11.





RUNOFF HYDROGRAPH FOR THE CROSS-SECTION HC5-10 CONTROL FIGURE E-13.



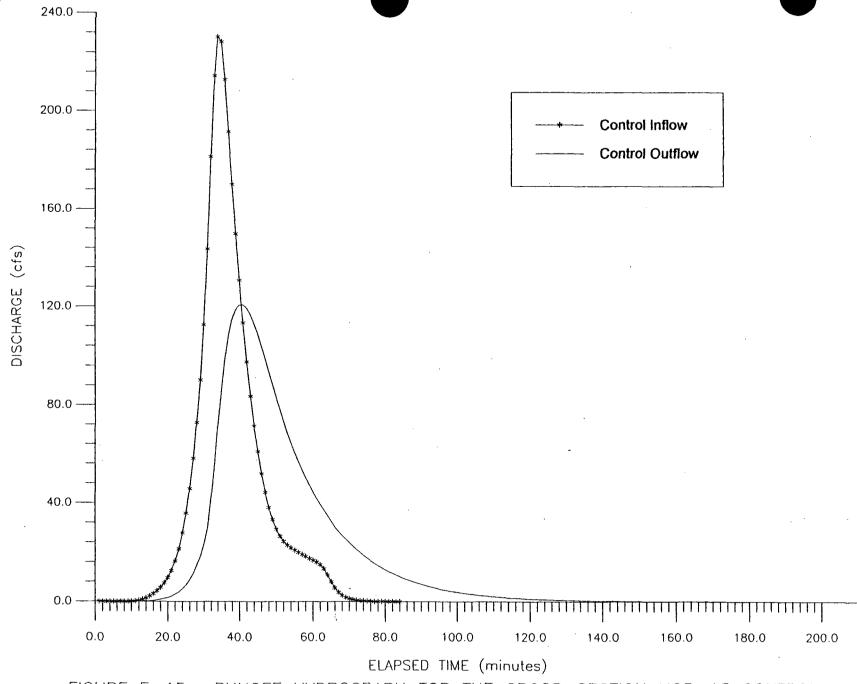


FIGURE E-15. RUNOFF HYDROGRAPH FOR THE CROSS-SECTION HC5-13 CONTROL

APPENDIX F
HEC-2 INPUT FILES

APPENDIX F

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						Pac	<u>je</u>	Νţ	mber
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TABLE F-1. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL A.

		ER SHIRL	EY BASIN	TAILING	s reclam	ATION		
	CHANNEL							
	PMF - 50	BCRITICA	L					2.00
J1				-1.0			180.0	7128.0
J2 -1		-1.0						
NC 0.035	0.035	0.035	0.1	0.3				
X180	4.0	0.01	36.00					
X2 180.0								
GR7128.0	0.01	7126.00	8.00	7126.00	28.00	7128.00	36.00	
X190	4.0	7126.00 0.01	36.00	10.00	10.00	10.00		
X2 180.0								
GR7128.4	0.01	7126.40 0.01	8.00	7126.40	28.00	7128.40	36.00	
X1100	4.0	0.01	36.00	10.00	10.00	10.00		
X2 180.0								
GR7128.8	0.01	7126.80	8.00	7126.80	28.00	7128.80	36.00	
X1200	4.0	0.01	36.00	100.00	100.00	100.00		
X2 170.0								
GR7130.8	0.01	7126.80 0.01 7128.80 0.01	8.00	7128.80	28.00	7130.80	36.00	
X1 220	4.0	0.01	36.00	20.00	20.00	20.00		
GR7131.4	0.01	7129.40 0.01	8.00	7129.40	28.00	7131.40	36.00	
X1240	4.0	0.01	36.00	20.00	20.00	20.00		
X2 160.0								
GR7132.0	0.01	7130.00	8.00	7130.00	28.00	7132.00	36.00	
X1 260	4.0	0.01	36.00	20.00	20.00	20.00		
X1240 X2 160.0 GR7132.0 X1 260 X2 153.3 GR7133.2								
GR7133.2 X1 280	0.01	7131.20	8.00	7131.20	28.00	7133.20	36.00	
X1 280	4.0	0.01	36.00	20.00	20.00	20.00		
X2 146.7								
X2 146.7 GR7134.4	0.01	7132.40	8.00	7132.40	28.00	7134.40	36.00	
X2 140.0								
X2 140.0 GR7135.6 X1400 X2 120.0	0.01	7133.60	8.00	7133.60	28.00	7135.60	36.00	
X1400	4.0	0.01	36.00	100.00	100.00	100.00		
X2 120.0								
GR7137.2 X1 500 X2 110.0	0.01	7135.20	8.00	7135.20	28.00	7137,20	36.00	
X1 500	4.0	0.01	36.00	100.00	100.00	100.00		
X2 110.0								
X2 110.0 GR7137.9 X1600 X2 100.0	0.01	7135.85	8.00	7135.85	28.00	7137.85	36.00	
X1600	4.0	0.01	36.00	100.00	100.00	100.00		
X2 100.0								
X2 100.0 GR7138.5	0.01	7136.50	8.00	7136.50	28.00	7138.50	36.00	
X1 700 /	4.0	0.01	36.00	100.00	100.00	100.00		
GR7139.1 X1800	0.01	7137.10	8.00	7137.10	28.00	7139.10	36.00	
X1800	4.0	0.01	36.00	100.00	100.00	100.00		
GR7139.7	0.01	7137.70	8.00	7137.70	28.00	7139.70	36.00	
EJ								
ER								

TABLE F-2. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL A.

T1 T2	PATHFIND CHANNEL	ER SHIRLE A	Y BASIN	TAILING:	s reclam	ATION		
T 3	PMF - SU	PERCRITIC	AL .					
J1			1.0	-1.0			80.0	7138.5
J2 -1	0	-1.0						
NC 0.035	0.035	-1.0 0.035	0.1	0.3				
X1800	4.0	0.01	36.00	100.00	100.00	100.00		
GR7139.7	0.01	7137.70	8.00	7137.70	28.00	7139.70	36.00	
X1 700	4.0	0.01	36.00	100.00	100.00	100.00		
X2 90.0								
GR7139.1	0.01	7137.10	8.00	7137.10	28.00	7139.10	36.00	
X1600	4.0	7137.10	36.00	100.00	100.00	100.00		
X2 100.0	ı							
GR7138.5	0.01	0.01 7136.50 0.01	8.00	7136.50	28.00	7138.50	36.00	
X1 500	4.0	0.01	36.00	100.00	100.00	100.00		
X2 110.0								
GR7137.9	0.01	7135.85 0.01	8.00	7135.85	28.00	7137.85	36.00	
X1400	4.0	0.01	36.00	100.00	100.00	100.00		
X2 120.0								
GR7137.2	0.01	7135.20	8.00	7135.20	28.00	7137.20	36.00	
X1300 X2 140.0	4.0	0.01	36.00	20.00	20.00	20.00		
X2 140.0								
GR7135.6	0.01	7133.60 0.01	8.00	7133.60	28.00	7135.60	36.00	
X1 280	4.0	0.01	36.00	20.00	20.00	20.00		
X2 146.7		7132.40						
GR7134.4	0.01	7132.40	8.00	7132.40	28.00	7134.40	36.00	
X1 260	4.0	0.01 7131.20 0.01	36.00	20.00	20.00	20.00		
X2 153.3								
GR7133.2	0.01	7131.20	8.00	7131.20	28.00	7133.20	36.00	
X1240	4.0	0.01	36.00	20.00	20.00	20.00		
X2 160.0		*						
GR7132.0	0.01	7130.00	8.00	7130.00	28 00	7132 00	36 00	
X1 220	4.0	0.01	36.00	20.00	20.00	20.00		
X2 165.0		7130.00 0.01 7129.40 0.01						
GR/131.4	0.01	/129.40	8.00	7129.40	28.00	7131.40	36.00	
		0.01	36.00	100.00	100.00	100.00		
X2 170.0								
GR7130.8	0.01	7128.80	8.00	7128.80	28.00	7130.80	36.00	
X1100	4.0	7128.80 0.01	36.00	10.00	10.00	10.00		
X2 180.0		7126.80						
GR7128.8	0.01	7126.80	8.00	7126.80	28.00	7128.80	36.00	
X190	4.0	0.01	36.00	10.00	10.00	10.00		
X2 180.0								
GR7128.4	0.01	7126.40	8.00	7126.40	28.00	7128.40	36.00	
X180	4.0	0.01	36.00					
X2 180.0								
GR7128.0	0.01	7126.00	8.00	7126.00	28.00	7128.00	36.00	
EJ								
ER								

TABLE F-3. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL D.

	PATHFIND CHANNEL	ER SHIRLE	y Basin	TAILINGS	RECLAM	ATION		
T3 J1	PMF - SU	BCRITICAL		-1.0			1325.0	7110 0
J2 -1	0	-1.0		-1.0			1323.0	7110.0
NC 0.035				0.3				
X1100 X21325.0	4.0	0.01	66.00					
GR7109.8	0.01	7107.80		7107.80			66.00	
X1150 X21325.0	4.0	0.01	66.00	50.00	50.00	50.00		
		7083.50	8.00	7083.50	58.00	7085.50	66.00	
X1200	4.0	0.01	66.00	50.00	50.00	50.00		
X21300.0 GR7087.0		7085.00	8.00	7085.00	58.00	7087.00	66.00	
X1413	4.0				15.00		00.00	
X21300.0 GR7087.8		7085.80	8 00	7085.80	E0 00	7097 00	66 00	
X1 220	4.0			5.00			66.00	
X21297.1								
X1 240	4.0	7086.06 0.01		7086.06			66.00	
X21285.3								
GR7089.1 X1 260	0.01	7087.09 0.01	8.00	7087.09	58.00	7089.09	66.00	
X21273.5		0.01	66.00	20.00	20.00	20.00		
GR7090.1		7088.13 0.01	8.00	7088.13			66.00	
X1 280 X21261.8		0.01	66.00	20.00	20.00	20.00	•	
GR7091.2		7089.17	8.00	7089.17	58.00	7091.17	66.00	
X1300	4.0	0.01		20.00				
X21250.0 GR7092.2		7090 20	8 00	7090.20	58 00	7002 20	66.00	
X1 320	4.0	0.01		20.00			00.00	
X21240.0		7000 00						
GR7093.0 X1 340	4.0	7090.98 0.01	66.00	7090.98			66.00	
X21230.0				20.00	20.00	20.00		
GR7093.8		7091.76		7091.76			66.00	
X1 360 X21220.0	4.0	0.01	66.00	20.00	20.00	20.00		
GR7094.5	0.01	7092.54	8.00	7092.54	58.00	7094.54	66.00	
X1 380 X21210.0		0.01	66.00	20.00	20.00	20.00		
GR7095.3	0.01	7093.32	8.00	7093.32	58.00	7095.32	66.00	
X1400	4.0	7093.32 0.01	66.00	20.00	20.00	20.00		
X21200.0 GR7096.1		7094.10	8.00	7094 10	58 00	7096 10	66.00	
X1 420		0.01					00.00	
X21190.0		7004 74	2 22					
GR7096.7 X1 440		7094.74 0.01	66.00	7094.74 20.00		7096.74	66.00	
X21180.0								
GR7097.4 X1 460		7095.38				7097.38	66.00	
X21170.0		0.01	00.00	20.00	20.00	20.00		
GR7098.0		7096.02		7096.02		7098.02	66.00	
X1 480 X21160.0	4.0	0.01	66.00	20.00	20.00	20.00		
GR7098.7	0.01	7096.66	8.00	7096.66	58.00	7098.66	66.00	
X1 500 X21150.0	4.0		66.00	20.00	20.00	20.00		
GR7099.3	0.01	7097.30	8.00	7097.30	58.00	7099.30	66.00	
X1 520	4.0	0.01	66.00	20.00	20.00	20.00	00.00	
X21140.0 GR7099.9	0.01	7097.94	0 00	7097.94	EO 00	7000 04		
X1 540	4.0	0.01	66.00	20.00	20.00	7099.94 20.00	66.00	
X21130.0								
GR7100.6 X1 560	0.01 4.0	7098.58 0.01	8.00 ·	7098.58		7100.58	66.00	ě
X21120.0			00.00	20.00	20.00	20.00		
GR7101.2		7099.22				7101.22	66.00	
X1 580 X21110.0	4.0	0.01	66.00	20.00	20.00	20.00		
GR7101.9					58.00	7101.86	66.00	
X1600 X21100.0	4.0	0.01	66.00	20.00	20.00	20.00		
21100.0						-		

TABLE F-3. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL D (continued).

GR7102.5 X1700		7100.50 0.01				7102.50 100.00	66.00
X21000.0 GR7104.9 X1 800	0.01 4. 0	7102.90 0.01	8.00 66.00	7102.90 100.00	58.00 100.00	7104.90 100.00	66.00
X2 950.0 GR7105.5 X1900	0.01	7103.55 0.01	8.00 66.00	7103.55 100.00	58.00 100.00	7105.55 100.00	66.00
X2 900.0 GR7106.2 X1 1000	0.01 4.0	7104.20	8.00 66.00	7104.20 100.00	58.00 100.00	7106.20 100.00	66.00
X2 875.0 GR7107.2 X11100		7105.15				7107.15	66.00
X11200	0.01 4.0	7106.10 0.01	8.00 66.00	7106.10 100.00	58.00 100.00	7108.10 100.00	66.00
X2 797.0 GR7109.0 X1 1220	0.01 4.0	7107.00	8.00 64.00	7107.00 20.00	58.00 20.00	7109.00 20.00	66.00
X2 797.0 GR7109.2 X1 1240 X2 797.0	0.01 4.0	7107.16	8.00 62.00	7107.16 20.00	56.00 20.00	7109.16 20.00	64.00
GR7109.3 X1 1260 X2 797.0	0.01 4.0	7107.32	8.00 60.00	7107.32 20.00	54.00 20.00	7109.32 20.00	62.00
GR7109.5 X1 1280 X2 797.0	0.01 4.0	7107.48 0.01	8.00 58.00	7107.48 20.00	52.00 20.00	7109.48 20.00	60.00
GR7109.6 X11300 X2 797.0	0.01 4.0	7107.64	8.00 56.00	7107.64 20.00	50.00	7109.64 20.00	58.00
GR7109.8 X1 1320 X2 797.0		7107.80	8.00 55.00	7107.80 20.00	48.00 20.00	7109.80 20.00	56.00
GR7110.0 X1 1340 X2 797.0	0.01 4.0	7108.04	8.00 54.00	7108.04 20.00	47.00 20.00	7110.04 20.00	55.00
GR7110.3 X1 1360 X2 797.0	0.01	7108.28	6.00 53.00	7108.28 20.00	46.00 20.00	7110.28 20.00	54.00
GR7110.5 X1 1380 X2 797.0	0.01 4.0	7108.52	8.00 52.00	7108.52 20.00	45.00 20.00	7110.52 20.00	53.00
GR7110.8 X11400 X2 797.0	4.0	7108.76 0.01	51.00	7108.76 20.00	20.00	7110.76 20.00	52.00
GR7111.0 X1 1420 X2 797.0	0.01 4.0	7109.00	8.00 49.70	7109.00	43.00 20.00	7111.00 20.00	51.00
GR7111.3 X1 1440 X2 797.0		7109.27				7111.27 20.00	49.70
GR7111.5 X1 1460 X2 797.0	0.01 4.0	7109.54	8.00 47.10	7109.54 20.00	40.40 20.00	7111.54 20.00	48.40
GR7111.8 X1 1480 X2 797.0	4.0	7109.81 0.01	45.80	20.00	20.00	7111.81 20.00	47.10
GR7112.1 X1 1500 X2 797.0		7110.08		7110.08 20.00	37.80 20.00	7112.08 20.00	45.80
GR7112.4 X1 1520 X2 797.0	4.0		43.20	7110.35	36.50 20.00	7112.35	44.50
GR7112.6 X1 1540 X2 797.0	0.01 4.0	7110.62	8.00 41.90	7110.62 20.00	20.00	7112.62	43.20
GR7112.9 X1 1560 X2 797.0	4.0		40.60	20.00	20.00	7112.89 20.00	41.90
GR7113.2 X1 1580 X2 797.0	4.0			7111.16 20.00			40.60
GR7113.4 X11600 X2 797.0		7111.43					39.30
GR7113.7	0.01	7111.70	8.00	7111.70	30.00	7113.70	38.00

TABLE F-3. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL D (continued).

							-
X2 797 0					20.00		
GR7113.9 X1 1640	0.01 4.0	7111.90 0.01	8.00 37.20	7111.90 20.00	29.60 20.00	7113.90 20.00	37.60
X2 797.0							37.20
X1 1660 X2 797.0	4.0	0.01	36.80	20.00	20.00	7114.10 20.00	
	0.01	7112.30	8.00 36.40	7112.30	28.80	7114.30 20.00	36.80
X2 797.0							36.40
V2 202 0				20.00	20.00	7114.50 20.00	50.40
GR7114.7 X11800		7112.70			28.00 100.00	7114.70	36.00
X2 797.0						7115.60	36.00
X11850	4.0	7113.60	36.00				36.00
X2 797.0 GR7116.0	0.01	7114.00	8.00	7114.00	28.00	7116.00 50.00	36.00
X1 1900 X2 819.0							
GR7116.1 X12000 X2 863.0	4.0	7114.10	15.33 58.00	7114.10	35.33 100.00	7116.10	43.33
GR7116.3 X1 2100	0.01	7114.30	30.00	7114.30	50.00	7116.30	58.00
X2 719.5							58.00
	4.0	0.01	58.00	100.00	100.00	7116.42	30.00
X2 576.0 GR7116.5	0.01	7114.55	30.00	7114.55	50.00	7116.55 100.00	58.00
¥2 432 5							
X1411	4.0	7114.67	30.00 58.00	7114.67	50.00	7116.67 100.00	58.00
X2 289.0 GR7116.8 X12500	0.01	7114.80		7114.80	50.00 100.00	7116.80 100.00	58.00
X2 289.0							50.00
GR7116.9 X1 2600	4.0	0.01	47.00	100.00	100.00	7116.90	58.00
X2 289.0 GR7117.8						7117.75	47.00
X12700 X2 289.0					100.00		
GR7118.6 X1 2800	0.01 4.0	7116.60	8.00 36.00	7116.60	28.00 100.00	7118.60 100.00	36.00
X2 289.0 GR7120.1	0.01	7118.13	8.00	7118.13	28.00	7120,13	36.00
X1 2900 X2 289.0		7118.13		100.00		100.00	
GR7121.7 X13000	0.01	7119.67	8.00 36.00	7119.67	28.00	7121.67 100.00	36.00
X2 289 0		7121.20				7123.20	36.00
X13100 X2 289.0	4.0	0.01	36.00	100.00	100.00	100.00	36.00
GR7125.0				7123.00		7125.00	36.00
X1 3110 X2 277.1	4.0	0.01		10.00			
GR7125.3 X1 3130	4.0	7123.32	8.00 36.00	7123.32	28.00 20.00	7125.32 20.00	36.00
X2 253.2 GR7126.0	0.01	7123.96	8.00	7123.96	28.00	7125.96	36.00
X1 3150 X2 229.3				20.00	20.00	20.00	
GR7126.6 X1 3170	0.01	7124.60	8.00 36.00			7126.60 20.00	36.00
X2 205.5 GR7127.2	0.01	7125.24	8.00	7125.24	28.00	7127.24	36.00
X1 3190 X2 181.6	4.0	0.01	36.00	20.00			
GR7127.9 X1 3210	0.01	7125.88	8.00 36.00	7125.88		7127.88	36.00
X2 157.7 GR7128.5		7126.52		7126.52		7128.52	36.00
X1 3230 X2 133.9	4.0			20.00		20.00	30.00
A4 133.3							

TABLE F-3. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL D (continued).

GR7129.2	0.01	7127.16	8.00	7127.16	28.00	7129.16	36.00
X1410	4.0	0.01	36.00	20.00	20.00	20.00	
X2 110.0							
GR7129.8	0.01	7127.80 0.01	8.00	7127.80	28.00	7129.80	36.00
X1 3300	4.0	0.01	36.00	50.00	50.00	50.00	
X2 110.0		****					
GR7130.8	0.01	/128.80	8.00	/128.80	28.00	7130.80	36.00
X13400 X2 110.0	4.0	0.01	36.00	100.00	100.00	100.00	
GR7132.8	0.01	7130 00	9 00	7130 60	30 00	2122 00	36.00
X1 3420							36.00
X2 110.0	4.0	0.01	30.00	20.00	20.00	20.00	
GR7133.5	0.01	7131 50	8 00	7131 50	28 00	7193 50	36.00
X1 3440	4.0	0.01	36.00	20.00		20.00	30.00
V2 110 0							
GR7134-2 X1 3460	0.01	7132.20	8.00	7132.20	28.00	7134.20	36.00
X1 3460	4.0	0.01	36.00	20.00	20.00	20.00	30.00
X2 110.0							
GR7134.9	0.01	7132.90	8.00	7132.90	28.00	7134.90	36.00
X1 3480	4.0	0.01	36.00	20.00	20.00	20.00	
X2 110.0							
GR7135.6							36.00
X1 3500	4.0	0.01	36.00	20.00	20.00	20.00	
X2 110.0							
GR7136.3 X1 3520	0.01	7134.30	8.00	7134.30	28.00	7136.30	36.00
	4.0	0.01	36.00	20.00	20.00	20.00	
X2 110.0							
GR7137.0 X1 3540	0.01	7135.00	8.00	7135.00	28.00	7137.00	36.00
	4.0	0.01	36.00	20.00	20.00	20.00	
X2 110.0	0 01	71.25 20		7175 70			
GR7137.7	0.01	/135./0	8.00	/135./0	28.00		36.00
X1 3560 X2 110.0	4.0	0.01	36.00	20.00	20.00	20.00	
GR7138.4	0 01	7176 40	g 00	7136 40	20.00	7120 .0	26.00
X1 3580	0.01	7136.40	36.00	7136.40	28.00	7138.40	36.00
X2 110.0	4.0	0.01	36.00	20.00	20.00	20.00	
GR7139-1	0.01	7137 10	8 00	7137 10	28 00	7130 10	36.00
X13600	4 0	7137.10	36.00	20.00	20.00	20.00	50.00
X2 110.0	•••	0.01	20.00	20.00	20.00	20.00	
GR7139.8	0.01	7137.80	8.00	7137.80	28.00	7139.80	36.00
EJ			2.00				50.00
ER							

TABLE F-4. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL D.

	PATHFIND		Y BASIN	TAILINGS	RECLAMA	ATION		
	CHANNEL PMF - SU		AL					
J1			1.0	-1.0			110.0	7138.5
J2 +1 NC 0.03	0 0.035	-1.0 0.035	0.1	0.3				
X13600	4.0	0.01	36.00	20.00	20.00	20.00		
X2 110.) 3 (1)	7137.80	8.00	7137.80	28.00	7139.80	36.00	
X1 3580	4.0	0.01	36.00	20.00	20.00	20.00		
X2 110.	0 01	7137 10	8.00	7137.10	28.00	7139.10	36.00	
X1 3560	4.0	0.01	36.00	20.00	20.00	7139.10 20.00		
X2 110.	0.01	7136.40	8.00	7136.40	28.00	7138.40	36.00	
X1 3540	4.0	0.01	36.00	20.00	20.00	7138.40 20.00		
X2 110.	0.01 7. 0.01	7135.70	8.00	7135.70	28.00	7137.70	36.00	
X1 3520	4.0	0.01	36.00	20.00	20.00	7137.70 20.00		
X2 110.	0.01	7135.00	8.00	7135.00	28.00	20.00 7137.00 20.00 7136.30 20.00 7135.60 20.00 7134.90	36.00	
X1 3500	4.0	0.01	36.00	20.00	20.00	20.00		
GR7136.	0.01	7134.30	8.00	7134.30	28.00	7136.30	36.00	
X1 3480	4.0	0.01	36.00	20.00	20.00	20.00		
X2 110.	0 6 0.01	7133.60	8.00	7133.60	28.00	7135.60	36.00	
X1 3460	4.0	0.01	36.00	20.00	20.00	20.00		
X2 110.	0 0 0 0 1	7132.90	8.00	7132.90	28.00	7134.90	36.00	
X1 3440	4.0	0.01	36.00	20.00	20.00	20.00 7134.90 20.00		
X2 110.	0 2 0 .01	7132.20	8.00	7132.20	28.00	7134.20 20.00	36.00	
X1 3420	4.0	0.01	36.00	20.00	20.00	20.00		
GR7133.	0.01	7131.50	8.00	7131.50	28.00	7133.50	36.00	
X13400	4.0	0.01	36.00	100.00	100.00	100.00		
GR7132.	B 0.01	7130.80	8.00	7130.80	28.00	7132.80 50.00	36.00	
X1 3300	4.0	0.01	36.00	50.00	50.00	50.00		
GR7130.	0.01 4.0	7128.80	8.00	7128.90	28.00	7130.80	36.00	
X1410	4.0	0.01	36.00	20.00	20.00	20.00		
GR7129.	8 0.01	7127.80	8.00	7127.80	28.00	7129.80	36.00	
X1 3230	4.0	0.01	36.00	20.00	20.00	7129.80 20.00 7129.16 20.00		
GR7129.	2 0.01	7127.16	8.00	7127.16	28.00	7129.16	36.00	
X1 3210	4.0	0.01	36.00	20.00	20.00	20.00	•	
GR7128.	, 5 0.01	7126.52	8.00	7126:52	28.00	20.00 7128.52 20.00	36.00	
X1 3190 X2 181.	4.0	0.01	36.00	20.00	20.00	20.00		
GR7127.	9 0.01	7125.88	8.00	7125.88	28.00	7127.88 20.00 7127.24 20.00	36.00	
X1 3170	4.0	0.01	36.00	20.00	20.00	20.00		
GR7127.	0.01	7125.24	8.00	7125.24	28.00	7127.24	36.00	•
X1 3150 X2 229.	4.0 a	0.01	36.00	20.00	20.00	20.00		
GR7126.	6 0.01	7124.60	8.00	7124.60		7126.60	36.00	
X1 3130 X2 253.	4.0	0.01	36.00	20.00	20.00	20.00		
GR7126.	0.01	7123.96				7125.96		
X1 3110 X2 277.		0.01	36.00	10.00	10.00	10.00		
GR7125.	0.01					7125.32	36.00	
X13100 X2 289.		0.01	36.00	100.00	100.00	100.00		
GR7125.	0.01					7125.00	36.00	
X13000 X2 289.		0.01	36.00	100.00	100.00	100.00		
GR7123.		7121.20 0.01	8.00				36.00	
X1 2900 X2 289.		0.01	36.00	100.00	100.00	100.00		
GR7121.	7 0.01	7119.67	8.00	7119.67 100.00	28.00	7121.67	36.00	
X1 2800 X2 289.		0.01	36.00	100.00	100.00	100.00		
GR7120.		7118.13		7118.13			36.00	
X2 289.		5.01	23.00	100.00	100.00	100.00		

TABLE F-4. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL D (continued).

GR7118.6 X1 2600	4.0 0.01				7118.60 100.00	36.00
X2 289.0 GR7117.8 X12500	0.01 7115.75 4.0 0.01	19.00 58.00		39.00 100.00	7117.75 100.00	47.00
X2 289.0 GR7116.9 X1411	0.01 7114.90 4.0 0.01	30.00 58.00	7114.90 100.00		7116.90 100.00	58.00
X2 289.0 GR7116.8 X1 2300 X2 432.5	0.01 7114.80 4.0 0.01		7114.80		7116.80	58.00
GR7116.7 X1 2200 X2 576.0	0.01 7114.67 4.0 0.01		7114.67 100.00			58.00
GR7116.5 X1 2100 X2 719.5	0.01 7114.55 4.0 0.01	30.00 58.00	7114.55 100.00	50.00 100.00	7116.55	58.00
GR7116.4 X12000 X2 863.0	0.01 7114.42 4.0 0.01	30.00 58.00	7114.42		7116.42	58.00
GR7116.3 X1 1900 X2 819.0	0.01 7114.30 4.0 0.01				7116.30 50.00	58.00
GR7116.1 X11850 X2 797.0	0.01 7114.10 4.0 0.01		7114.10 50.00		7116.10 50.00	43.33
GR7116.0 X11800 X2 797.0		36.00	100.00	100.00	7116.00 100.00	36.00
GR7115.6 X11700 X2 797.0	4.0 0.01					36.00
GR7114.7 X1 1680 X2 797.0	4.0 0.01					36.00
GR7114.5 X1 1660 X2 797.0	0.01 7112.50 4.0 0.01	36.80	7112.50	20.00	7114.50 20.00	36,40
GR7114.3 X1 1640 X2 797.0	0.01 7112.30 4.0 0.01	37.20		28.80	7114.30 20.00	36.80
GR7114.1 X1 1620 X2 797.0	0.01 7112.10 4.0 0.01	37.60	7112.10 20.00	20.00	7114.10 20.00	37.20
GR7113.9 X11600 X2 797.0	0.01 7111.90 4.0 0.01	38.00		20.00		37.60
GR7113.7 X1 1580 X2 797.0	0.01 7111.70 4.0 0.01	39.30	7111.70	20.00	7113.70	38.00
GR7113.4 X1 1560 X2 797.0	4.0 0.01			20.00	7113.43	39.30
GR7113.2 X1 1540 X2 797.0	4.0 0.01	41.90	20.00	20.00	7113.16	40.60
GR7112.9 X1 1520 X2 797.0	0.01 7110.89 4.0 0.01	43.20	7110.89 20.00	20.00		
GR7112.6 X1 1500 X2 797.0	0.01 7110.62 4.0 0.01	44.50	7110.62 20.00	20.00	7112.62 20.00	43.20
GR7112.4 X1 1480 X2 797.0	0.01 7110.35 4.0 0.01	45.80	7110.35	20.00	7112.35	44.50
GR7112.1 X1 1460 X2 797.0	0.01 7110.08 4.0 0.01	47.10	7110.08	20.00	7112.08	45.80
GR7111.8 X1 1440 X2 797.0	0.01 7109.81 4.0 0.01	48.40	7109.81 20.00	20.00	7111.81 20.00	47.10
GR7111.5 X1 1420 X2 797.0		49.70	7109.54 20.00	20.00	7111.54 20.00	48.40
GR7111.3 X11400 X2 797.0	0.01 7109.27 4.0 0.01	51.00	7109.27	20.00	7111.27 20.00	49.70
GR7111.0 X1 1380	0.01 7109.00 4.0 0.01	8.00 52.00	7109.00	43.00	7111.00 20.00	51.00

TABLE F-4. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL D (continued).

X2 797.0 GR7110.8		7108.76		7108.76		7110.76	52.00
X1 1360	4.0	0.01	53.00	20.00	20.00	20.00	
X2 797.0	0.01	7100 52	0.00	7100 50	45.00	7110 52	E2 00
GR7110.5 X1 1340		7108.52	54.00	7108.52 20.00	20.00	7110.52	53.00
X2 797.0	4.0	0.01	34.00	20.00	20.00	20.00	
GR7110.3	0.01	7108.28	B 00	7108.28	46.00	7110.28	54.00
X1 1320	4.0		55.00				31.00
X2 797.0		*.**	00.00	20110			
GR7110.0	0.01	7108.04	8.00	7108.04	47.00	7110.04	55.00
X11300	4.0		56.00				
X2 797.0							
GR7109.8	0.01	7107.80	8.00	7107.80	48.00	7109.80	56.00
X1 1280	4.0	0.01	58.00	20.00	20.00	20.00	
X2 797.0							
GR7109.6		7107.64		7107.64		7109.64	58.00
X1 1260	4.0	0.01	60.00	20.00	20.00	20.00	
X2 797.0		21.00 10		2102 10	50.00	~. ~ ~ . ~	
GR7109.5				7107.48		7109.48	60.00
X1 1240	4.0	0.01	62.00	20.00	20.00	20.00	
X2 797.0 GR7109.3	0 01	7107.32	6 00	7107.32	E4 00	7109.32	62.00
X1 1220		0.01					62.00
X2 797.0	4.0	0.01	04.00	20.00	20.00	20.00	
GR7109.2	0.01	7107.16	8.00	7107.16	56.00	7109.16	64.00
X11200		0.01					01.00
X2 797.0	•••	0.01	00.00	100.00	100.00	100.00	
GR7109.0	0.01	7107.00	8.00	7107.00	58.00	7109.00	66.00
X11100	4.0			100.00			
X2 850.0			*****	•			
GR7108.1	0.01	7106.10	8.00	7106.10	58.00	7108.10	66.00
X1 1000	4.0	0.01	66.00	100.00		100.00	
X2 875.0							
GR7107.2	0.01	7105.15	8.00	7105.15	58.00	7107.15	66.00
X1900	4.0	0.01	66.00	100.00	100.00	100.00	
X2.900.0							
GR7106.2		7104.20		7104.20		7106.20	66.00
X1 800	4.0	0.01	66.00	100.00	100.00	100.00	
X2 950.0							
GR7105.5		7103.55		7103.55		7105.55	66.00
X1700	4.0	0.01	66.00	100.00	100.00	100.00	
X21000.0	0 01	2102 00	0 00	7100 00	50.00	7104 00	66.00
GR7104.9 X1600		7102.90		7102.90		7104.90	66.00
X21100.0	4.0	0.01	66.00	20.00	20.00	20.00	
GR7102.5	6 01	7100.50	9 00	7100.50	50 00	7102.50	66.00
X1 580	4.0			20.00		20.00	00.00
X21110.0	4.0	0.01	00.00	20.00	20.00	20.00	
GR7101.9	0.01	7099.86	8.00	7099.86	58.00	7101.86	66.00
X1 560	4.0		66.00		20.00		
X21120.0							
GR7101.2	0.01	7099.22	8.00	7099.22	58.00	7101.22	66.00
X1 540	4.0				20.00		
X21130.0							
GR7100.6	0.01	7098.58	8.00	7098.58	58.00	7100.58	66.00
X1 520	4.0	0.01	66.00	20.00	20.00	20.00	
X21140.0	_						
GR7099.9		7097.94		7097.94		7099.94	66.00
X1 500	4.0	0.01	66.00	20.00	20.00	20.00	
X21150.0		7007 70		2002 20	F0 00		
GR7099.3 X1 480		7097.30		7097.30		7099.30	66.00
X21160.0	4.0	0.01	66.00	20.00	20.00	20.00	
GR7098.7	0.01	7096.66	8 00	7096.66	58 00	7098.66	66.00
X1 460	4.0	0.01	66.00	20.00	20.00	20.00	00.00
X21170.0	0	0.01	55.50	20.00	20.00	20.00	
GR7098.0	0.01	7096.02	8.00	7096.02	58 00	7098.02	66.00
X1 440	4.0	0.01	66.00	20.00	20.00	20.00	55.00
X21180.0							
GR7097.4	0.01	7095.38	8.00	7095.38	58.00	7097.38	66.00
X1 420	4.0	0.01	66.00	20.00	20.00	20.00	
X21190.0	· ·						
GR7096.7	0.01	7094.74	8.00	7094.74	58.00	7096.74	66.00
X1400	4.0	0.01	66.00	20.00	20.00	20.00	
X21200.0							
GR7096.1	0.01	7094.10		7094.10	58.00	7096.10	66.00
GR7096.1 X1 380		7094.10 0.01	8.00 66.00	7094.10 20.00	58.00 20.00	7096.10 20.00	66.00
GR7096.1	0.01		66.00		20.00		66.00

TABLE F-4. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL D (continued).

X1 360	4.0	0.01	66.00	20.00	20.00	20.00	
X21220.0							
GR7094.5	0.01	7092.54	8.00	7092.54	58.00	7094.54	66.00
X1 340	4.0	0.01	66.00	20.00	20.00	20.00	
X21230.0							
GR7093.8							66.00
X1 320	4.0	0.01	66.00	20.00	20.00	20.00	
X21240.0							
GR7093.0							66.00
X1300	4.0	0.01	66.00	20.00	20.00	20.00	
X21250.0							
GR7092.2	0.01	7090.20	8.00	7090.20	58.00	7092.20	66.00
X1 280	4.0	0.01	66.00	20.00	20.00	20.00	
X21261.8							
GR7091.2							66.00
X1 260	4.0	0.01	66.00	20.00	20.00	20.00	
X21273.5							
GR7090.1							66.00
X1 240	4.0	0.01	66.00	20.00	20.00	20.00	
X21285.3					•		
GR7089.1	0.01	7087.09	8.00	7087.09	58.00	7089.09	66.00
X1 220	4.0	0.01	66.00	5.00	5.00	5.00	
X21297.1							
GR7088.1							66.00
X1413	4.0	0.01	66.00	15.00	15.00	15.00	
X21300.0							
GR7087.8							66.00
X1200	4.0	0.01	66.00	50.00	50.00	50.00	
X21300.0							
GR7087.0							66.00
	4.0	0.01	66.00	50.00	50.00	50.00	
X21325.0							
GR7085.5				7083.50	58.00	7085.50	66.00
X1100	4.0	0.01	66.00				
X21325.0							
GR7109.8	0.01	7107.80	8.00	7107.80	58.00	7109.80	66.00
EJ							
ER							

TABLE F-5. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL H.

	PATHFINE CHANNEL	ER SHIRLE	Y BASIN	TAILINGS	RECLAM	ATION		
тз		BCRITICAL					620.0	6065.0
J1 J2 -1	c	-1.0		-1.0			620.0	6965.0
NC 0.035				0.3				
X140 X2 620.0	4.0		72.00					
GR6967.6	0.01	6963.60					72.00	
X150 X2 620.0	1	0.01						
GR6967.7	0.01	6963.70	16.00	6963.70	56.00	6967.70	72.00	
		0.01	72.00	50.00	50.00	50.00		
X2 620.0 GR6967.8	0.01	6963.80	16.00	6963.80	56.00	6967.80	72.00	
XII3/	4.0	0.01	72.00	57.00	57.00	57.00		
X2 620.0	0.01	6964.10	16 00	6964 10	56 00	6969 10	72.00	
X1165	4.0	0.01	72.00	8.00	8.00	8.00	72.00	
X2 620.0				•				
X1175	4.0	6965.10 0.01	72.00	10.00	10.00	10.00	72.00	
X2 620.0								
GR6970.4	0.01	6966.40 0.01	16.00	6966.40	56.00	6970.40	72.00	
X2 620.0								
	0.01	6967.50	12.00	6967.50	52.00	6970.50	64.00	
X1193 X2 620.0		0.01	64.00	10.00	10.00	10.00		
		6968.80	12.00	6968.80	52.00	6971.80	64.00	
X1203 X2 620.0	4.0	0.01	64.00	10.00	10.00	10.00		
		6970.10	12.00	6970.10	52.00	6973.10	64.00	
VI512	4.0	0.01	64.00	10.00	10.00	10.00		
X2 620.0	0.01	6971.40	12.00	6973 40	F2 00	6974 40	64.00	
X1223		0.01					64.00	
X2 620.0								
X1233	4.0	6972.70 0.01	12.00	10.00	52.00	10.00	64.00	
X2 620.0								
GR6976.0 X1240	0.01	6974.00 0,01	8.00	6974.00	48.00	6976.00	56.00	
V2 620 0						7.00		
GR6977.0	0.01	6975.00 0.01	8.00	6975.00	48.00	6977.00	56.00	
X2 620.0	4.0	0.01	56.00	20.00	20.00	20.00		
GR6979.8	0.01	6977.83 0.01	8.00	6977.83	48.00	6979.83	56.00	
X1 280 X2 620.0	4.0	0.01	56.00	20.00	20.00	20.00		
GR6982.7	0.01	6980.67	8.00	6980.67	48.00	6982.67	56.00	
X1300	4.0	0.01	56.00	20.00	20.00	20.00		
X2 620.0 GR6985.5	0.01	6983.50	8.00	6983.50	48.00	6985 50	56.00	
X1 320	4.0	6983.50 0.01	56.00	20.00	20.00	20.00	30.00	
XZ 620.0								
X1 340	4.0	6986.10 0.01	56.00	20.00	20.00	20.00	30.00	•
X2 620.0 GR6990.7								•
X1 360		6988.70 0.01		20.00			56.00	
X2 620.0								
GR6993.3 X1 380		6991.30 0.01	8.00	6991.30	48.00	6993.30	56.00	
X2 620.0								
GR6995.9 X1 400	0.01	6993.90 0.01	8.00	6993.90	48.00	6995.90	56.00	
X2 620.0								
GR6998.5	0.01	6996.50 0.01	8.00	6996.50	48.00	6998.50	56.00	
X1 420 X2 620.0	4.0	0.01	56.00	20.00	20.00	20.00		
GR7001.1	0.01	6999.10	8.00	6999.10	48.00	7001.10	56.00	
X1 440 X2 620.0	4.0	0.01	56.00	20.00	20.00	20.00		
GR7003.7	0.01	7001.70	8.00	7001.70	48.00	7003.70	56.00	
X1 460	4.0	7001.70 0.01	56.00	20.00	20.00	20.00	20.00	
X2 620.0	0.01	7004.30	8 00	7004 30	48 00	7006 30	56.00	
X1 480	4.0	0.01	56.00	20.00	20.00	20.00	30.00	
X2 620.0								

TABLE F-5. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL H (continued). 0.01 7006.90 GR7008.9 8.00 7006.90 48.00 7008.90 4.0 0.01 56.00 20.00 20.00 20.00 X1500 X2 620.0 0.01 7009.50 GR7011.5 8.00 7009.50 48.00 7011.50 56.00 56.00 20.00 20.00 20.00 4.0 0.01 X1 520 X2 620.0 0.01 7012.15 8.00 7012.15 48.00 7014.15 56,00 GR7014.1 4.0 0.01 56.00 20.00 20.00 20.00 X1 540 X2 620.0 8.00 7014.80 48.00 7016.80 0.01 7014.80 GR7016.8 56.00 56.00 20.00 20.00 20.00 X1 560 4.0 0.01 X2 620.0 0.01 7017.45 8.00 7017.45 48.00 7019.45 56.00 GR7019.5 4.0 0.01 56.00 20.00 20.00 20.00 X1 580 X2 620.0 48.00 7022,10 0.01 7020.10 8.00 7020.10 56.00 GR7022.1 56.00 20.00 20.00 20,00 4.0 0.01 X1 600 X2 620.0 GR7024.8 0.01 7022.75 8.00 7022.75 48.00 7024.75 56.00 4.0 0.01 56.00 20.00 20.00 20.00 X1 620 X2 620.0 0.01 7025.40 8.00 7025.40 48.00 7027.40 56.00 GR7027.4 4.0 0.01 56.00 20.00 20.00 20.00 X1 640 X2 620.0 GR7030.0 0.01 7028.05 8.00 7028.05 48.00 7030.05 56.00 X1 660 4.0 0.01 56.00 20.00 20.00 20.00 X2 620.0 0.01 7030.70 8.00 7030.70 48.00 7032.70 56.00 GR7032.7 4.0 0.01 56.00 20.00 20.00 20.00 X1 680 X2 620.0 GR7035.4 0.01 7033.35 8.00 7033.35 48.00 7035.35 56.00 0.01 X1700 4.0 56.00 20.00 20.00 20.00 X2 620.0 GR7038.0 0.01 7036.00 8.00 7036.00 48.00 7038.00 56.00 . X1 720 4.0 0.01 56.00 20.00 20.00 20.00 X2 620.0 GR7040.6 0.01 7038.60 8.00 7038.60 48.00 7040.60 56.00 4.0 0.01 56.00 20.00 20.00 20.00 X1 740 X2 620.0 GR7043.2 0.01 7041.20 8.00 7041.20 48.00 7043.20 56.00 X1 760 4.0 0.01 56.00 20.00 20.00 20.00 X2 620.0 0.01 7043.80 8.00 7043.80 48.00 7045.80 56.00 GR7045.8 20.00 20.00 X1 780 4.0 0.01 56.00 20.00 X2 620.0 48.00 7048.40 0.01 7046.40 8.00 7046.40 GR7048.4 56.00 20.00 20.00 56.00 20.00 X1 800 4.0 0.01 X2 620.0 8.00 7049.00 48.00 7051.00 0.01 7049.00 56.00 GR7051.0 56.00 20.00 20.00 20.00 X1 820 4.0 0.01 X2 620.0 GR7053.6 0.01 7051.60 8.00 7051.60 48.00 7053.60 56.00 X1 840 4.0 0.01 56.00 20.00 20.00 20.00 X2 620.0 0.01 7054.20 8.00 7054.20 48.00 7056.20 GR7056.2 56.00 4.0 0.01 56.00 20.00 20.00 20.00 X1 860 X2 620.0 GR7058.8 0.01 7056.80 8.00 7056.80 48.00 7058.80 56.00 X1 880 4.0 0.01 56.00 20.00 20.00 20.00 X2 620.0 8.00 7059.40 0.01 7059.40 48.00 7061.40 56.00 GR7061.4 4.0 0.01 56.00 20.00 20.00 20.00 X1900 X2 620.0 GR7064.0 0.01 7062.00 8.00 7062.00 48.00 7064.00 56.00 20.00 20.00 X1 920 4.0 0.01 56.00 20.00 X2 620.0 48.00 7066.70 GR7066.7 0.01 7064.70 8.00 7064.70 56.00 20.00 20.00 X1 940 4.0 0.01 56.00 20.00 X2 620.0 0.01 7067.40 8.00 7067.40 46.00 7069.40 GR7069.4 56.00 20.00 20.00 X1 960 4 - 0 0.01 56.00 20.00 X2 620.0 0.01 7070.10 8.00 7070.10 48.00 7072.10 56.00 GR7072.1 4.0 0.01 56.00 20.00 20.00 20.00 X1 980 X2 620.0 0.01 7072.80 8.00 7072.80 48.00 7074.80 GR7074.8 56.00 56.00 20.00 20.00 20.00 X11000 4.0 0.01 X2 620.0 GR7077.5 0.01 7075.50 8.00 7075.50 48.00 7077.50 0.01 104.00 10.00 10.00 10.00 X11010

TABLE F-5. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL H (continued).

X2 620.0 GR7085.0	0.01	7077.00	32.00	7077.00	72.00	7085.00	104.00
X11030	4.0			20.00			201.00
X2 620.0 GR7086.5	0.01	7078.50	32 00	7078 50	72 00	7086.50	104.00
X11050	4.0			20.00		20.00	104.00
X2 620.0		7070 00	20.00		30.00	3003.00	
GR7087.0 X11100		0.01				7087.00 50.00	104.00
X2 620.0							
GR7087.4 X11200		0.01				7087.40	104.00
X2 620.0	1.0	0.01	104.00	100.00	100.00	100.00	
GR7087.6		7079.60		7079.60		7087.60	104.00
X1 1220 X2 620.0	4.0	0.01	102.75	20.00	20.00	20.00	
GR7087.7		7079.70		7079.70		7087.70	102.75
X1 1240 X2 620.0	4.0	0.01	101.50	20.00	20.00	20.00	
GR7087.8	0.01	7079.80	32.00	7079.80	69.50	7087.80	101.50
X1 1260	4.0	0.01	100.25	20.00	20.00	20.00	
X2 620.0 GR7087.9	0.01	7079.90	32.00	7079.90	68.25	7087.90	100.25
X11280		0.01		20.00		20.00	100.10
X2 620.0 GR7088.0	0.01	7080.00	32 00	7000 00	67.00	7088.00	99.00
X1 1282		0.01					99.00
X2 620.0							
GR7088.0 X1 1285		7080.04				7088.04	97.70
X2 620.0	4.0	0.01	20.75	3.00	3.00	3.00	
GR7088.1		7080.10	31.00	7080.10		7088.10	95.75
X1 1288 X2 620.0	4.0	0.01	93.80	3.00	3.00	3.00	
GR7088.2		7080.16		7080.16			93.80
X1 1291 X2 620.0	4.0	0.01	91.85	3.00	3.00	3.00	
GR7088.2	0.01	7080.22	29.80	7080.22	62.05	7088.22	91.85
X1 1294	4.0						
X2 620.0 GR7088.3	0.01	7080.28	29.20	7080.28	60.70	7088.28	89.90
X1 1297		0.01					05.50
X2 620.0 GR7088.3	2.01	7000 24	20.50	2000 21		3000 34	
X11300		7080.34 0.01		7080.34 3.00	3.00	7088.34	87.95
X2 620.0							
GR7088.4 X1 1303		7080.40		7080.40		7088.40	86.00
X2 620.0							
GR7088.4 X1 1306		7080.41		7080.41	56.95	7088.41	84.65
X2 620.0	4.0	0.01	83.30	3.00	3.00	3.00	
GR7088.4	0.01	7080.42	27.40			7088.42	83.30
X1 1309 X2 620.0	4.0	0.01	81.95	3.00	3.00	3.00	
GR7088.4	0.01	7080.44	27.10	7080.44	54.85	7088.44	81.95
X1 1312 X2 620.0	4.0	0.01	80.60	3.00	3.00	3.00	
GR7088.4	0.01	7080.45	26.80	7080.45	53.80	7088.45	80.60
X1 1315		0.01		3.00			
X2 620.0 GR7088.5	0.01	7080 46	26 50	7090 46	52 75	7000 46	70.25
X1 1318	4.0	7080.46 0.01	77.90	3.00	3.00	3.00	79.25
X2 620.0							
GR7088.5 X1 1321	4.0	7080.48	76.55	7080.48	3.00	3.00	77.90
X2 620.0							
GR7088.5 X1 1324	0.01	7080.49	25.90 75.20	7080.49			76.55
X2 620.0				3.00		3.00	
GR7088.5	0.01	7080.50	25.60			7088.50	75.20
X1 1327 X2 620.0	4.0	0.01	73.85	3.00	3.00	3.00	
GR7088.5		7080.51	25.30	7080.51	48.55	7088.51	73.85
X1 1330 X2 620.0	4.0	0.01	72.50	3.00	3.00	3.00	
GR7088.5	0.01	7080.52	25.00	7080.52	47.50	7088.52	72.50
X1 1333	4.0	0.01		3.00		3.00	•
X2 620.0 GR7088.5	0.01	7080.54	24.70	7080.54	46 45	7088 54	71.15
						. 555154	

TABLE F-5. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL H (continued). X1 1336 4.0 0.01 69.80 3.00 3.00 3.00 X2 620.0 0.01 7080.55 GR7088.5 24.40 7080.55 45.40 7088.55 69.80 X1 1339 4.0 0.01 68.45 3.00 3.00 3,00 X2 620.0 0.01 7080.56 GR7088.6 24.10 7080.56 44.35 7088.56 68.45 X1 1342 4.0 0.01 67.10 3.00 3.00 3.00 X2 620.0 GR7088.6 0.01 7080.57 23.80 7080.57 43.30 7088.57 67.10 4.0 0.01 65.75 X1 1345 3.00 3.00 3.00 X2 620.0 GR7086.6 0.01 7080.59 23.50 7080.59 42.25 7088.59 65.75 3.00 3.00 X1 1348 X2 620.0 4.0 0.01 64.40 3.00 0.01 7080.60 23.20 7080.60 GR7088.6 41.20 7088.60 64.40 4.0 0.01 63.05 X1 1351 3.00 3.00 X2 620.0 0.01 7080.61 22,90 7080.61 40.15 7088.61 GR7088.6 63.05 X1 1354 4.0 0.01 61.70 3.00 3.00 3.00 X2 620.0 0.01 7080.63 GR7088.6 22.60 7080.63 39.10 7088.63 61.70 4.0 0.01 X1 1357 60.35 3.00 3.00 3.00 X2 620.0 0.01 7080.64 22.30 7080.64 38.05 7088.64 GR7088.6 60.35 0.01 X1 1360 59.00 4.0 3.00 3.00 3.00 X2 620.0 0.01 7080.65 22.00 7080.65 37.00 7088.65 GR7088.6 59.00 X1 1363 3.00 3.00 4.0 0.01 57.65 3.00 X2 620.0 0.01 7080.66 21.70 7080.66 35.95 7088.66 GR7088.7 57.65 X1 1366 4.0 0.01 56.30 3.00 3.00 3.00 X2 620.0 GR7088.7 0.01 7080.67 21.40 7080.67 34.90 7088.67 56.30 X1 1369 4.0 0.01 54.95 3.00 3.00 3.00 X2 620.0 0.01 7080.69 21.10 7080.69 33.85 7088.69 GR7088.7 54.95 X1 1372 0.01 53.60 3.00 3.00 4.0 3.00 X2 620.0 0.01 7080.70 GR7088.7 20.80 7080.70 32.80 7088.70 53.60 3.00 0.01 X1 1375 4.0 52.25 3.00 3.00 X2 620.0 GR7088.7 0.01 7080.71 20.50 7080.71 31.75 7088.71 52.25 X1 1378 4.0 0.01 50.90 3.00 3.00 3.00 X2 620.0 GR7088.7 0.01 7080.73 20.20 7080.73 30.70 7088.73 50.90 0.01 X1 1381 49.55 3.00 3.00 4.0 3.00 X2 620.0 GR7088.7 0.01 7080.74 19.90 7080.74 29.65 7086.74 X1 1384 4.0 0.01 48.20 3.00 3.00 3.00 X2 620.0 0.01 7080.75 19.60 7080.75 28.60 7088.75 GR7088.8 48.20 X1 1387 0.01 46.85 4.0 3.00 3.00 3.00 X2 620.0 GR7088.8 0.01 7080.76 19.30 7080.76 27.55 7088.76 46.85 X1 1390 4.0 0.01 45.50 3.00 3.00 3.00 X2 620.0 0.01 7080,77 19.00 7080.77 26.50 7088.77 GR7088.8 45.50 X1 1393 4.0 0.01 44.15 3.00 3.00 3.00 X2 620.0 GR7088.8 0.01 7080.79 18.70 7080.79 25.45 7088.79 44.15 X1 1396 4.0 0.01 42.80 3.00 3.00 3.00 X2 620.0 GR7088.8 0.01 7080.80 18.40 7080.80 24.40 7088.80 42.80 X1 1399 X2 620.0 0.01 3.00 3.00 4.0 41.45 -3.00 GR7088.8 0.01 7080.81 18.10 7080.81 23.35 7088.81 41.45 X1 1402 4.0 0.01 40.10 3.00 3.00 3,00 X2 620.0 GR7088.8 0.01 7080.82 17.80 7080.82 22.30 7088.82 40.10 X1 1405 4.0 0.01 38.75 3.00 3.00 3.00 X2 620.0 GR7088.8 0.01 7080.84 17.50 7080.84 21.25 7088.84 38.75 X1 1408 4.0 0.01 37.40 3.00 3.00 3.00 X2 620.0 GR7088.9 0.01 7080.85 17.20 7080.85 20.20 7088.85 37.40 3.00 X1 1411 4.0 0.01 36.05 3.00 3.00 X2 620.0 GR7088.9 0.01 7080.86 16.90 7080.86 19.15 7088.86 36.05 X1 1414 4.0 0.01 34.70 3.00 3.00

X2 620.0

TABLE F-5. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL H (continued).

GR7086.9 X1 1417		7080.88	16.60 33.35			7088.88 3.00	34.70
X2 620.0 GR7088.9 X11420	0.01 4.0		16.30 32.00			7088.89 3.00	33.35
X2 620.0 GR7088.9 X11	0.01 4.0	7080.90 0.01	16.00 32.00	7080.90 40.00		7088.90 40.00	32.00
X2 620.0 GR7089.0 X11490 X2 620.0	0.01 4.0	7081.00 0.01		7081.00 30.00		7089.00 30.00	32.00
GR7089.0 X11500 X2 620.0		7081.00		7081.00 10.00		7089.00 10.00	32.00
GR7089.0 X1 1501 X2 620.0	0.01 4.0	7081.00 0.01	16.00 37.67	7081.00 1.00	21.00	7089.00	37.00
GR7089.0 X1 1504 X2 620.0	0.01 4.0	7081.00	16.16 39.68	7081.00 3.00	21.51 3.00	7089.00 3.00	37.67
GR7089.0 X1 1507 X2 620.0	0.01 4.0	70B1.00 0.01	16.64 41.69	7081.00 3.00		7089.00 3.00	39.68
GR7089.0 X1 1510 X2 620.0	4.0		43.70		3.00	3.00	41.69
GR7089.0 X1 1513 X2 620.0	4.0		45.71		3.00		43.70
GR7089.0 X1 1516 X2 620.0	4.0		47.72		3.00		45.71
GR7089.0 X1 1519 X2 620.0	4.0	7081.02	49.73		3.00	7089.02	47.72
GR7089.0 X1 1522 X2 620.0 GR7089.0	4.0	7081.02 0.01 7081.02 0.01	51.74		3.00	7089.02 3.00 7089.02	49.73
X1 1525 X2 620.0 GR7089.0		0.01 7081.02		3.00 7081.02	3.00		53.75
X1 1528 X2 620.0 GR7089.0	4.0	0.01	55.76		3.00		55.76
X1 1531 X2 620.0 GR7089.0	4.0		57.77		3.00		57,77
X1 1534 X2 620.0 GR7089.0	0.01	0.01	59.78 21.44	3.00 7081.03	3.00		59.78
X1 1537 X2 620.0 GR7089.0	4.0 0.01	0.01 7081.04	61.79 21.92	3.00 7081.04	3.00		61.79
X1 1540 X2 620.0 GR7089.0	4.0 0.01	0.01	63.80	3.00	3.00	3.00 7089.04	63.80
X1 1543 X2 620.0 GR7089.0	0.01	7081.04	22.88	7081.04	42.93	3.00 7089.04	65.81
X1 1546 X2 620.0 GR7089.0 X1 1549	4.0 0.01 4.0	0.01 7081.05 0.01	67.82 23.36 69.83	3.00 7081.05 3.00	3.00 44.46 3.00	7089.05	67.82
X2 620.0 GR7089.0 X1 1552		7081.05		7081.05		3.00 7089.05 3.00	69.83
X2 620.0 GR7089.1 X1 1555		7081.05		7081.05		7089.05	71.84
X2 620.0 GR7089.1 X1 1558		7081.06		7081.06 3.00		7089.06 3.00	73.05
X2 620.0 GR7089.1 X1 1561	0.01	7081.06 0.01		7081.06		7089.06 3.00	75.86
X2 620.0 GR7089.1 X1 1564	0.01	7081.06 0.01	25.76 79.88	7081.06	52.11 3.00	7089.06 3.00	77.87

TABLE F-5. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL H (continued).

X2 620.0							
GR7089.1 X1 1567	0.01	7081.06	26.24	7081.06	53.64	7089.06	79.88
X1 1567	4.0	0.01	81.89	3.00	3.00	3.00	
X2 620.0	• • • •						
GR7089 1	0.01	7081.07	26.72	7081.07	55.17	7089.07	81.89
X1 1570	4.0	0.01	83.90	3.00	3.00	3.00	
X1 1570 X2 620.0						5.00	
GR7089.1	0.01	7081 - 07	27.20	7081.07	56.70	7089.07	83.90
X1 1573	4.0	0.01	85.91	3.00	3.00	3.00	00.50
X2 620.0				• • • •		0.00	
GR7089.1		7081.07	27 68	7081.07	58.23	7089 07	85 91
X1 1576							
X2 620.0		0.02	0.122	••••	0.00	3.00	
GR7089.1	0.01	7081.08	28.16	7081.08	59.76	7089.08	87.92
X1 1579	4.0	0.01	89.93	3.00	3,00	3.00	052
X2 620.0			02.20				
X2 620.0 GR7089.1 X1 1582	0.01	7081.08	28.64	7081.08	61.29	7089.08	89.93
X1 1582	4.0	0.01	91.94	3.00	3.00	3.00	*****
X2 620.0			,,,,,				
GR7089.1 X1 1585 X2 620.0	0.01	7081.08	29.12	7081.08	62.82	7089.08	91,94
X1 1585	4.0	0.01	93.95	3.00	3.00	3.00	22121
X2 620.0							
GR7089.1	0.01	7081.09	29.60	7081.09	64.35	7089.09	93.95
X1 1598	4.0	0.01	95.96	3.00	3.00	3.00	
X2 620.0							
GR7089.1	0.01	7081.09	30.08	7081.09	65.88	7089.09	95.96
X1 1591	4.0	0.01	97.97	3.00	3.00	3.00	
X2 620.0							
GR7089.1	0.01	7081.09	30.56	7081.09	67.41	7089.09	97.97
X1 1594	4.0	0.01	99.98	3.00	3.00	3.00	
X2 620.0							
GR7089.1	0.01	7081.09	31.04	7081.09	68.94	7089.09	99.98
X1 1597							
X2 620 0							
GR7089.1 X11600	0.01	7081.10	31.52	7081.10	70.47	7089.10	101.99
X11600	4.0	0.01	104.00	3.00	3.00	3.00	
X2 620.0							
GR7089.1 X11700	0.01	7081.10	32,00	7081.10	72.00	7089.10	104.00
X11700	4.0	0.01	104.00	100.00	100.00	100.00	
X2 620.0							
GR7089.2	0.01	7081.20	32.00	7081.20	72.00	7089.20	104.00
EJ							
ER							

TABLE F-6. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL H.

T1	PATHFIND	ER SHIRL	Y BASIN	TATI.ING	s RECLAM	ATTON		
T2	CHANNEL :	Н		17421110	,			
T3 J1	PMF - SU	PERCRITIO		-1.0			620.0	7088.2
J2 -1				0.3				
X11700	0.035 4.0		104.00		100.00	100.00		
X2 620.0)					~~~		
GR7089.2 X11600		7081.20 0.01					104.00	
X2 620.0)							
GR7089.1 X1 1597	L 0.01	7081.10 0.01	32.00	7081.10	72.00	7089.10	104.00	
X2 620.0)							
GR7089.1 X1 1594		7081.10				7089.10	101.99	
X2 620.0)							
GR7089.1 X1 1591	0.01	7081.09	31.04	7081.09	68.94	7089.09	99.98	
X2 620.0)							
GR7089.1 X1 1588		7081.09					97.97	
X2 620.0)	0.01	33.30	3.00	3.00	3.00		
GR7089,1 X1 1585	0.01	7081.09 0.01		7081.09 3.00		7089.09		
X2 620.0)							
GR7089.1	0.01	7081.09	29.60	7081.09	64.35	7089.09	93.95	
X1 1582 X2 620.0)	0.01		3.00				
GR7089.1	0.01	7081.08 0.01	29.12	7081.08	62.82	7089.08	91.94	
X1 15/9 X2 620.0)	0.01	89.93	3.00	3.00	3.00		
GR7089.1		7081.08	28.64	7081.08	61.29	7089.08	89.93	
X1 1576 X2 620.0		0.01	87.92	3.00	3.00	3.00		
GR7089.1	0.01							
X1 1573 X2 620.0		0.01	85.91	3.00	3.00	3.00		
GR7089.1		7081.07 0.01	27.68	7081.07	58.23	7089.07	85.91	
X1 1570 X2 620.0		0.01	83.90	3.00	3.00	3.00		
GR7089.1		7081.07	27.20	7081.07			83.90	
X1 1567 X2 620.0		0.01	81.89	3.00	3.00	3.00		
GR7089.1		7081.07	26.72	7081.07	55.17	7089.07	81.89	
X1 1564 X2 620.0		0.01	79.88	3.00	3.00	3.00		
	0.01	7081.06	26.24	7081.06	53.64	7089.06	79.88	
X1 1561		0.01	77.87	3.00	3.00	3.00		
X2 620.0 GR7089.1		7081.06	25.76	7081.06	52.11	7089.06	77.87	
X1 1558	4.0	0.01	75.86	3.00	3.00	3.00		
X2 620.0 GR7089.1		7081.06	25.28	7081.06	50.58	7089.06	75.86	
X1 1555	4.0	0.01	73.85	3.00	3.00	3.00		
X2 620.0 GR7089.1		7081.06	24.80	7081.06	49.05	7089.06	73.85	
X1 1552	4.0	0.01		3.00				
X2 620.0 GR7089.1		7081.05	24.32	7081.05	47.52	7089.05	71.84	
X1 1549		0.01	69.83	3.00	3.00	3.00		
X2 620.0 GR7089.0		7081.05	23.84	7081.05	45.99	7089.05	69.83	
X1 1546	4.0	0.01	67.82					
X2 620.0 GR7089.0		7081.05	23.36	7081.05	44.46	7089.05	67.82	
X1 1543	4.0				3.00			
X2 620.0 GR7089.0		7081.04	22.88	7081.04	42.93	7089.04	65.81	
X1 1540	4.0							
X2 620.0 GR7089.0		7081.04	22.40	7081.04	41.40	7089.04	63.80	
X1 1537	4.0				3.00			
X2 620.0 GR7089.0		7081.04	21.92	7081.04	39.R7	7089.04	61.79	
X1 1534	4.0				3.00	3.00		
X2 620.0 GR7089.0		7081.03	21.44	7081.03	38.34	7089.03	59.78	
X1 1531	4.0			3.00			0,,,0	
X2 620.0								

TABLE F-6. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL H (continued).

GR7089.0 X1 1528	0.01 4.0	7081.03	20.96 55.76	7081.03 3.00			57.77
X2 620.0 GR7089.0 X1 1525	0.01 4.0	7081.03	20.46 53.75	7081.03 3.00	-	7089.03 3.00	55.76
X2 620.0 GR7089.0 X1 1522	0.01	7081.02	20.00 51.74	7081.02		7089.02	53.75
X2 620.0 GR7089.0 X1 1519		7081.02	19.52 49.73	7081.02		7089.02 3.00	51.74
X2 620.0 GR7089.0 X1 1516 X2 620.0	0.01	7081.02		7081.02 3.00	30.69 3.00	7089.02	49.73
GR7089.0 X1 1513 X2 620.0	0.01	7081.02	18.56 45.71	7081.02 3.00	29.16 3.00	7089.02 3.00	47.72
GR7089.0 X1 1510 X2 620.0		7081.01	18.08 43.70	7081.01 3.00		7089.01 3.00	45.71
GR7089.0 X1 1507 X2 620.0	0.01 4.0	7081.01	17.60 41.69	7081.01 3.00	26.10 3.00	7089.01 3.00	43.70
GR7089.0 X1 1504 X2 620.0	0.01 4.0	7081.01 0.01	17.12 39.68	7081.01 3.00	24.57 3.00	7089.01 3.00	41.69
GR7089.0 X1 1501 X2 620.0	0.01 4.0	7081.00	16.64 37.67	7081.00		7089.00	39.68
GR7089.0 X11500 X2 620.0		7081.00 0.01					37.67
GR7089.0 X11490 X2 620.0		7081.00 0.01					37.00
GR7089.0 X11 X2 620.0		7081.00 0.01		7081.00 40.00			32.00
GR7089.0 X11420 X2 620.0	0.01 4.0	7081.00	16.00 32.00	7081.00 3.00		7089.00 3.00	32.00
GR7088.9 X1 1417 X2 620.0		7080.90 0.01		7080.90 3.00		7088.90 3.00	32.00
GR7088.9 X1 1414 X2 620.0	0.01 4.0	7080.89 0.01	16.30 34.70	7080.89 3.00		7088.89 3.00	33.35
GR7088.9 X1 1411 X2 620.0	0.01 4.0	7080.88	36.05		3.00	7088.88 3.00	34.70
GR7088.9 X1 1408 X2 620.0	4.0	7090.86	37.40	7080.86 3.00	3.00	3.00	36.05
GR7088.9 X1 1405 X2 620.0	4.0	7080.85	38.75	7080.85	3.00	3.00	37.40
GR7088.8 X1 1402 X2 620.0		7060.84					38.75
GR7088.8 X1 1399 X2 620.0	4.0	7080.82	41.45	7080.82	3.00	7088.82	40.10
GR7088.8 X1 1396 X2 620.0	4.0	7080.81	42.80	7080.81	3.00	7088.81	41.45
GR7088.8 X1 1393 X2 620.0	4.0	7080.80	44.15	7080.80	3.00	7088.80	42.80
GR7088.8 X1 1390 X2 620.0	4.0	7080.79	45.50	7080.79	3.00	7088.79 3.00	44.15
GR7088.8 X1 1387 X2 620.0	4.0	0.01	46.85	7080.77	3.00	7088.77	45.50
GR7088.8 X1 1384 X2 620.0	4.0	7080.76	48.20	7080.76	3.00	7088.76	46.85
GR7088.8 X1 1381	4.0	7080.75 0.01	19.60 49.55	7080.75 3.00	28.60 3.00	7088.75 3.00	48.20

TABLE F-6. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL H (continued).

X2 620.0 GR7088.7	0.01	7080.74	19.90	7080.74	29.65	7088.74	49.55
X1 1378	4.0	0.01	50.90	3.00	3.00	3.00	
X2 620.0				2000 73	20.20	2000 22	E0 00
GR7088.7	0.01	7080.73	20.20	7080.73 3.00	30.70	7088.73 3.00	50.90
X1 1375	4.0	0.01	52.25	3.00	3.00	3.00	
X2 620.0	0 01	7000 71	20 50	7000 71	21 75	7000 71	E2 25
GR7088.7		7080.71		7080.71 3.00		7088.71	52.25
X1 1372	4.0	0.01	53.60	3.00	3.00	3.00	
X2 620.0 GR7088.7	0 01	7080.70	20 00	7080.70	32 80	7088.70	53.60
X1 1369		0.01	54.95				33.00
X2 620.0	4.0	0.01	34.33	3.00	3.00	5.00	
GR7088.7	0.01	7080.69	21 10	7080.69	33 85	7088.69	54.95
X1 1366		0.01		3.00		3.00	34.33
X2 620.0	1.0	0.01	50.50	2.00	3.00	5.00	
GR7088.7	0.01	7080.67	21.40	7080.67	34.90	7088.67	56.30
X1 1363	4.0	0.01	57.65	3.00	3.00	3.00	
X2 620.0						-	
GR7088.7	0.01	7080.66	21.70	7080.66	35.95	7088.66	57.65
X1 1360		0.01				3.00	
X2 620.0							
GR7088.6	0.01	7080.65	22.00	7080.65	37.00	7088.65	59.00
X1 1357	4.0	7080.65 0.01	60.35	3.00	3.00	3.00	
X2 620.0							
GR7088.6	0.01	7080.64	22.30	7080.64	38.05	7088.64	60.35
X1 1354	4.0	7080.64 0.01	61.70	3.00	3.00	3.00	
X2 620.0		-					
GR7088.6	0.01	7080.63	22.60	7080.63	39.10	7088.63	61.70
X1 1351		0.01		3.00			
X2 620.0							
GR7086.6	0.01	7080.61	22.90	7080.61	40.15	7088.61	63.05
X1 1348	4.0	0.01	64.40				
X2 620.0							
GR7088.6	0.01	7080.60 0.01	23.20	7080.60	41.20	7088.60	64.40
X1 1345	4.0	0.01	65.75	3.00	3.00	3.00	
X2 620.0							
GR7088.6		7080.59		7080.59		7088.59	65.75
X1 1342	4.0	0.01	67.10	3.00	3.00	3.00	
X2 620.0							
GR7088.6	0.01	7080.57					67.10
X1 1339	4.0	0.01	68.45	3.00	3.00	3.00	
X2 620.0							
GR7088.6		7080.56		7080.56			68.45
X1 1336	4.0	0.01	69.80	3.00	3.00	3.00	
X2 620.0							
GR7088.5	0.01	7080.55	24.40	7080.55	45.40	7088.55	69.80
X1 1333	4.0	0.01	71.15	3.00	3.00	3.00	
X2 620.0							
GR7088.5		7080.54		7080.54			71.15
X1 1330	4.0	0.01	72.50	3.00	3.00	3.00	
X2 620.0	^ ^	7000 50	25 22	2000 50		7000 50	70 50
GR7088.5 X1 1327		7080.52 0.01		7080.52			72.50
	4.0	0.01	73.85	3.00	3.00	3.00	
X2 620.0 GR7088.5	0.01	7080.51	25 30	7080.51	10 EE	7088.51	73.85
X1 1324	4.0						, 5.05
X2 620.0	4.0	0.01		3.00	3.00	3.00	
GR7088.5	0.01	7080.50	25.60	7080.50	49.60	7088.50	75.20
X1 1321	4.0		76.55		3.00		
X2 620.0			3.00	2	2.00	5.05	
GR7088.5	0.01	7080.49	25.90	7080.49	50.65	7088.49	76.55
X1 1318	4.0			3.00			
X2 620.0				3.40	3.00	3,00	
GR7088.5	0.01	7080.48	26.20	7080.48	51.70	7088.48	77.90
X1 1315	4.0		79.25		3.00	3.00	
X2 620.0						•	
GR7088.5	0.01	7080.46	26.50	7080.46	52.75	7088.46	79.25
X1 1312	4.0		80.60	3.00	3.00	3.00	
X2 620.0	-		-				
GR7088.4	0.01	7080.45	26.80	7080.45	53.80	7088.45	80.60
X1 1309	4.0		81.95	3.00		3.00	
X2 620.0					=		
		3000 44	27 10	7080.44	54.85	7088.44	81.95
GR7088.4	0.01	7080.44	2,110				
X1 1306	0.01		83.30	3.00	3.00	3.00	
			83.30	3.00	3.00		
X1 1306 X2 620.0 GR7088.4	4.0	0.01 7080.42	83.30	3.00	3.00	3.00 7088.42	
X1 1306 X2 620.0 GR7088.4 X1 1303	4.0	0.01 7080.42	83.30		3.00 55.90	3.00 7088.42	
X1 1306 X2 620.0 GR7088.4	4.0 0.01 4.0	0.01 7080.42	83.30 27.40 84.65	3.00 7080.42	3.00 55.90 3.00	3.00 7088.42	

TABLE F-6. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL H (continued).

	-						
X11300 X2 620.0	4.0	0.01	86.00	3.00	3.00	3.00	
GR7088.4						7088.40	86.00
X1 1297 X2 620.0		0.01					
GR7088.3 X1 1294	0.01	7080.34	28.60	7080.34	59.35 3.00	7088.34	87.95
X2 620.0							
GR7088.3 X1 1291	0.01	7080.28	29.20 91.85	7080.28	60.70 3.00	7088.2B 3.00	89.90
X2 620.0							
GR7088.2 X1 1288		7080.22 0.01	93.80	3.00	3.00	7088.22 3.00	91.85
X2 620.0 GR7088.2	0.01	7080.16	30.40	7080.16	63.40	7088.16	93.80
X1 1285		0.01		3.00			30.00
X2 620.0 GR7088.1	0.01	7080.10	31.00	7080.10	64.75	7088.10	95.75
X1 1282 X2 620.0	4.0	0.01	97.70	2.00	2.00	2.00	
GR7088.0		7080.04		7080.04	66.10		97.70
X11280 X2 620.0	4.0	0.01	99.00	20.00	20.00	20.00	
GR7088.0 X1 1260	0.01	7080.00				7088.00	99.00
X2 620.0				20.00		20.00	
GR7087.9 X1 1240	0.01	7079.90	32.00	7079.90		7087.90	100.25
X2 620.0							
GR7087.8 X1 1220	4.0	0.01	102.75	7079.60		20.00	101.50
X2 620.0 GR7087.7	0.01	חד פרחד	32 00	חר פרחר	70.75	7087.70	102.75
X11200				100.00			102.75
X2 620.0 GR7087.6	0.01	7079.60	32.00	7079.60	72.00	7087.60	104.00
X11100 X2 620.0				50.00			
GR7087.4				7079.40			104.00
X11050 X2 620.0	4.0	0.01	104.00	20.00	20.00	20.00	
GR7087.0				7079,00		7087.00	104.00
X11030 X2 620.0	4.0			20.00		20.00	
GR7086.5 X11010		7078.50		7078.50		7086.50	104.00
X2 620.0							
GR7085.0 X11000	4.0	0.01		7077.00		20.00	104.00
X2 620.0 GR7077.5	0.01	7075.50	8.00	7075.50	48.00	7077 50	56.00
X1 980	4.0	0.01	56.00	20.00	20.00	20.00	00.00
X2 620.0 GR7074.8	0.01	7072.80	8.00	7072.80	48.00	7074.80	56.00
X1 960 X2 620.0	4.0	0.01	56.00	20.00	20.00	20.00	
GR7072.1		7070.10		7070.10			56.00
X1 940 X2 620.0	4.0	0.01		20.00	20.00	20.00	•
GR7069.4 X1 920	0.01 4.0	7067.40	8.00 56.00	7067.40	48.00 20.00	7069.40 20.00	56.00
X2 620.0							
GR7066.7 X1900	4.0	7064.70		7064.70		7066.70 20.00	56.00
X2 620.0 GR7064.0		7062.00	8 00	7062.00	48.00	7064.00	56.00
X1 680	4.0	0.01	56.00	20.00			36.00
X2 620.0 GR7061.4	0.01	7059.40	8.00	7059.40	48.00	7061.40	56.00
X1 860	4.0	0.01	56.00	20.00		20.00	
X2 620.0 GR7058.8		7056.80		7056.80		7058.80	56.00
X1 840 X2 620.0	4.0	0.01	56.00	20.00	20.00	20.00	
GR7056.2		7054.20		7054.20		7056.20	56.00
X1 820 X2 620.0	4.0	0.01	56.00	20.00	20.00	20.00	
GR7053.6 X1 800	0.01	7051.60 0.01	8.00 56.00	7051.60		7053.60	56.00
X2 620.0				2	,	20.00	

TABLE F-6. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL H (continued).

V1 700		7049.00	56 AC	20 00	20 00	20 00	
X2 620.0 GR7046.4 X1 760	0.00	7046.40	8.00 56.00	7046.40	48.00 20.00	7048.40	56.00
GR7045.8 X1 740	0.0						
GR7043.2 X1 720	0.03	7041.20	8.00	7041.20	48.00	7043.20	56.00
X2 620.0 GR7040.6 X1700	0.03	7038.60	8.00 56.00	7038.60	48.00	7040.60	56.00
GR7038.0 X1 680	0.01	7036.00	8.00 56.00	7036.00	48.00 20.00	7038.00	56.00
X2 620.0 GR7035.4 X1 660 X2 620.0	0.01	7033.35	8.00	7033.35	48.00	7035.35	56.00
GR7032.7	0.01	7030.70	8.00	7030.70	48.00	7032.70	56.00
X2 620.0 GR7030.0 X1 620 X2 620.0	0.01	7028.05	8.00 56.00	7028.05 20.00	48.00 20.00	7030.05	56.00
GR7027.4	0.01	7025.40	8.00	7025.40	48.00	7027.40	56.00
X2 620.0 GR7024.8 X1 580 X2 620.0	0.01	7022.75	8.00 56.00	7022.75	48.00	7024.75	56.00
V1 560	0.01	7020.10	56.00	7020.10	48.00	7022.10	56.00
X2 620.0 GR7019.5 X1 540 X2 620.0	0.01	7017.45	8.00	7017.45	48.00	7019.45	56.00
X2 620.0 GR7016.8 X1 520 X2 620.0	0.01	7014.80	8.00	7014.80	48.00	7016.80	56.00
X2 620.0 GR7014.1 X1500 X2 620.0	0.01	7012.15	8.00	7012.15	48.00	7014.15	56.00
X2 620.0 GR7011.5 X1 480 X2 620.0	0.01	7009.50	8.00	7009.50	48.00	7011.50	56.00
X2 620.0 GR7008.9 X1 460	0.01	7006.90	8.00	7006.90	48.00	7008.90	56.00
GR7006.3		7004.30					56.00
X1 440 X2 620.0 GR7003.7 X1 420 X2 620.0	0.01	7001.70	8.00	7001.70	48.00	7003.70	56.00
X1 420 X2 620.0 GR7001.1 X1 400	0.01	0.01	8.00	20.00	20.00	20.00 7001.10	56.00
GR6998.5	0.01	6996.50	8.00	6996.50	48.00	6998.50	56.00
X1 380 X2 620.0 GR6995.9		6993.90		20.00		20.00	56.00
X1 360 X2 620.0 GR6993.3		0.01		20.00		20.00	56.00
X1 340 X2 620.0 GR6990.7		6988.70	56.00 8.00	20.00	20.00	20.00	56.00
X1 320 X2 620.0 GR6988.1	4.0 0.01	0.01	56.00 8.00	20.00	20.00	20.00	56.00
X1300 X2 620.0 GR6985.5	4.0 0.01	0.01 6983.50	56.00 8.00	20.00		20.00	56.00
X1 280 X2 620.0 GR6982.7	4.0 0.01	0.01	56.00	20.00	20.00	20.00	56.00
X1 260	4.0	0.01	56.00	20.00	20.00	20.00	

TABLE F-6. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL H (continued).

X2 620.0	0 01	6077 05	6 00	6077 02	40.00	6070 03	F.C. 00
GR6979.8 X1240	0.01	09//.83	56.00	7 00	7.00	7 00	56.00
X1240 X2 620.0	4.0	0.01	36.00	7.00	7.00	7.00	
GR6977.0	0.03	4975 DD	8 00	6075 00	40 00	6877 00	56.00
X1233				10.00			36.00
X2 620.0	4.0	0.01	30.00	10.00	10.00	10.00	
	0.01	5074 00	9 00	6974 00	40 00	6076 00	56.00
GR6976.0 X1223	0.01	0974.00	5.00	10.00	10.00	10.00	36.00
	4.0	0.01	6400	10.00	10.00	10.00	
X2 620.0	0.01	6070 70	10.00	6030 30	50.00	6075 70	<i>5</i> 4 . 0 0
GR6975.7							64.00
X1213	4.0	0.01	64.00	10.00	10.00	10.00	
X2 620.0							
GR6974.4	0.01	6971.40	12.00	6971.40	52.00	6974.40	64.00
X1203	4.0	0.01	64.00	10.00	10.00	10.00	
X2 620.0							
GR6973.1	0.01	6970.10	12.00	6970.10	52.00	6973.10	64.00
X1193	4.0	0.01	64.00	10.00	10.00	10.00	
X2 620.0							4
GR6971.8	0.01	6968.80	12.00	6968.80	52.00	6971.80	64.00
X1183	4.0	0.01	64.00	8.00	8.00	8.00	
X2 620.0							
GR6970.5	0.01	6967.50	12.00	6967.50	52.00	6970.50	64.00
X1175	4.0	0.01	72.00	10.00	10.00	10.00	
X2 620.0							
GR6970.4	0.01	6966.40	16.00	6966.40	56.00	6970.40	72.00
X1165	4.0	0.01	72.00	8.00	8.00	8.00	
X2 620.0							
GR6969.1	0.01	6965.10	16.00	6965.10	56.00	6969.10	72.00
X1157	4.0	0.01	72.00	57.00	57.00	57.00	
X2 620.0							
GR6968.1	0.01	6964.10	16.00	6964.10	56.00	6968.10	72.00
X1100	4.0	0.01	12.00	50.00	50.00	50.00	
X2 620.0	2 0.						
GR6967.8	0.01	6963.80	16.00	6963.80	56.00	6967.80	72.00
X150	4.0	0.01	72.00	10.00	10.00	10.00	
X2 620.0							
GR6967.7 X140	0.01	6963.70	16.00	6963,70	56.00	6967.70	72.00
	4.0	0.01	72.00				
X2 620.0							
GR6967.6	0.01	6963.60	16.00	6963.60	56.00	6967.60	72.00
EJ							
ER							

TABLE F-7. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL I.

			ER SHIRLE						
			I - INCLUI SCRITICAL		SS SECTI	ONS HC4-	·1 THROUGH	HC4-3	
	J1				-1.0			581.0	7086.0
	J2 -1 NC 0.035		-1.0 0.035		0.3				
	X11440	4.0	0.01						
	X2 581.0 GR7083.0	0.01	7001 00	9 00	7081 00	£0 00	7083.00	66.00	
	X11450	4.0	0.01	66.00	10.00	10.00	10.00	00.00	
	X2 581.0		,						
	GR7083.0 X1 1490	4.0					7083.00	66.00	
	X2 581.0								
	GR7083.3	0.01	7081.31	8.00	7081.31	58.00	7083.31	66.00	
	X2 581.0								
	GR7084.1	0.01	7082.10	8.00	7082.10	58.00	7084.10	66.00	
	X1 1630 X2 581.0	4.0	0.01	66.00	40.00	40.00	40.00		
	GR7084.4	0.01	7082.41	8.00	7082.41	58.00	7084.41	66.00	
	X11730 X2 581.0	4.0	0.01	66.00	100.00	100.00	100.00		
	GR7085.2	0.01	7083.20 0.01	8.00	7083.20	58.00	7085.20	66.00	
	X1 1770 X2 581.0	4.0	0.01	66.00	40.00	40.00	40.00		
		0.01	7083.80	8.00	7083.80	58.00	7085.80	66.00	
	X11870 X2 581.0		0.01	66.00	100.00	100.00	100.00		
			7085.30	8.00	7085.30	58.00	7087.30	66.00	
	X1 1890	4.0	7085.30 0.01	66.00	20.00	20.00	20.00		
	X2 581.0 GR7088.0	0.01	7085.97	8.00	7085 97	58.00	7087.97	66.00	
	X1 1910	4.0	0.01	66.00	20.00	20.00	20.00	00.00	
	X2 581.0 GR7088.6		7086.63	B 00	7096 63	50 00	7088.63	66.00	
-	X11930	4.0	0.01	66.00	20.00		20.00	66.00	
	X2 581.0		7087.30	0 00	7007 20	F0 00	7000 00		
:	X1 1940	4.0	0.01				7089.30 10.00	66.00	
	X2 581.0	0.01	7007 64						
	X1 1960	4.0	7087.64 0.01	66.00	20.00	20.00	20.00	66.00	
	X2 581.0		7088.32						
2	K13		0.01				7090.32 20.00	66.00	
3	X2 581.0								
	K1 1995	4.0	7089.00 0.01	66.00	15.00	15.00	7091.00 15.00	66.00	
- 2	KZ 581.U								
2	GR7091.5	0.01	7089.49 0.01	8.00 66.00	7089.49	58.00	7091.49	66.00	
;	K2 581.0								
2	SR7092.1 K1 2035	0.01	7090.14 0.01	8.00	7090.14	58.00	7092.14 20.00	66.00	
,	(2.581.0								
2	SR7092.8 (1 2055	0.01	7090.79	8.00	7090.79	58.00	7092.79	66.00	
-	75 201.0								
	GR7093.4 K12075	0.01	7091.45 0.01	8.00	7091.45	58.00 20.00		66.00	
2	3 581.0			00.00	20.00	20.00	20.00		
	SR7094.1 K1 2090		7092.10		7092.10			66.00	
	2 581.0	4.0	0.01	66.00	15.00	15.00	15.00		
	R7094.6				7092.61		7094.61	66.00	
	<pre></pre>	4.0	0.01	66.00	20.00	20.00	20.00		
Ģ	R7095.3		7093.28		7093.28	58.00		66.00	
	1 2130 2 581.0	4.0	0.01	66.00	20.00	20.00	20.00		
C	R7096.0		7093.95	8.00	7093.95			66.00	
	1 2150 2 581.0	4.0	0.01	66.00	20.00	20.00	20.00		
G	R7096.6	0.01 7	094.63	8.00	7094.63	58.00	7096.63	66.00	
	12170 2 581.0	4.0	0.01	66.00	20.00	20.00	20.00		
G	R7097.3	0.01 7	095.30 0.01	8.00	7095.30	58.00	7097.30	66.00	
	1 2270 2 540.5	4.0	0.01	66.00	100.00	100.00	100.00		

TABLE F-7. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL I (continued).

	0.01 7098.00 4.0 0.01	8.00 7098.00 66.00 100.00	58.00 7100.00 100.00 100.00	66.00
X2 500.0 GR7102.7 X1 2390	0.01 7100.70 4.0 0.01	8.00 7100.70 65.09 20.00	58.00 7102.70 20.00 20.00	66.00
X2 495.5 GR7102.9 X1 2410			57.09 7102.94 20.00 20.00	
X2 490.9 GR7103.2 X1 2430			56.18 7103.17 20.00 20.00	
X2 486.4 GR7103.4 X1 2450	0.01 7101.41 4.0 0.01	8.00 7101.41 62.36 20.00	55.27 7103.41 20.00 20.00	63.27
X2 481.8 GR7103.6 X1 2470			54.36 7103.65 20.00 20.00	
	0.01 7101.88 4.0 0.01	8.00 7101.88 60.55 20.00	53.45 7103.88 20.00 20.00	61.45
X2 472.7 GR7104.1 X1 2510	0.01 7102.12 4.0 0.01	8.00 7102.12 59.64 20.00	52.55 7104.12 20.00 20.00	60.55
GR7104.4 X1 2530	0.01 7102.35 4.0 0.01	8.00 7102.35 58.73 20.00	51.64 7104.35 20.00 20.00	59.64
GR7104.6 X1 2550	0.01 7102.59 4.0 0.01	8.00 7102.59 57.82 20.00	50.73 7104.59 20.00 20.00	58.73
GR7104.8 X1 2570			49.82 7104.83 20.00 20.00	
GR7105.1 X12590 X2 450.0	0.01 7103.06 4.0 0.01		48.91 7105.06 20.00 20.00	
GR7105.3 X1 2614	0.01 7103.30 4.0 0.01	8.00 7103.30 56.00 24.00	48.00 7105.30 24.00 24.00	56,00
GR7105.7 X1 2714 X2 421.8	0.01 7103.66 4.0 0.01	8.00 7103.66 56.00 100.00	48.00 7105.66 100.00 100.00	56.00
GR7107.2 X12	0.01 7105.18 4.0 0.01	8.00 7105.18 56.00 100.00	48.00 7107.18 100.00 100.00	56.00
GR7108.7 X12815 X2 399.0	0.01 7106.70 4.0 0.01	8.00 7106.70 56.00 1.00	46.00 7108.70 1.00 1.00	56.00
V2 200 0			48.00 7108.70 15.00 15.00	56.00
X / 399.U			48.00 7108.76 100.00 100.00	56.00
X2 399.0			48.00 7109.18 100.00 100.00	56.00
X2 399.0				56.00
GR7109.6 X1 3060 X2 399.0	0.01 7107.64	8.00 7107.64 53.60 20.00	47.20 7109.64 20.00 20.00	55.20
GR7109.7 X1 3080 X2 399.0	0.01 7107.72 4.0 0.01	8.00 7107.72 52.00 20.00	45.60 7109.72 20.00 20.00	53.60
GR7109.8 X1 3100 X2 399.0	0.01 7107.80 4.0 0.01	8.00 7107.80 50.40 20.00	44.00 7109.80 20.00 20.00	52.00
GR7109.9 X1 3120 X2 399.0	0.01 7107.88 4.0 0.01	8.00 7107.88 48.80 20.00 8.00 7107.96	42.40 7109.88 20.00 20.00	50.40
GR7110.0 X1 3140 X2 399.0	0.01 7107.96 4.0 0.01	47.20 20.00	40.80 7109.96 20.00 20.00	48.80
GR7110.0 X1 3160 X2 399.0	0.01 7108.04 4.0 0.01	8.00 7108.04 45.60 20.00	20.00 20.00	47.20
GR7110.1 X1 3180	0.01 7108.12 4.0 0.01	8.00 7108.12 44.00 20.00	37.60 7110.12 20.00 20.00	45.60

TABLE F-7. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL I (continued).

X2 399.0							
GR7110.2	0.01	7108.20	8.00	7108.20	36.00	7110.20	44.00
X1 3200		0.01	42.40	20.00	20.00	20.00	
X2 399.0						4.6.2.22	
GR7110.3 X1 3220	0.01	7108.28	8.00	7108.28	34.40	7110.28	42.40
		0.01	40.80	20.00	20.00	20.00	
X2 399.0							
GR7110.4	0.01	7108.36	8.00	7108.36	32.80	7110.36	40.80
X1 3240	4.0	0.01	39.20	20.00	20.00	20.00	
X2 399.0							
GR7110.4 X1 3260	0.01	7108.44	8.00	7108.44	31.20	7110.44	39.20
X1 3260	4.0	0.01	37.60	20.00	20.00	20.00	
X2 399.0							
GR7110.5 X11	0.01	7108.52	8.00	7108.52	29.60	7110.52	37.60
X11	4.0	0.01	36.00	20.00	20.00	20.00	
X2 399.0							
GR7110.6	0.01	7108.60	8.00	7108.60	28,00	7110.60	36.00
X1 3290	4 0	7108.60 0.01	37.43	10.00	10.00	10.00	20.00
X2 399.0	٦.٠	0.01	37.43		10.00	10.00	
	0.01	7100 07	0.00	7100 02	20.42	7110 03	27 42
GR7110.8 %1 3310	0.01	,100.63	40.00	1100.03	20.43	7110.63	37.43
X1 3310 X2 399.0		0.01	40.29	20.00	20.00	20.00	
X2 399.0		3100 00					
GR7111.3 X1 3330	0.01	/109.29	8.00	/109.29	32.29	/111.29	40.29
X1 3330	4.0	0.01	43.14	20.00	20.00	20.00	
X2 399.0	_						
GR7111.7 X13350	0.01	7109.74	8.00	7109.74	35.14	7111.74	43.14
X13350	4.0	0.01	46.00	20.00	20.00	20.00	
X2 399.0							
GR7112.2	0.01	7110.20	8.00	7110.20	38.00	7112.20	46.00
X1 3360	4.0	0.01	45.33	10.00	10.00	10.00	
X2 399.0							
GR7112.3	0.01	7110.26	8.00	7110.26	37.33	7112.26	45.33
X1 3380	4.0	0.01	44.00	20.00	20.00	20.00	
X2 399.0							
GR7112.4	0.01	7110.38	8.00	7110.38	36.00	7112.38	44.00
X1 3400	4.0	0.01	42.67	20.00	20.00	20.00	
X2 399.0							
GR7112.5	0.01	7110.50	8.00	7110.50	34.67	7112.50	42.67
X1 3420	4.0	0.01	41.33	20.00	20.00	20.00	
X2 399.0							
GR7112.6	0.01	7110.62	8.00	7110.62	33.33	7112.62	41.33
X1 3440	4.0	0.01	40.00	20.00	20.00	20.00	
X2 399.0							
GR7112.7	0.01	7110.74	8.00	7110.74	32.00	7112.74	40.00
X1 3460	4.0	0.01	38.67	20.00	20.00	20.00	10.00
X2 399.0							
GR7112.9	0.01	7110.86	8.00	7110.86	30.67	7112.86	38.67
X1 3480	4.0	0.01	37.33	20.00	20.00	20.00	20.0
X2 399.0							
GR7113.0	0.01	7110.99	8 00	7110 99	20 33	7112 90	37.33
X13500	4 0	0.01	36.00	20.00	20.00	20.00	٠,٠٠٥
X2 399.0	4.0	0.01	50.00	20.00	20.00	20.00	
GR7113.1	0.01	7111 10	8 00	7111 10	28 00	7113 10	36.00
EJ	0.01	.111.10	0.00	,111.10	20.00	,113.10	36.00
ER							
₩11							

TABLE F-8. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL I.

	_ ••							
Tl	PATHFIND	ER SHIRLE	Y BASIN	TAILINGS	RECLAM	ATION L THROUGH	1104 3	
		PERCRITIC		22 250110	NS HC4	LINKOUGH	HC4-3	
Ji			1.0	-1.0			399.0	7113.0
J2 -1	0	-1.0		^ 3				
NC 0.035	4.0	0.035	36.00	20.00	20.00	20.00		
X2 399.0	1	****		20.00				
GR7113.1	0.01	7111.10	8.00	7111.10	28.00	7113.10	36.00	
XI 3480	4.0	0.01	31.33	20.00	20.00	20.00		
GR7113.0	0.01	7110.98	8.00	7110.98	29.33	20.00 7113.10 20.00 7112.98 20.00 7112.86 20.00 7112.74	37.33	
X1 3460	4.0	0.01	38.67	20.00	20.00	20.00		
X2 399.0	0.01	7110 96	9 00	7110 86	30 67	7112 B6	38.67	
X1 3440	4.0	0.01	40.00	20.00	20.00	20.00	50.0	
X2 399.0	ı					20.00 7112.74 20.00		
GR7112.7	0.01	7110.74	8.00	7110.74	32.00	7112.74	40.00	
X2 399.0	4.0	0.01	41.33	20.00	20.00	20.00		
GR7112.6	0.01	7110.62	8.00	7110.62	33.33	7112.62 20.00	41.33	
X1 3400	4.0	0.01	42.67	20.00	20.00	20.00		
GR7112.5	0.01	7110.50	8.00	7110.50	34.67	7112.50	42.67	
X1 3380	4.0	0.01	44.00	20.00	20.00	20.00		
X2 399.0						7112.50 20.00 7112.38 10.00		
GR7112.4	0.01	7110.38	8.00 45 33	10.38	10.00	10.00	44.00	
X2 399.0	4.0	0.01	45.55	10.00	10.00	10.00		
GR7112.3	0.01	7110.26	8.00	7110.26	37.33	7112.26	45.33	
X13350	4.0	0.01	46.00	20.00	20.00	7112.26		
GR7112.2	0.01	7110.20	8.00	7110.20	38.00	7112.20 20.00	46.00	
X1 3330	4.0	0.01	43.14	20.00	20.00	20.00		
X2 399.0	0.01	7100 74	0 00	7100 74	25 14	7111 74	43.14	
X1 3310	4.0	0.01	40.29	20.00	20.00	20.00	43.14	
X2 399.0	• • •							
GR7111.3	0.01	7109.29	8.00	7109.29	32.29	7111.29	40.29	
X1 3290 X2 399.0	4.0	0.01	37.43	10.00	10.00	7111.74 20.00 7111.29 10.00 7110.83 20.00 7110.60		
GR7110.8	0.01	7108.83	8.00	7108.83	29.43	7110.83	37.43	
X11	4.0	0.01	36.00	20.00	20.00	20.00		
GR7110.6	0.01	7108.60	8.00	7108.60	28.00	7110.60	36.00	
X1 3260	4.0	0.01	37.60	20.00	20.00	20.00		
X2 399.0	0.01	7100 52	0 00	7100 52	20 60	7110.60 20.00 7110.52	37.60	
X1 3240	4.0	0.01	39.20	20.00	20.00	20.00	37.00	
X2 399.0						20.00 7110.44 20.00		
GR7110.4	0.01	7108.44	8.00	7108.44	31.20	7110.44	39.20	
X1 3220 X2 399.0	4.0	0.01	40.80	20.00	20.00	20.00		
GR7110.4	0.01	7108.36	8.00	7108.36	32.80	7110.36	40.80	
X1 3200	4.0	0.01	42.40	20.00	20:00	7110.36		
X2 399.0 GR7110.3	0.01	7108.28	8.00	7108.28	34.40	7110.28	42.40	
VI DIOO	4.0	0.01	44.00	20.00	20.00	20.00		
X2 399.0		73.00 00	5 00	7100 20	36 00	7110 20	44.00	
GR7110.2 X1 3160	4.0	7108.20	45.60	7108.20 20.00	20.00	7110.20	44.00	
X2 399.0								
GR7110.1		7108.12 0.01	8.00	7108.12		7110.12	45.60	
X1 3140 X2 399.0	4.0	0.01	47.20	20.00	20.00	20.00		
GR7110.0		7108.04		7108.04	39.20	7110.04	47.20	
X1 3120	4.0	0.01	48.80	20.00	20.00	20.00		
X2 399.0 GR7110.0		7107.96	8.00	7107.96	40.80	7109.96	48.80	
X1 3100	4.0	0.01	50.40	20.00	20.00	20.00		
X2 399.0		7107 00	0 00	7107 00	42.40	7109.88	50.40	
GR7109.9 X1 3080	4.0	7107.88	52.00	7107.88		20.00	50.40	
X2 399.0								
GR7109.8		7107.80		7107.80 20.00		7109.80	52.00	
X1 3060 X2 399.0		0.01	53.60	20.00	20.00	20.00		
GR7109.7	0.01	7107.72		7107.72		7109.72	53.60	
X1 3040		0.01	55.20	10.00	10.00	10.00		
X2 399.0								

TABLE F-8. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL I (continued).

GR7109.6 X13030	0.01 71 4.0	07.64	8.00 56.00	7107.64	47.20 100.00	7109.64	55.20
	0.01 71						
X2 399.0 GR7109.2 X1 2830							
X2 399.0	0.01.71	06.76	8.00	7106.76	48.00	7108.76	
X12815	4.0	0.01	56.00	1.00	1.00	1.00	
	0.01 71						
X2 421 B	0.01 71 4.0						
GR7107.2 X1 2614	0.01 71	05.18	8.00 56.00	7105.18	48.00 24.00	7107.18	56.00
GR7105.7 X12590	0.01 71	03.66	8.00 56.00	7103.66 20.00	48.00 20.00	7105.66 20.00	56.00
X2 450.0 GR7105.3 X1 2570	0.01 71 4.0	03.30	8.00 56.91	7103.30	48.00 20.00	7105.30 20.00	56.00
	0.01 71 4.0						
X2 459.1	0.01 71						
X2 463.6							58.73
X1 2510 X2 468.2 GR7104.4	0.01 71						59.64
X1 2490 X2 472.7	4.0						
¥2 477 3	0.01 71 4.0						
GR7103.9 X1 2450 X2 481.8	0.01 7 <u>1</u> 4.0.	0.01	8.00 62.36	7101.88	53.45 20.00	7103.88	61.45
GR7103.6 X1 2430	4.0	01.65	8.00 63.27	7101.65 20.00	54.36 20.00	7103.65 20.00	62.36
X2 486.4 GR7103.4 X1 2410	0.01 71	01.41	8.00 64.18	7101.41 20.00	55.27 20.00	7103.41 20.00	63.27
X2 490.9	0.01 71					7103.17	
X2 495.5	0.01 71	.00.94	8.00	7100.94	57.09	7102.94 100.00	65.09
X2 500.0 GR7102.7	0.01 71	.00.70	8.00	100.00	58.00	7102.70	66.00
X1 2270 X2 540.5 GR7100.0	4.0 0.01 70			100.00		7100.00	66.00
X12170 X2 581.0 GR7097.3	4.0	0.01	66.00	20.00	20.00	20.00	66.00
X1 2150 X2 581.0	4.0	0.01	66.00	20.00	20.00	20.00	
GR7096.6 X1 2130 X2 581.0	0.01 70 4.0	0.01	8.00 66.00	7094.63	58.00 20.00	7096.63	66.00
GR7096.0 X1 2110	0.01 70 4.0	0.01	8.00 66.00	7093.95 20.00	58.00 20.00	7095.95 20.00	66.00
X2 581.0 GR7095.3 X1 2090	0.01 70	0.01	8.00 66.00	7093.28 15.00	58.00 15.00	7095.28 15.00	66.00
X2 581.0 GR7094.6 X12075	0.01 70		8.00	7092.61	58.00 20.00	7094.61	66.00
X2 581.0 GR7094.1 X1 2055	0.01 70			7092.10		7094.10	66.00
2000	3.0	0.01	55.00	20.00	20.00	20.00	

TABLE F-8. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL I (continued).

X2 581.0 GR7093.4								
X1 2035	X2 581.0	0 01	7001 45	0 00	7001 45	EG 00	7002 45	
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GR7092.8 0.01 7090.79 8.00 7090.79 58.00 7092.79 66.00 X1 2015 4.0 0.01 66.00 20.00 20.00 20.00 20.00 X2 581.0 0.01 7090.14 8.00 7090.14 58.00 7092.14 66.00 X1 1995 4.0 0.01 66.00 15.00 15.00 15.00 X2 581.0 0.01 7089.49 8.00 7089.49 58.00 7091.49 66.00 X2 581.0 0.01 7089.00 8.00 7089.00 58.00 7091.00 66.00 X2 581.0 0.01 7089.00 8.00 7089.00 58.00 7091.00 66.00 X1 1960 4.0 0.01 66.00 20.00 20.00 20.00 X2 581.0 0.01 7088.32 8.00 7088.32 58.00 7090.32 66.00 X1 1940 4.0 0.01 66.00 10.00 10.00 10.00 X2 581.0 0.01 7087.64 8.00 7087.64 58.00 7089.64 66.00 X1 1930 4.0 0.01 66.00 20.00 20.00 20.00 X2 581.0 0.01 7087.30 8.00 7087.30 58.00 7089.30 66.00	V0 E01 0							
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AZ 381.0 OR7092.1 0.01 7090.14 8.00 7090.14 58.00 7092.14 66.00 X1 1995 4.0 0.01 66.00 15.00 15.00 15.00 46.00 15.00	X1 2015	4.0	0.01	66.00	20.00	20.00	20.00	00.00
GR7092.1 0.01 7090.14 8.00 7090.14 58.00 7092.14 66.00 X1 1995 4.0 0.01 66.00 15.00 15.00 15.00 46.00 GR7091.5 0.01 7089.49 8.00 7089.49 58.00 7091.49 66.00 X2 581.0 687091.0 0.01 7089.00 8.00 7089.00 58.00 7091.00 66.00 X1 1960 4.0 0.01 66.00 20.00 20.00 20.00 X2 581.0 687090.3 0.01 7088.32 8.00 7089.30 58.00 7090.32 66.00 X1 1940 4.0 0.01 66.00 10.00 10.00 10.00 X2 581.0 687089.3 4.0 0.01 66.00 20.00 20.00 20.00 X2 581.0 687089.3 8.00 7087.64 58.00 7089.30 66.00 X1 1910 4.0								
X2 581.0 GR7091.5	GR7092.1	0.01	7090.14	8.00	7090.14	58.00	7092.14	66.00
X2 581.0 GR7091.5	X1 1995	4.0	0.01	66.00	15.00	15.00	15.00	
GR7091.5 0.01 7089.49 8.00 7089.49 58.00 7091.49 66.00 X13 4.0 0.01 66.00 20.00 20.00 20.00 20.00 GR7091.0 0.01 7089.00 8.00 7089.00 58.00 7091.00 66.00 X1 1960 4.0 0.01 66.00 20.00 20.00 20.00 20.00 X2 581.0 67709.3 0.01 7088.32 8.00 7088.32 58.00 7099.32 66.00 X1 1940 4.0 0.01 66.00 10.00 10.00 10.00 X2 581.0 677089.6 0.01 7087.64 8.00 7087.64 58.00 7089.64 66.00 X1 1910 4.0 0.01 66.00 20.00 20.00 20.00 20.00 X1 1890 4.0 0.01 66.00 20.00 20.00 20.00 20.00 X2 581.0 67087.3 8.00 7085.97 8.00 7085.97 58.0	X2 581.0							
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GR7091.0 0.01 7089.00 8.00 7089.00 58.00 7091.00 66.00 X1 1960 4.0 0.01 66.00 20.00 20.00 20.00 20.00 GR7090.3 0.01 7088.32 8.00 7088.32 58.00 7090.32 66.00 X1 1940 4.0 0.01 66.00 10.00 10.00 10.00 10.00 10.00 GR7089.6 0.01 7087.64 0.00 8.00 7087.64 58.00 58.00 7089.64 66.00 66.00 X1 1910 4.0 0.01 66.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 X1 1910 4.0 0.01 66.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 X1 1910 4.0 0.01 66.03 8.00 7087.30 58.00 7089.30 66.00 66.00 66.00 20.00 20.00 20.00 20.00 X2 581.0 GR7088.0 0.01 7085.97 8.00 7085.97 58.00 7087.97 66.00 66.00 66.00 20.00 20.00 20.00 20.00 X1 1870 4.0 0.01 66.00 100.00 100.00 100.00 100.00 100.00 100.00 100.00 100.00 X2 581.0 GR7085.8 0.01 7083.80 8.00 7085.30 58.00 7087.30 66.00 66.00 X1 1770 4.0 0.01 66.00 100.00 100.00 100.00 100.00 100.00 100.00 X2 581.0 GR7085.2 0	X13	4.0	0.01	66,00	20.00	20.00	20.00	
GR7091.0 0.01 7089.00 8.00 7089.00 58.00 7091.00 66.00 X1 1960 4.0 0.01 66.00 20.00 20.00 20.00 20.00 GR7090.3 0.01 7088.32 8.00 7088.32 58.00 7090.32 66.00 X1 1940 4.0 0.01 66.00 10.00 10.00 10.00 10.00 10.00 GR7089.6 0.01 7087.64 0.00 8.00 7087.64 58.00 58.00 7089.64 66.00 66.00 X1 1910 4.0 0.01 66.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 X1 1910 4.0 0.01 66.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 X1 1910 4.0 0.01 66.03 8.00 7087.30 58.00 7089.30 66.00 66.00 66.00 20.00 20.00 20.00 20.00 X2 581.0 GR7088.0 0.01 7085.97 8.00 7085.97 58.00 7087.97 66.00 66.00 66.00 20.00 20.00 20.00 20.00 X1 1870 4.0 0.01 66.00 100.00 100.00 100.00 100.00 100.00 100.00 100.00 100.00 X2 581.0 GR7085.8 0.01 7083.80 8.00 7085.30 58.00 7087.30 66.00 66.00 X1 1770 4.0 0.01 66.00 100.00 100.00 100.00 100.00 100.00 100.00 X2 581.0 GR7085.2 0	X2 581.0							
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GR7090.3 0.01 7088.32 8.00 7088.32 58.00 7090.32 66.00 X1 1940 4.0 0.01 66.00 10.00 10.00 10.00 10.00 10.00 10.00 10.00 X2 581.0 0.01 7087.64 4.0 0.01 66.00 20.00 20.00 20.00 20.00 4.0 0.01 66.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 X2 581.0 0.01 7087.30 8.00 7087.30 58.00 7089.30 66.00 66.00 20.00 20.00 20.00 20.00 20.00 X1 1910 4.0 0.01 66.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 X1 1890 52 81.0 0.01 7086.63 8.00 7086.63 58.00 7088.63 66.00 66.00 20.00 20.00 20.00 20.00 20.00 X2 581.0 0.01 7085.97 8.00 7085.97 58.00 7087.97 66.00 66.00 66.00 20.00 20.00 20.00 66.00 X2 581.0 0.01 7085.30 8.00 7085.30 58.00 7087.97 66.00 66.00 66.00 100.00 100.00 100.00 66.00 X2 581.0 0.01 7083.80 8.00 7083.80 58.00 7087.30 66.00 66.00 66.00 40.00 40.00 40.00 66.00 X2 581.0 0.01 7083.20 8.00 7083.80 58.00 7085.80 66.00 66.00 66.00 40.00 40.00 40.00 66.00 X2 581.0 0.01 7082.11 8.00 7082.11 58.00 7084.41 66.00 66.00 40.00 40.	X1 1960	4.0	0.01	66.00	20.00	20.00	20.00	
GR7090.3 0.01 7088.32 8.00 7088.32 58.00 7090.32 66.00 X1 1940 4.0 0.01 66.00 10.00 10.00 10.00 10.00 10.00 10.00 10.00 X2 581.0 0.01 7087.64 4.0 0.01 66.00 20.00 20.00 20.00 20.00 4.0 0.01 66.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 X2 581.0 0.01 7087.30 8.00 7087.30 58.00 7089.30 66.00 66.00 20.00 20.00 20.00 20.00 20.00 X1 1910 4.0 0.01 66.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 20.00 X1 1890 52 81.0 0.01 7086.63 8.00 7086.63 58.00 7088.63 66.00 66.00 20.00 20.00 20.00 20.00 20.00 X2 581.0 0.01 7085.97 8.00 7085.97 58.00 7087.97 66.00 66.00 66.00 20.00 20.00 20.00 66.00 X2 581.0 0.01 7085.30 8.00 7085.30 58.00 7087.97 66.00 66.00 66.00 100.00 100.00 100.00 66.00 X2 581.0 0.01 7083.80 8.00 7083.80 58.00 7087.30 66.00 66.00 66.00 40.00 40.00 40.00 66.00 X2 581.0 0.01 7083.20 8.00 7083.80 58.00 7085.80 66.00 66.00 66.00 40.00 40.00 40.00 66.00 X2 581.0 0.01 7082.11 8.00 7082.11 58.00 7084.41 66.00 66.00 40.00 40.	X2 581.0							
X1 1940	GR7090.3	0.01	7088.32	8.00	7088.32	58.00	7090.32	66.00
X2 581.0 GR7089.6 0.01 7087.64 8.00 7087.64 58.00 7089.64 66.00 X11930 4.0 0.01 66.00 20.00 20.00 20.00 X2 581.0 GR7089.3 0.01 7087.30 8.00 7087.30 58.00 7089.30 66.00 X1 1910 4.0 0.01 66.00 20.00 20.00 20.00 X2 581.0 GR7088.6 0.01 7086.63 8.00 7086.63 58.00 7088.63 66.00 X1 1890 4.0 0.01 66.00 20.00 20.00 20.00 X2 581.0 GR7088.0 0.01 7085.97 8.00 7085.97 58.00 7087.97 66.00 X1 1870 4.0 0.01 66.00 100.00 100.00 100.00 X2 581.0 GR7087.3 0.01 7085.30 8.00 7085.30 58.00 7087.30 66.00 X1 1770 4.0 0.01 66.00 40.00 40.00 40.00 X2 581.0 GR7085.8 0.01 7083.80 8.00 7083.80 58.00 7085.80 66.00 X11730 4.0 0.01 66.00 100.00 100.00 100.00 X2 581.0 GR7085.2 0.01 7083.20 8.00 7083.20 58.00 7085.20 66.00 X2 581.0 GR7084.4 0.01 7082.41 8.00 7082.41 58.00 7084.41 66.00 X1 1490 4.0 0.01 66.00 40.00 40.00 40.00 X2 581.0 GR7084.1 0.01 7082.10 8.00 7082.10 58.00 7084.41 66.00 X1 1490 4.0 0.01 66.00 40.00 40.00 40.00 X2 581.0 GR7083.0 0.01 7081.31 8.00 7081.31 58.00 7083.31 66.00 X11440 4.0 0.01 66.00 8.00 7081.00 58.00 7083.00 66.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 X1 1440 4.0 0.01 66.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00	X1 1940	4.0	0.01	66.00	10.00	10.00	10.00	
R7089.3								
R7089.3	GR7089.6	0.01	7087.64	8.00	7087.64	58.00	7089.64	66.00
R7089.3	X11930	4.0	0.01	66.00	20.00	20.00	20.00	
RZ 581.0 GR708B.6 0.01 7086.63 8.00 7086.63 58.00 7088.63 66.00 X1 1890 X2 581.0 GR708B.0 0.01 7085.97 8.00 7085.97 58.00 7087.97 66.00 X2 581.0 GR7087.3 0.01 7085.30 8.00 7085.30 58.00 7087.30 66.00 X1 1770 4.0 0.01 66.00 40.00 40.00 40.00 X2 581.0 GR7085.8 0.01 7083.80 8.00 7083.80 58.00 7085.80 66.00 X1 1730 4.0 0.01 66.00 100.00 100.00 100.00 X2 581.0 GR7085.2 0.01 7083.20 8.00 7083.20 58.00 7085.20 66.00 X1 1630 X2 581.0 GR7084.4 0.01 7082.41 8.00 7082.41 58.00 7084.41 66.00 X2 581.0 GR7084.1 0.01 7082.10 8.00 7082.10 58.00 7084.10 66.00 X1 1490 X2 581.0 GR7083.3 0.01 7081.31 8.00 7082.10 58.00 7083.31 66.00 X1 1490 4.0 0.01 66.00 40.00 40.0								
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RZ 581.0 GR708B.6 0.01 7086.63 8.00 7086.63 58.00 7088.63 66.00 X1 1890 X2 581.0 GR708B.0 0.01 7085.97 8.00 7085.97 58.00 7087.97 66.00 X2 581.0 GR7087.3 0.01 7085.30 8.00 7085.30 58.00 7087.30 66.00 X1 1770 4.0 0.01 66.00 40.00 40.00 40.00 X2 581.0 GR7085.8 0.01 7083.80 8.00 7083.80 58.00 7085.80 66.00 X1 1730 4.0 0.01 66.00 100.00 100.00 100.00 X2 581.0 GR7085.2 0.01 7083.20 8.00 7083.20 58.00 7085.20 66.00 X1 1630 X2 581.0 GR7084.4 0.01 7082.41 8.00 7082.41 58.00 7084.41 66.00 X2 581.0 GR7084.1 0.01 7082.10 8.00 7082.10 58.00 7084.10 66.00 X1 1490 X2 581.0 GR7083.3 0.01 7081.31 8.00 7082.10 58.00 7083.31 66.00 X1 1490 4.0 0.01 66.00 40.00 40.0	X1 1910	4.0	0.01	66.00	20.00	20.00	20.00	
GR708B.0	X2 581.0							
GR708B.0	GR7088.6	0.01	7086.63	8.00	7086.63	58.00	7088.63	66.00
GR708B.0	X1 1890	4.0	0.01	66.00	20.00	20.00	20.00	
X2 581.0 0.01 7085.30 8.00 7085.30 58.00 7087.30 66.00 X1 1770 4.0 0.01 66.00 40.00 40.00 40.00 40.00 40.00 X2 581.0 GR7085.8 0.01 7083.80 8.00 7083.80 58.00 7085.80 66.00 X11730 4.0 0.01 66.00 100.00 100.00 100.00 100.00 X2 581.0 GR7085.2 0.01 7083.20 8.00 7083.20 58.00 7085.20 66.00 X1 1630 4.0 0.01 66.00 40.00 40.00 40.00 40.00 X2 581.0 GR7084.4 0.01 7082.41 8.00 7082.41 58.00 7084.41 66.00 X1 1590 4.0 0.1 7082.10 8.00 7082.10 58.00 7084.10 66.00 X2 581.0 GR7084.1 0.01 7082.10 8.00 7082.10 58.00 7084.10 66.00 X1 1490 4.0 0.01 66.00 40.00 40.00 40.00 X2 581.0 GR7083.3 0.01 7081.31 8.00 7081.31 58.00 7083.31 66.00 X1 1440 4.0 0.01 66.00 10.00 10.00<	X2 581.0							•
X2 581.0 0.01 7085.30 8.00 7085.30 58.00 7087.30 66.00 X1 1770 4.0 0.01 66.00 40.00 40.00 40.00 40.00 40.00 X2 581.0 GR7085.8 0.01 7083.80 8.00 7083.80 58.00 7085.80 66.00 X11730 4.0 0.01 66.00 100.00 100.00 100.00 100.00 X2 581.0 GR7085.2 0.01 7083.20 8.00 7083.20 58.00 7085.20 66.00 X1 1630 4.0 0.01 66.00 40.00 40.00 40.00 40.00 X2 581.0 GR7084.4 0.01 7082.41 8.00 7082.41 58.00 7084.41 66.00 X1 1590 4.0 0.1 7082.10 8.00 7082.10 58.00 7084.10 66.00 X2 581.0 GR7084.1 0.01 7082.10 8.00 7082.10 58.00 7084.10 66.00 X1 1490 4.0 0.01 66.00 40.00 40.00 40.00 X2 581.0 GR7083.3 0.01 7081.31 8.00 7081.31 58.00 7083.31 66.00 X1 1440 4.0 0.01 66.00 10.00 10.00<	GR7088.0	0.01	7085.97	8.00	7085.97	58.00	7087.97	66.00
X2 581.0 0.01 7085.30 8.00 7085.30 58.00 7087.30 66.00 X1 1770 4.0 0.01 66.00 40.00 40.00 40.00 40.00 40.00 X2 581.0 GR7085.8 0.01 7083.80 8.00 7083.80 58.00 7085.80 66.00 X11730 4.0 0.01 66.00 100.00 100.00 100.00 100.00 X2 581.0 GR7085.2 0.01 7083.20 8.00 7083.20 58.00 7085.20 66.00 X1 1630 4.0 0.01 66.00 40.00 40.00 40.00 40.00 X2 581.0 GR7084.4 0.01 7082.41 8.00 7082.41 58.00 7084.41 66.00 X1 1590 4.0 0.1 7082.10 8.00 7082.10 58.00 7084.10 66.00 X2 581.0 GR7084.1 0.01 7082.10 8.00 7082.10 58.00 7084.10 66.00 X1 1490 4.0 0.01 66.00 40.00 40.00 40.00 X2 581.0 GR7083.3 0.01 7081.31 8.00 7081.31 58.00 7083.31 66.00 X1 1440 4.0 0.01 66.00 10.00 10.00<	X11870	4.0	0.01	66.00	100.00	100.00	100.00	
X2 581.0 GR7085.8 0.01 7083.80 8.00 7083.80 58.00 7085.80 66.00 X11730 4.0 0.01 66.00 100.00 100.00 100.00 X2 581.0 GR7085.2 0.01 7083.20 8.00 7083.20 58.00 7085.20 66.00 X1 1630 4.0 0.01 66.00 40.00 40.00 40.00 X2 581.0 GR7084.4 0.01 7082.41 8.00 7082.41 58.00 7084.41 66.00 X11590 4.0 0.01 66.00 100.00 100.00 100.00 100.00 0 0 0 0	X2 581.0							
X2 581.0 GR7085.8 0.01 7083.80 8.00 7083.80 58.00 7085.80 66.00 X11730 4.0 0.01 66.00 100.00 100.00 100.00 X2 581.0 GR7085.2 0.01 7083.20 8.00 7083.20 58.00 7085.20 66.00 X1 1630 4.0 0.01 66.00 40.00 40.00 40.00 X2 581.0 GR7084.4 0.01 7082.41 8.00 7082.41 58.00 7084.41 66.00 X11590 4.0 0.01 66.00 100.00 100.00 100.00 100.00 0 0 0 0	GR7087.3	0.01	7085.30	8.00	7085.30	58.00	7087.30	66.00
GR7085.8		4.0	0.01	66.00	40.00	40.00	40.00	
GR7085.2	X2 581.0							
GR7085.2	GR7085.8	0.01	7083.80	8.00	7083.80	58.00	7085.80	66.00
GR7085.2	X11730	4.0	0.01	66.00	100.00	100.00	100.00	
X2 581.0 GR7084.4 0.01 7082.41 8.00 7082.41 58.00 7084.41 66.00 X11590 4.0 0.01 66.00 100.00 100.00 100.00 100.00 X2 581.0 GR7084.1 0.01 7082.10 8.00 7082.10 58.00 7084.10 66.00 X1 1490 4.0 0.01 66.00 40.00 40.00 40.00 40.00 X2 581.0 GR7083.3 0.01 7081.31 8.00 7081.31 58.00 7083.31 66.00 X11450 4.0 0.01 66.00 10.00 10.00 10.00 10.00 00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 X11440 4.0 0.01 66.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 EJ	X2 581.0							
X2 581.0 GR7084.4 0.01 7082.41 8.00 7082.41 58.00 7084.41 66.00 X11590 4.0 0.01 66.00 100.00 100.00 100.00 100.00 X2 581.0 GR7084.1 0.01 7082.10 8.00 7082.10 58.00 7084.10 66.00 X1 1490 4.0 0.01 66.00 40.00 40.00 40.00 40.00 X2 581.0 GR7083.3 0.01 7081.31 8.00 7081.31 58.00 7083.31 66.00 X11450 4.0 0.01 66.00 10.00 10.00 10.00 10.00 00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 X11440 4.0 0.01 66.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 EJ	GR7085.2	0.01	7083.20	9.00	7083.20	58.00	7085.20	66.00
X2 581.0 GR7084.4 0.01 7082.41 8.00 7082.41 58.00 7084.41 66.00 X11590 4.0 0.01 66.00 100.00 100.00 100.00 100.00 X2 581.0 GR7084.1 0.01 7082.10 8.00 7082.10 58.00 7084.10 66.00 X1 1490 4.0 0.01 66.00 40.00 40.00 40.00 40.00 X2 581.0 GR7083.3 0.01 7081.31 8.00 7081.31 58.00 7083.31 66.00 X11450 4.0 0.01 66.00 10.00 10.00 10.00 10.00 00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 X11440 4.0 0.01 66.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 EJ	X1 1630	4.0	0.01	66.00	40.00	40.00	40.00	
X2 581.0 GR7084.1 0.01 7082.10 8.00 7082.10 58.00 7084.10 66.00 X1 1490 4.0 0.01 66.00 40.00 40.00 40.00 X2 581.0 GR7083.3 0.01 7081.31 8.00 7081.31 58.00 7083.31 66.00 X11450 4.0 0.01 66.00 10.00 10.00 10.00 10.00 CR7083.0 0.01 7081.00								
X2 581.0 GR7084.1 0.01 7082.10 8.00 7082.10 58.00 7084.10 66.00 X1 1490 4.0 0.01 66.00 40.00 40.00 40.00 X2 581.0 GR7083.3 0.01 7081.31 8.00 7081.31 58.00 7083.31 66.00 X11450 4.0 0.01 66.00 10.00 10.00 10.00 10.00 CR7083.0 0.01 7081.00	GR7084.4	0.01	7082.41	8.00	7082.41	58.00	7084.41	66.00
GR7084.1 0.01 7082.10 8.00 7082.10 58.00 7084.10 66.00 X1 1490 4.0 0.01 66.00 40.00 40.00 40.00 X2 581.0 GR7083.3 0.01 7081.31 8.00 7081.31 58.00 7083.31 66.00 X11450 4.0 0.01 66.00 10.00 10.00 10.00 10.00 X2 581.0 GR7083.0 0.01 7081.00 66.00 7081.00 58.00 7083.00 66.00 X11440 4.0 0.01 66.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 EJ	X11590	4.0	0.01	66.00	100.00	100.00	100.00	
X2 581.0 GR7083.3 0.01 7081.31 8.00 7081.31 58.00 7083.31 66.00 X11450 4.0 0.01 66.00 10.00 10.00 10.00 10.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 X11440 4.0 0.01 66.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 EJ								
X2 581.0 GR7083.3 0.01 7081.31 8.00 7081.31 58.00 7083.31 66.00 X11450 4.0 0.01 66.00 10.00 10.00 10.00 10.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 X11440 4.0 0.01 66.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 EJ	GR/084.1	0.01	7082.10	8.00	7082.10	58.00	7084.10	66.00
GR7083.3	X1 1490	4.0	0.01	66.00	40.00	40.00	40.00	
X11450 4.0 0.01 66.00 10.00 10.00 10.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 X11440 4.0 0.01 66.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 EJ	AZ 361.U	0 01	7001 31	0 00	7001 21	E0 00	7003 31	66.00
X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 X11440 4.0 0.01 66.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 EJ	V11450	4.0	7081.31	66 00	7081.31	10.00	10 00	86.00
GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 X11440 4.0 0.01 66.00 X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 EJ	Y2 581 0							
X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 EJ	GR7083.0	0.01	7081.00	8.00	7081.00	58.00	7083 00	66 00
X2 581.0 GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 EJ	X11440	4.0	0.01	66.00	. 301.00	30.30	. 505.00	00.00
GR7083.0 0.01 7081.00 8.00 7081.00 58.00 7083.00 66.00 EJ	X2 581 0	•••	0.01	50.00				
EJ ·	GR7083.0	0.01	7081.00	8.00	7081-00	58.00	7083.00	66.00
	EJ			0.00		55.50	. 505.00	00.00

TABLE F-9. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL M.

T 1	PATHFINI	DER SHIRLEY	AASIN	TAILINGS	RECLAMA	TION		
T 2	CHANNEL	M - INCLUI					HC4-5	
ТЗ J1	PMF - St	BCRITICAL		-1.0			170 0	7084.0
J2 -1	C	-1.0		-1.0			170.0	7004.0
NC 0.035	0.035	0.035	0.1	0.3				
X1 100.0 X2 170.0		0.1	114.0					
GR7085.7	0.1	7081.7	32.0	7081.7	82.0	7085.7	114.0	
X1 200.0	4.0	0.1	114.0	100.0	100.0	100.0		
X2 170.0 GR7086.5		7082.5	32 0	7082.5	82 n	7086.5	114.0	
X1 270.0	4.0	0.1					114.0	
X2 170.0)	2002.0	20.0					
GR7087.0	4.0	7083.0	114.0	25.0	25.0	7087.0 25.0	114.0	
X2 170.0)							
GR7089.0	0.1	7084.0	32.0	7084.0	82.0	7089.0	114.0	
X1 395.0 X2 170.0		0.1	114.0	100.0	100.0	100.0		
GR7092.0	0.1	7088.0		7088.0	82.0	7092.0	114.0	
X1 495.0 X2 150.0		0.1	114.0	100.0	100.0	100.0		
		7094.1	32.0	7094.1	82.0	7098.1	114.0	
X1 595.0	4.0	7094.1	114.0	100.0	100.0	100.0		
X2 80.0		7000 0	32.0	7000 0	00.0	7100 0		
X1 605.0	4.0	7099.9 0.1	114.0	7099.9	10.0	10.0	114.0	
X2 36.0								
GR7103.9	0.1	7099.92 0.1	32.0	7099.92	82.0	7103.9	114.0	
X1 612.0 X2 36.0								
GR7103.9	0.1	7099.93	28.0	7099.93	71.0	7103.9	99.0	
X1 618.0 X2 36.0	4.0	0.1	99.0	6.0	6.0	6.0		
GR7103.9		7099.94	20.0	7099.94	56.0	7103.9	76.0	
X1 625.0		0.1	62.0	7099.94 7.0	7.0	7.0		
X2 36.0 GR7104.0		7099.96	16.0	7099 96	46.0	7104 0	76.0	
X1 633.0		0.1		8.0			70.0	
X2 36.0 GR7105.0		7000 07		7000 00				
X1 641.0		7099.97 0.1	66.0	8.0	8.0	8.0	60.0	
X2 36.0								
GR7107.0 X1 645.0	0.1	7099.98 0.1	28.0	7099.98	38.0	7107.0	66.0	
X2 36.0	4.0	0.1	61.0	4.0	4.0	4.0		
GR7107.0	0.1	7099.99 0.1	28.0	7099.99	33.0	7107.0	61.0	
X1 650.0 X2 36.0	3.0	0.1	80.0	5.0	5.0	5.0		
GR7110.0	0.1	7100.00	40.0	7110.00	80.0			
X1 660.0	3.0	7100.00 0.1	60.0	10.0	10.0	10.0		
X2 36.0 GR7110.0								
X1 670.0	3.0	7100.02	50.0	10.02	10.0	10.0		
X2 36.0								
GR7110.0 X1 680.0	3.0	7100.04	25.0	7110.04 10.0	50.0	10.0		
X2 36.0						10.0		
GR7110.1		7100.06						
X1 5.0 X2 36.0		0.1	52.0	25.0	25.0	25.0		
GR7113.1	0.1			7113.1	52.0			
X1 730.0			52.0	25.0	25.0	25.0		
X2 36.0 GR7113.2		7100.16	26.0	7113.2	52.0			
X1 755.0	4.0	0.1	105.0	25.0		25.0		
X2 36.0								
GR7110.2 EJ	0.1	7100.21	40.0	/100.21	65.0	7110.2	105.0	
ER								

TABLE F-10. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL M.

T2	CHANNEL		DES CROS	TAILINGS			HC4-5	
J1 J2 -1			1.0	-1.0			36.0	7105.0
NC 0.035	0.035			0.3				
X1 755.0 X2 36.0	4.0		105.0		25.0	25.0		
GR7110.2	0.1	7100.21	40.0	7100.21	65.0	7110.2	105.0	
X1 730 0	3.0	0.1	52.0	7100.21 25.0	25.0	25.0	100.0	
X2 36.0)	0.1	02.0	23.0	20.0	20.0		
GR7113.2		7100.16	26.0	7113.2	52.0			
X1 5.0	3.0	0.1	52.0	25.0		25.0		
X2 36.0								
GR7113.1	0.1	7100.11	26.0	7113.1	52.0			
X1 680.0	3.0	0.1	40.0	10.0	10.0	10.0		•
X2 36.0								
GR7110.1	0.1	7100.06	20.0	7110.06	40.0			
X1 670.0	3.0	0.1	50.0	10.0	10.0	10.0		-
X2 36.0								
GR7110.0	0.1	7100.04	25.0	7110.04	50.0			
X1 660.0	3.0	0.1	60.0	10.0	10.0	10.0		
X2 36.0	٠							
GR7110.0	0.1	7100.02	30.0	7110.02 5.0	60.0	5.0		
X1 650.0	3.0	0.1	80.0	5.0	5.0	5.0		
X2 36.0		7100 00	40.0	7110 00	90 0			
X1 645.0	4.0	0.1	40.0 61 0	7110.00 4.0	4.0	4.0		
		0.1	01.0	4.0	4.0	4.0		•
X2 36.0 GR7107.0		7099 99	-28 0	7099 99	33.0	7107.0	61.0	
X1 641.0	4.0	0.1	66 0	7099.99 8.0	8.0	8.0	01.0	
X2 36.0		0.1	00.0	0.0	0.0	. 0.0		
GR7107.0	0.1	7099 98	28.0	7099 98	38.0	7107.0	66.0	
GR7107.0 X1 633.0	4.0	0.1	60.0	7099.98 8.0	8.0	8.0		
X2 36.0		• • • •	• • • •					
GR7105.0		7099.97	20.0	7099.97	40.0	7105.0	60.0	
X1 625.0		0.1		7.0	7.0			
X2 36.0								
GR7104.0	0.1	7099.96	16.0	7099.96	46.0	7104.0	76.0	
X1 618.0	4.0	0.1		6.0	6.0			
X2 36.0								
GR7103.9	0.1	7099.94	20.0	7099.94	56.0	7103.9	76.0	
X1 612.0	4.0	0.1	99.0	7.0	7.0	7.0		
X2 36.0								
GR7103.9	0.1	7099.93	28.0	7099.93 10.0	71.0	7103.9	99.0	
		0.1	114.0	10.0	10.0	10.0		
X2 36.0		7000 00	20.0	7000 00		7100 0	•••	
GR7103.9	0.1	7099.92 0.1	32.0	7099.92 100.0	100.0	103.9	·114.0	
X1 595.0 X2 80.0	4.0	0.1	114.0	100.0	100.0	100.0		
GR7103.9	0.1	7099 9	32 0	7099.9	82 N	7103 9	114.0	
X1 495.0	4.0	0 1		100.0			114.0	
X2 150.0		0.1	22110	100.0	100.0	100.0		
GR7098.1	0.1	7094.1	32.0	7094.1	82.0	7098.1	114.0	
GR7098.1 X1 395.0	4.0	0.1	114.0	7094.1 100.0	100.0	100.0		
X2 170.0								
GR7092.0	0.1	7088.0	32.0	7088.0	82.0	7092.0	114.0	
X1 6.0	4.0	0.1	114.0	7088.0 25.0	25.0	25.0		
X2 170.0								
GR7089.0	0.1	7084.0	32.0	7084.0	82.0	7089.0	114.0	
X1 270.0	4.0	0.1	114.0	70.0	70.0	70.0		
X2 170.0								
GR7087.0	0.1	7083.0	32.0	7083.0		7087.0	114.0	
X1 200.0	4.0	0.1	114.0	100.0	100.0	100.0		
X2 170.0	_							
GR7086.5	0.1	7082.5	32.0	7082.5		7086.5	114.0	
X1 100.0	4.0	0.1	114.0	100.0	100.0	100.0		•
X2 170.0	٠.	7001 7	32.0	7001 7	02.0	7005 7	114 0	
GR7085.7 EJ	0.1	7081.7	32.0	7081.7	82.0	7085.7	114.0	
ER								

TABLE F-11. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL N.

T 2	CHANNEL	DER SHIRLE N - INCLU	DES CRO				HC5-5	
T3 J1	PMF - SU	BCRITICAL		-1.0			229.0	7080.6
J2 -1		-1.0		1.0			223.0	
NC 0.035 X180.0	0.035 4.0			0.3				
X2 229.0		0.01	70.00					
GR7080.0		7075.00				7080.00	70.00	
X1100.0 X2 229.0		0.01	70.00	20.00	20.00	20.00		
GR7081.2		7076.20		7076.20		7081.20	70.00	
X1 110 X2 229.0		0.01	70.00	10.00	10.00	10.00		
GR7081.6		7076.63	20.00	7076.63	50.00	7081.63	70.00	
X15.0	4.0	0.01			20.00			
X2 229.0 GR7082.5		7077.50	20.00	7077.50	50.00	7082.50	70.00	
X1140.0	4.0	0.01	70.00					
X2 229.0 GR7083.3		7078.25	20.00	7070 25	FO 00	7083.25	70.00	
X1190.0	4.0		70.00				70.00	
X2 229.0								
GR7084.0 X1 200		7079.00 0.01		10.00		7084.00 10.00	70.00	
X2 212.7			•					
GR7084.3 X1220.0		7079.33 0.01		7079.33		7084.33	70.00	
X2 180.0		0.01	70.00	20.00	20.00	20.00		
GR7085.0		7080.00		7080.00		7085.00	70.00	
X1 230 X2 180.0	4.0	0.01	68.33	10.00	10.00	10.00		
GR7085.5	0.01	7080.50				7085.50	68.33	
X1 240 X2 180.0	4.0	0.01	66.67	10.00	10.00	10.00		
GR7086.0		7081.00	20.00	7081.00	46.67	7086.00	66.67	
X1250.0		0.01	65.00	10.00	10.00	10.00		
X2 180.0 GR7086.5		7081.50	20.00	7081.50	45.00	7086.50	65.00	
X1 270	4.0			20.00				
X2 180.0 GR7087.4		7082.40	30.00	7002 40	. 45 00	7087.40	65.00	
X1 290	4.0			20.00		20.00	65.00	
X2 180.0								
GR7088.3 X1 310	4.0	7083.30 0.01		20:00		20.00	65.00	
X2 180.0								
GR7089.2 X1 330	0.01 4.0	7084.20 0.01		7084.20		7089.20	65.00	
X2 180.0		0.01	05.00	20.00	20.00	20.00		
GR7090.1		7085.10		7085.10		7090.10	65.00	
X1350.0 X2 180.0	4.0	0.01	65.00	20.00	20.00	20.00		
GR7091.0	0.01			7086.00		7091.00	65.00	
X1 365 X2 180.0	-4.0	0.01	65.00	15.00	15.00	15.00		
GR7092.2		7087.24	20.00	7087.24	45.00	7092.24	65.00	
X14.0 X2 180.0	4.0	0.01	65.00	20.00	20.00	20.00		
GR7093.9	0.01	7088.90	20.00	7088.90	45.00	7093.90	65.00	
X1395.0 X2 180.0		0.01	65.00	10.00	10.00	10.00		
		7090.00	20.00	7090.00	45.00	7095.00	65.00	
X1400.0				5.00	5.00			
X2 180.0 GR7095.2	0.01	7090.20	20.00	7090.20	40.00	7095 20	60.00	
X1 402	4.0			2.00			30.00	
X2 180.0 GR7095.4		7090.36	10.50	7090.36	20.50	2005 26	50.00	
X1 404	4.0		56.00	2.00		7095.36 2.00	58.00	
X2 180.0								
GR7095.5 X1 406	0.01 4.0	7090.52 0.01	19.00 54.00	7090.52 2.00		7095.52	56.00	
X2 180.0								
GR7095.7 X1 408				7090.68			54.00	
X2 180.0	4.0			2.00				
GR7095.8	0.01	7090.84 0.01	18.00	7090.84	34.00	7095.84	52.00	
X1410.0 X2 180.0	4.0	0.01	50.00	2.00	2.00	2.00		

TABLE F-11. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL N (continued). GR7096.0 0.01 7091.00 17.50 7091.00 32.50 7096.00 50.00 4.0 0.01 48.00 2.00 2.00 2.00 X1 412 X2 180.0 GR7096.1 0.01 7091.10 17.00 7091.10 31.00 7096.10 X1 414 4.0 0.01 46.00 2.00 2.00 2.00 X2 180.0 0.01 7091.20 16.50 7091.20 29.50 7096.20 GR7096.2 46.00 4.0 0.01 44.00 2.00 X1 416 2.00 X2 180.0 GR7096.3 0.01 7091.30 16.00 7091.30 28.00 7096.30 44.00 2.00 X1 418 4.0 0.01 42.00 2.00 X2 180.0 15.50 7091.40 0.01 7091.40 26.50 7096.40 GR7096.4 42.00 4.0 0.01 40.00 2.00 X1420.0 2.00 2.00 X2 180.0 GR7096.5 0.01 7091.50 15.00 7091.50 25.00 7096.50 40.00 X1430.0 4.0 0.01 35.00 10.00 10.00 10.00 X2 180.0 GR7097.0 15.00 7092.00 0.01 7092.00 20.00 7097.00 35.00 30.00 10.00 10.00 10.00 X1440.0 0.01 X2 180.0 0.01 7092.50 15.00 7092.50 15.00 7097.50 GR7097.5 30.00 X1445.0 25.00 5.00 4.0 0.01 5.00 5.00 X2 180.0 GR7097.8 0.01 7092.80 12.50 7092.80 12.50 7097.80 25.00 X1450.0 4.0 0.01 20.00 5.00 5.00 5.00 X2 180.0 GR7098.0 0.01 7093.00 10.00 7093.00 10.00 7098.00 20.00 20.00 25.00 X13.0 4.0 0.01 25.00 25.00 X2 180.0 GR7098.0 10.00 7093.05 0.01 7093.05 10.00 7098.05 20.00 X1500.0 0.01 20.00 25.00 25.00 25.00 4.0 X2 180.0 GR7098.1 0.01 7093.10 10.00 7093.10 10.00 7098.10 20.00 X1 503 X2 180.0 4.0 0.01 25.19 3.00 3.00 GR7098.1 0.01 7093.11 11.15 7093.11 14.04 7098.11 25.19 X1 508 0.01 33.85 5.00 X2 180.0 20.77 7098.12 0.01 7093.12 13.08 7093.12 GR7098.1 33.85 X1513.0 4.0 0.01 42.50 5.00 5.00 5.00 X2 180.0 0.01 7093.13 GR7098.1 15.00 7093.13 27.50 7098.13 42.50 48.75 X1 515 4.0 0.01 2.00 2.00 2.00 X2 180.0 0.01 7093.13 GR7098.1 15.83 7093.13 32.92 7098.13 48.75 0.01 X1 520 4.0 64.38 5.00 5.00 5.00 X2 180.0 0.01 7093.15 17.92 7093.15 GR7098.1 46.46 7098.15 64.38 X1525.0 4.0 0.01 80.00 5.00 5.00 5.00 X2 180.0 GR7098.2 0.01 7093.16 20.00 7093.16 60.00 7098.16 80.00 X1600.0 4.0 0.01 80.00 75.00 75.00 75.00 X2 450.0 GR7099.0 0.01 7094.00 20.00 7094.00 60.00 7099.00 80.00 0.01 X1 680 4.0 80.00 80.00 80.00 80.00 X2 649.7 GR7099.1 0.01 7094.14 20.00 7094.14 60.00 7099.14 80.00 0.01 80.00 100.00 X1 780 4.0 100.00 100.00 X2 899.4 0.01 7094.32 20.00 7094.32 60.00 7099.32 GR7099.3 80.00 X12.0 4.0 0.01 80.00 100.00 100.00 100.00 X21149.0 0.01 7094.50 20.00 7094.50 60.00 7099.50 GR7099.5 80.00 X1 885 4.0 81,25 5.00 0.01 5.00 5.00 X21149.0 GR7099.5 0.01 7094.52 20.47 7094.52 60.78 7099.52 X1 890 4.0 0.01 82.50 5.00 5.00 5.00 X21149.0 0.01 7094.53 GR7099.5 20.94 7094.53 61.56 7099.53 82.50 X1 895 4.0 0.01 83.75 5.00 5.00 5.00 X21149.0 GR7099.5 0.01 7094.55 21.41 7094.55 62.34 7099.55 X1 900 4.0 0.01 85.00 5.00 5.00 5.00 X21149.0 0.01 7094.56 21.88 7094.56 63.13 7099.56 GR7099.6 85.00 X1 905 4.0 0.01 86.25 5.00 5.00 5.00 X21149.0

GR7099.6

X1 910

0.01 7094.58

0.01

4.0

22.34 7094.58

5.00

87.50

63.91 7099.58

5.00

5.00

TABLE F-11. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL N (continued).

X21149.0 GR7099.6	0.01 7094.59	22.81 7094.59	64.69 7099.59	87.50
X1 915	4.0 0.01	88.75 5.00		
X21149.0 GR7099.6	0.01 7094.61	23.28 7094.61	65.47 7099.61	88.75
X1 920	4.0 0.01	90.00 5.00	5.00 5.00	
X21149.0 GR7099.6	0.01 7094.63	23.75 7094.63	66.25 7099.63	90.00
X1 925	4.0 0.01	91.25 5.00		20.00
X21149.0 GR7099.6	0.01 7094.64	24.22 7094.64	67.03 7099.64	91.25
X1 930	4.0 0.01	92.50 5.00		71.20
X21149.0 GR7099.7	0.01 7094.66	24.69 7094.66	67.81 7099.66	92.50
X1 935	4.0 0.01	93.75 5.00		72.30
X21149.0 GR7099.7	0.01 7094.67	25.16 7094.67	68.59 7099.67	93.75
X1 940	4.0 0.01	95.00 5.00	5.00 5.00	231,70
X21149.0 GR7099.7	0.01 7094.69	25.63 7094.69	69.38 7099.69	95.00
X1 945	4.0 0.01	96.25 5.00	5.00 5.00	23.00
X21149.0 GR7099.7	0.01 7094.70	26.09 7094.70	70.16 7099.70	96.25
X1 950		97.50 5.00	5.00 5.00	30123
X21149.0 GR7099.7	0.01 7094.72	26.56 7094.72	70.94 7099.72	97.50
X1 955	4.0 0.01	98.75 5.00	5.00 5.00	330
X21149.0 GR7099.7	0.01 7094.73	27.03 7094.73	71.72 7099.73	98.75
X1 960	4.0 0.01	100.00 5.00	5.00 5.00	90.75
X21149.0 GR7099.8	0.01 7094.75	27.50 7094.75	70 60 7000 76	100.00
X1 965		101.25 5.00	72.50 7099.75 5.00 5.00	100.00
X21149.0 GR7099.8	0.01 7094.77	27 67 7664 77	73.28 7099.77	101 05
X1 970	4.0 0.01	27.97 7094.77 102.50 5.00	5.00 5.00	101.25
X21149.0	0.01 7094.78	20 44 7004 70	7. 06 7000 70	
GR7099.8 Xl 975	4.0 0.01	28.44 7094.78 103.75 5.00	74.06 7099.78 5.00 5.00	102.50
X21149.0	5 61 7664 66		7. 0. 7000 00	
GR7099.8 X1 980	0.01 7094.80 4.0 0.01	28.91 7094.80 105.00 5.00	74.84 7099.80 5.00 5.00	103.75
X21149.0	6 01 7004 01	20 20 2001 01		
GR7099.8 X1 985	0.01 7094.81 4.0 0.01	29.38 7094.81 106.25 5.00	75.63 7099.81 5.00 5.00	105.00
X21149.0	0.01.2004.00	** ** ***		
GR7099.8 X1 990	0.01 7094.83 4.0 0.01	29.84 7094.83 107.50 5.00	76.41 7099.83 5.00 5.00	106.25
X21149.0 GR7099.8	0.01 7094.84	20 21 7004 04	77 10 7000 01	107 -0
X1 995	4.0 0.01	30.31 7094.84 108.75 5.00	77.19 7099.84 5.00 5.00	107.50
X21149.0 GR7099.9	0.01.7004.06	20 70 7004 05	72 07 7000 04	
X1 1000	0.01 7094.86 4.0 0.01	30.78 7094.86 110.00 5.00	77.97 7099.86 5.00 5.00	108.75
X21149.0 GR7099.9	0.01 7094.88	31 25 7004 00	70 75 7000 00	
X1 1005		31.25 7094.88 111.25 5.00	76.75 7099.88 5.00 5.00	110.00
X21149.0 GR7099.9	0.01 7094.89	31.72 7094.89	79,53 7099.89	111 05
X1 1010	4.0 0.01		5.00 5.00	111.25
X21149.0 GR7099.9	0.01 7094.91	32.19 7094.91	80.31 7099.91	110 50
X1 1015	4.0 0.01	113.75 5.00	5.00 5.00	112.50
X21149.0 GR7099.9	0.01 7094.92	32.66 7094.92	61 00 7000 00	112 25
X1 1020	4.0 0.01	115.00 5.00	81.09 7099.92 5.00 5.00	113.75
X21149.0 GR7099.9	0.01 7094.94	22 12 7004 04		
X1 1025	4.0 0.01	33.13 7094.94 116.25 5.00	81.88 7099.94 5.00 5.00	115.00
X21149.0	0.01.7004.05			
GR7100.0 X1 1030	0.01 7094.95 4.0 0.01	33.59 7094.95 117.50 5.00	82.66 7099.95 5.00 5.00	116.25
X21149.0				
GR7100.0 X1 1035	0.01 7094.97 4.0 0.01	34.06 7094.97 118.75 5.00	83.44 7099.97 5.00 5.00	117.50
X21149.0				
GR7100.0 X1 1040	0.01 7094.98 4.0 0.01	34.53 7094.98 120.00 5.00	84.22 7099.98 5.00 5.00	118.75
X21149.0				
GR7100.0	0.01 7095.00	35.00 7095.00	85.00 7100.00	120.00

TABLE F-11. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL N (continued). 0.01 121.25 X1 1045 4.0 5.00 5.00 5.00 X21149.0 GR7100.0 0.01 7095.02 35.47 7095.02 85.78 7100.02 121.25 X1 1050 4.0 0.01 122.50 5.00 5.00 X21149.0 35.94 7095.03 0.01 7095.03 86.56 7100.03 122.50 GR7100.0 4.0 0.01 123.75 5.00 5.00 X1 1055 5.00 X21149.0 GR7100.0 0.01 7095.05 36.41 7095.05 87.34 7100.05 123.75 X1 1060 4.0 0.01 125.00 5.00 5.00 5.00 X21149.0 0.01 7095.06 36.88 7095.06 88.13 7100.06 125.00 GR7100.1 4.0 0.01 126.25 5.00 X1 1065 5.00 5.00 X21149.0 GR7100.1 0.01 7095.08 37.34 7095.08 88.91 7100.08 126.25 X1 1070 4.0 0.01 127.50 5.00 5.00 5.00 X21149.0 GR7100.1 0.01 7095.09 37.81 7095.09 89.69 7100.09 127.50 4.0 0.01 128.75 5.00 X1 1075 5.00 5.00 X21149.0 GR7100.1 0.01 7095.11 38.28 7095.11 90.47 7100.11 128.75 4.0 0.01 130.00 5.00 X1 1080 5,00 5.00 X21149.0 GR7100.1 0.01 7095.13 38.75 7095.13 91.25 7100.13 130.00 X1 1085 0.01 131.25 5.00 5.00 4.0 5.00 X21149.0 0.01 7095.14 GR7100.1 39.22 7095.14 92.03 7100.14 131.25 4.0 0.01 132.50 5.00 5.00 X1 1090 5.00 X21149.0 0.01 7095.16 39.69 7095.16 92.81 7100.16 132.50 GR7100.2 X1 1095 4.0 0.01 133.75 5.00 5.00 X21149.0 0.01 7095.17 40.16 7095.17 93.59 7100.17 133.75 GR7100.2 X1 1100 4.0 0.01 135.00 5.00 5.00 5.00 X21149.0 GR7100.2 0.01 7095.19 40.63 7095.19 94.38 7100.19 135.00 X1 1105 4.0 0.01 136.25 5.00 5.00 5.00 X21149.0 95.16 7100.20 136.25 GR7100.2 0.01 7095.20 41.09 7095.20 5.00 0.01 137.50 5.00 X1 1110 4.0 5.00 X21149.0 GR7100.2 0.01 7095.22 41.56 7095.22 95.94 7100.22 137.50 0.01 138.75 5.00 5.00 X1 1115 5.00 X21149.0 42.03 7095.23 GR7100.2 0.01 7095.23 96,72 7100.23 138.75 0.01 140.00 5.00 X1 1120 4.0 5.00 5.00 X21149.0 GR7100.3 0.01 7095.25 42.50 7095.25 97.50 7100.25 140.00 4.0 0.01 141.25 5.00 X1 1125 5.00 X21149.0 GR7100.3 0.01 7095.27 42.97 7095.27 98.28 7100.27 141.25 X1 1130 4.0 0.01 142.50 5.00 5.00 5.00 X21149.0 0.01 7095.28 43.44 7095.28 99.06 7100.28 142.50 GR7100.3 4.0 0.01 143.75 5.00 5.00 5.00 X1 1135 X21149.0 GR7100.3 0.01 7095.30 43.91 7095.30 99.84 7100.30 143.75 X1 1140 4.0 0.01 145.00 5.00 5.00 5.00 X21149.0 GR7100.3 0.01 7095.31 44.38 7095.31 100.63 7100.31 145.00 4.0 0.01 146.25 5.00 X1 1145 5.00 5.00 X21149.0 GR7100.3 0.01 7095.33 44.84 7095.33 101.41 7100.33 146.25 X1 1150 X21149.0 4.0 0.01 147.50 5.00 5,00 5.00 GR7100.3 0.01 7095.34 45.31 7095.34 102.19 7100.34

5.00

45.78 7095.36 102.97 7100.36 148.75

46.25 7095.38 103.75 7100.38

46.72 7095.39 104.53 7100.39

5.00

5.00

5.00

5.00

47.19 7095.41 105.31 7100.41 152.50

5.00

5.00

5.00

5.00

X1 1155

X21149.0

GR7100.4

X21149.0 GR7100.4

X1 1160

X1.1165

X1 1170

X21149.0

GR7100.4

X1 1175

X21149.0

X21149.0 GR7100.4 4.0 0.01 148.75 5.00

4.0 0.01 150.00 5.00

4.0 0.01 151.25 5.00

0.01 152.50

4.0 0.01 153.75 5.00

0.01 7095.36

0.01 7095.38

0.01 7095.39

0.01 7095.41

4.0

151.25

TABLE F-11. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL N (continued).

GR7100.4		7095.42 0.01		7095.42		7100.42	153.75
X1 1180	4.0	0.01	155.00	5.00	5.00	5.00	
X21149.0							
		7095.44		7095.44		7100.44	155.00
X1 1185	4.0	0.01	156.25	5.00	5.00	5.00	
X21149.0							
GR7100.5	0.01	7095.45	48.59	7095.45	107.66	7100.45	156.25
X1 1190	4.0	0.01	157.50	5.00	5.00	5.00	
X21149.0							
GR7100.5	0.01	7095.47	49.06	7095.47	108.44	7100.47	157.50
X1 1195	4.0		158.75	5.00	5.00		
X21149.0	1.0	0.01	100.70	0.00	5.00	0,00	
GR7100.5	0 01	7095.48	40 53	7095.48	109 22	7100.48	158.75
							130.73
X11200.0	4.0	0.01	160.00	5.00	5.00	5.00	
X21149.0							
		7095.50		7095.50		7100.50	160.00
X1 1300	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0							
GR7100.7	0.01	7095.67	50.00	7095.67	110.00	7100.67	160.00
X1 1400	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0							
GR7100.8	0.01	7095.83	50.00	7095.83	110.00	7100.83	160.00
X1 1500	4.0					100.00	
X21149.0		0.01	200.00	100.00	100.00	100.00	
GR7101.0	0.01	7096.00	E0 00	7096.00	110 00	7101.00	160.00
							160.00
X1 1600	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0							
GR7101.2		7096.17		7096.17		7101.17	160.00
X1 1700	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0							
GR7101.3	0.01	7096.33	50.00	7096.33	110.00	7101.33	160.00
X11800.0	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0							
GR7101.5	0.01	7096.50	50.00	7096.50	110 00	7101.50	160.00
X1 1850	4.0		160.00	50.00	50.00	50.00	100.00
	4.0	0.01	180.00	30.00	30.00	50.00	
X21149.0		2026 66		2004 55			
GR7101.6		7096.56		7096.56		7101.56	160.00
X1 1950	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0			•	-			
GR7101.7	0.01	7096.67	50.00	7096.67	110.00	7101.67	160.00
X1 2050	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0							
GR7101.8	0.01	7096.78	50.00	7096.78	110.00	7101.78	160.00
X1 2150		0.01		100.00	100.00	100.00	
X21149.0		0.01	100.00	200.00	100.00	100.00	
GR7101.9	0.01	7096.89	50.00	7096.89	110 00	7101.89	160.00
X11.0	4.0	0.01	160.00	100.00	100.00	100.00	100.00
	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0		2002 00	50.00	2002 '00			
GR7102.0		7097.00		7097.00		7102.00	160.00
X1 2300	4.0	0.01	160.00	50.00	50.00	50.00	
X21149.0							
GR7102.1	0.01	7097.07	50.00	7097.07	110.00	7102.07	160.00
X1 2400	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0							
GR7102.2	0.01	7097.20	50.00	7097.20	110.00	7102.20	160.00
X1 2500	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0			4				
GR7102.3	0 01	7097.33	'60 00	7097.33	110 00	7102.33	160.00
X1 2600	4.0	0.01					100.00
	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0							
GR7102.5		7097.47		7097.47		7102.47	160.00
X1 2700	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0							
GR7102.6	0.01	7097.60	50.00	7097.60	110.00	7102.60	160.00
X1 2800	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0	· · · ·			•			
GR7102.7	0.01	7097.73	50.00	7097.73	110 00	7102.73	160.00
X1 2900	4.0	0.01	160.00	100.00			100.00
X21149.0	٦.٠	0.01	100.00	100.00	100.00	100.00	
	0.01	2007 07	E0 00	7007 07	110 00	7100 07	160.00
GR7102.9		7097.87		7097.87		7102.87	160.00
X13000.0	4.0	0.01	160.00	100.00	100.00	100.00	
X21149.0							
GR7103.0	0.01	7098.00	50.00	7098.00	110.00	7103.00	160.00
EJ							
ER							

TABLE F-12. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL N.

T23	T1 T2					RECLAMATION	CU UCS-5	
1.0 -					33 35011	MS NOS-I INNOC	GH NC3-3	
NC 0.035					-1.0		1149.0	7101.4
X13100.0								
X21149.0 CR7102.0 C.01 7095.00 50.00 7098.00 110.00 7103.00 160.00 X1 2900 X.0						100 00 100 0	0	
CRT102.0.0 0.01 7095.00 50.00 7098.00 110.00 7103.00 160.00 X21149.0 CRT102.9 0.01 7097.87 50.00 7097.87 110.00 7102.87 160.00 X21149.0 CRT102.7 CRT102.5 CRT102.5 CRT102.5 CRT102.5 CRT102.5 CRT102.5 CRT102.5 CRT102.7 CRT			0.01	160.00	100.00	100.00 100.0	O	
X21149.0 CR7102.7 C.01 7097.87 50.00 7097.87 110.00 7102.87 160.00 X21149.0 C.01 7097.73 50.00 7097.73 110.00 7102.73 160.00 X21149.0 C.01 7097.60 50.00 7097.60 110.00 7102.73 160.00 X21149.0 C.01 7097.47 50.00 7097.47 110.00 7102.47 160.00 X21149.0 C.01 7097.47 S0.00 7097.47 110.00 7102.47 160.00 X21149.0 C.01 7097.47 S0.00 7097.47 110.00 7102.43 160.00 X21149.0 C.01 7097.20 50.00 7097.33 110.00 7102.33 160.00 X21149.0 C.01 7097.20 50.00 7097.20 110.00 7102.27 160.00 X21149.0 C.01 7097.20 50.00 7097.20 110.00 7102.27 160.00 X21149.0 C.01 7097.20 50.00 7097.20 110.00 7102.27 160.00 X21149.0 C.01 7097.07 50.00 7097.07 110.00 7102.07 160.00 X21149.0 C.01 7097.07 50.00 7097.07 110.00 7102.07 160.00 X21149.0 C.01 7097.08 C.01 160.00 100.00 100.00 100.00 X21149.0 C.01 7097.08 C.00 7097.07 110.00 7102.07 160.00 X21149.0 C.01 7097.08 C.00 7097.09 C.01 160.00 100.00 100.00 100.00 X21149.0 C.01 7097.08 C.00 7097.09 C.01 160.00 C.00 C			7098.00	50.00	7098.00	110.00 7103.0	0 160.00	
CRT102.9 0.01 7097.87 50.00 7097.87 110.00 7102.87 160.00 X21149.0 GR7102.7 160.00 100.00 100.00 100.00 100.00 X21149.0 GR7102.5 0.01 7097.60 50.00 7097.73 110.00 7102.73 160.00 X21149.0 GR7102.5 0.01 7097.60 50.00 7097.60 110.00 7102.60 160.00 X21149.0 GR7102.3 0.01 7097.47 50.00 7097.47 110.00 7102.47 160.00 X21149.0 GR7102.3 0.01 7097.33 50.00 7097.33 110.00 7102.33 160.00 X21149.0 GR7102.3 0.01 7097.33 50.00 7097.33 110.00 7102.33 160.00 X21149.0 GR7102.1 0.01 7097.07 50.00 7097.07 110.00 7102.20 160.00 X21149.0 GR7102.1 0.01 7097.07 50.00 7097.07 110.00 7102.00 160.00 X21149.0 GR7102.1 0.01 7097.07 50.00 7097.07 110.00 7102.00 160.00 X21149.0 GR7101.9 0.01 7096.89 50.00 7096.89 110.00 7101.89 160.00 X21149.0 GR7101.9 0.01 7096.89 50.00 7096.89 110.00 7101.89 160.00 X21149.0 GR7101.7 0.01 7096.67 50.00 7096.78 110.00 7101.78 160.00 X21149.0 GR7101.7 0.01 7096.67 50.00 7096.78 110.00 7101.56 160.00 X21149.0 GR7101.5 0.01 7096.67 50.00 7096.67 110.00 7101.56 160.00 X21149.0 GR7101.5 0.01 7096.67 50.00 7096.67 110.00 7101.56 160.00 X21149.0 GR7101.5 0.01 7096.50 50.00 7096.50 110.00 7101.56 160.00 X21149.0 GR7101.5 0.01 7096.50 50.00 7096.50 110.00 7101.56 160.00 X21149.0 GR7101.5 0.01 7096.50 50.00 7096.50 110.00 7101.50 160.00 X21149.0 GR7101.5 0.01 7096.50 50.00 7096.50 110.00 7101.50 160.00 X21149.0 GR7101.5 0.01 7096.50 50.00 7096.50 110.00 7101.50 160.00 X21149.0 GR7101.5 0.01 7096.50 50.00 7096.50 110.00 7101.50 160.00 X21149.0 GR7101.5 0.01 7096.50 50.00 7096.50 110.00 7101.50 160.00 X21149.0 GR7101.5 0.01 7096.50 50.00 7096.50 110.00 7101.50 160.00 X21149.0 GR7101.5 0.01 7096.50 50.00 7096.50 110.00 7101.50 160.00 X21149.0 GR7101.5 0.01 7096.50 50.00 7096.50 110.00 7101.50 160.00 X21149.0 GR			0.01	160.00	100.00	100.00 100.0	0	
X1 12800								
X21149.0 GR7102.7 0.01 7097.73 50.00 7097.73 110.00 7102.73 160.00 X21149.0 GR7102.5 0.01 7097.60 50.00 7097.60 110.00 7102.60 160.00 X21149.0 GR7102.5 X1 2500 X21149.0 GR7102.2 X1 2600 X21149.0 GR7102.2 X1 2600 X21149.0 GR7102.3 X1 2600 X21149.0 GR7102.2 X1 2500 X21149.0 GR7102.0 X21149.0 GR7101.9 X1 2500 X21149.0 GR7101.0 X1 1550 X1 1550 X1 1550 X1 1550 X1 1550 X1 1500 X21149.0 GR7101.5 X1 1550 X1 1500								
GRT102.7			0.01	100.00	100.00	100.00 100.0	U	
X1 1700			7097.73	50.00	7097.73	110.00 7102.7	3 160.00	
SRT102.6	X1 2700	4.0	0.01	160.00	100.00	100.00 100.0		
X1 12600								
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GR7102.5			0.01	100.00	100.00	100.00 100.0	U	
X1 2500			7097.47	50.00	7097.47	110.00 7102.4	7 160.00	
SR7102.3								
X12400								
X21149.0 CR7102.2 O.01 7097.20 50.00 7097.20 110.00 7102.20 160.00 X21149.0 CR7102.1 O.01 7097.07 S0.00 7097.07 110.00 7102.07 160.00 X21149.0 CR7102.1 O.01 7097.00 S0.00 7097.07 110.00 7102.00 160.00 X21149.0 CR7102.0 O.01 7097.00 S0.00 7097.00 110.00 7102.00 160.00 X21149.0 CR7101.9 O.01 7096.89 50.00 7096.89 110.00 7101.89 160.00 X21149.0 CR7101.9 O.01 7096.78 S0.00 7096.89 110.00 7101.89 160.00 X21149.0 CR7101.9 O.01 7096.78 S0.00 7096.78 110.00 7101.78 160.00 X21149.0 CR7101.8 O.01 7096.78 S0.00 7096.78 110.00 7101.78 160.00 X21149.0 CR7101.8 O.01 7096.67 S0.00 7096.78 110.00 7101.78 160.00 X21149.0 CR7101.7 O.01 7096.67 S0.00 7096.78 110.00 7101.67 160.00 X21149.0 CR7101.6 O.01 7096.67 S0.00 7096.67 110.00 7101.67 160.00 X21149.0 CR7101.6 O.01 7096.56 S0.00 7096.56 110.00 7101.56 160.00 X21149.0 CR7101.5 O.01 7096.56 S0.00 7096.56 110.00 7101.55 160.00 X21149.0 CR7101.5 O.01 7096.50 S0.00 7096.50 I10.00 7101.50 160.00 X21149.0 CR7101.3 O.01 7096.33 S0.00 7096.50 I10.00 7101.50 160.00 X21149.0 CR7101.3 O.01 7096.33 S0.00 7096.33 I10.00 7101.33 I60.00 X21149.0 CR7101.2 O.01 7096.33 S0.00 7096.33 I10.00 7101.33 I60.00 X21149.0 CR7101.2 O.01 7096.33 S0.00 7096.33 I10.00 7101.00 I60.00 X21149.0 CR7101.2 O.01 7096.00 S0.00 7096.00 I10.00 7101.00 I60.00 X21149.0 CR7100.2 O.01 7096.00 S0.00 7096.00 I10.00 7100.00 X21149.0 CR7100.2 O.01 7095.87 S0.00 7095.87 I10.00 7100.83 I60.00 X21149.0 CR7100.5 O.01 7095.87 S0.00 7095.87 I10.00 7100.83 I60.00 X21149.0 CR7100.5 O.01 7095.87 S0.00 7095.87 I10.00 7100.80 I60.00 X21149.0 CR7100.5 O.01 7095.87 S0.00 7095.87 I10.00 7100.80 I60.00 X21149.0 CR7100.5 O.01 7095.47 A9.06 7095.47 I08.44 7100.47 I57.50 S0.00 X21149.0 CR7100.5 O.01 7095.47 A9.06 7095.47 I08.44 7100.47 I5								
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X1 2300			7097.20	50.00	7097.20	110.00 7102.2	0 160.00	
GR7102.1								
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X21149.0 CR7101.5 C.0.0 T097.00 50.00 7097.00 110.00 7102.00 160.00 X21149.0 CR7101.9 C.0.0 160.00 100.00 100.00 100.00 X21149.0 CR7101.8 C.0.0 T096.89 50.00 7096.89 110.00 7101.89 160.00 X21149.0 CR7101.8 C.0.0 7096.78 50.00 7096.78 110.00 7101.78 160.00 X21149.0 CR7101.7 C.0.0 7096.67 50.00 7096.67 110.00 7101.67 160.00 X21149.0 CR7101.7 C.0.0 7096.67 50.00 7096.67 110.00 7101.67 160.00 X21149.0 CR7101.6 C.0.0 160.00 50.00 50.00 50.00 X21149.0 CR7101.5 C.0.0 7096.56 50.00 7096.56 110.00 7101.67 160.00 X21149.0 CR7101.5 C.0.0 160.00 100.00 100.00 100.00 X21149.0 CR7101.5 C.0.0 7096.56 50.00 7096.56 110.00 7101.50 160.00 X21149.0 CR7101.5 C.0.0 7096.50 50.00 7096.50 110.00 7101.50 160.00 X21149.0 CR7101.3 C.0.0 7096.33 50.00 7096.50 110.00 7101.50 160.00 X21149.0 CR7101.3 C.0.0 7096.33 50.00 7096.33 110.00 7101.33 160.00 X21149.0 CR7101.2 C.0.0 7096.00 50.00 7096.00 100.00 100.00 X21149.0 CR7101.0 C.0.0 160.00 100.00 100.00 100.00 X21149.0 CR7100.8 C.0.0 C								
CR7101.5			0.01	160.00	100.00	100.00 100.0	0	
X1 2150 4.0 0.01 160.00 100.00 100.00 100.00 100.00 X21149.0 0.01 7096.89 50.00 7096.89 110.00 7101.89 160.00 X21149.0 0.01 7096.78 50.00 7096.78 110.00 7101.78 160.00 X1 1950 4.0 0.01 160.00 100.00 100.00 100.00 X1 1950 4.0 0.01 160.00 100.00 100.00 100.00 X1 1950 4.0 0.01 160.00 50.00 7096.67 110.00 7101.67 160.00 X1 1850 4.0 0.01 160.00 50.00 7096.56 110.00 7101.67 160.00 X21149.0 6R7101.5 0.01 7096.56 50.00 7096.56 110.00 7101.50 160.00 X1 1700 4.0 0.01 160.00 100.00 100.00 100.00 X21149.0 6R7101.3 0.01 7096.50 50.00 7096.33 110.00 7101.33 160.00 X1 1500 4.0 0.01			7097 00	50.00	7097.00	110 00 7102 0	160.00	
X21149.0 GR7101.9 0.01 7096.89 50.00 7096.89 110.00 7101.89 160.00 X112050 4.0 0.01 160.00 100.00 100.00 100.00 X21149.0 GR7101.8 4.0 0.01 160.00 100.00 100.00 100.00 X21149.0 GR7101.6 0.01 7096.67 50.00 7096.67 110.00 7101.67 160.00 X21149.0 GR7101.5 0.01 7096.56 50.00 7096.56 110.00 7101.56 160.00 X21149.0 GR7101.5 0.01 7096.56 50.00 7096.56 110.00 7101.56 160.00 X21149.0 GR7101.5 0.01 7096.50 50.00 7096.50 110.00 7101.50 160.00 X21149.0 GR7101.5 0.01 7096.50 50.00 7096.50 110.00 7101.50 160.00 X21149.0 GR7101.5 0.01 7096.50 50.00 7096.50 110.00 7101.50 160.00 X21149.0 GR7101.3 0.01 7096.33 50.00 7096.33 110.00 7101.33 160.00 X21149.0 GR7101.3 0.01 7096.33 50.00 7096.33 110.00 7101.33 160.00 X21149.0 GR7101.2 0.01 7096.17 50.00 7096.17 110.00 7101.7 160.00 X21149.0 GR7101.0 0.01 7096.17 50.00 7096.17 110.00 7101.17 160.00 X21149.0 GR7101.0 0.01 7096.00 50.00 7096.00 110.00 7101.00 160.00 X21149.0 GR7101.0 0.01 7096.00 50.00 7096.00 110.00 7101.00 160.00 X21149.0 GR7101.0 0.01 7096.00 50.00 7096.00 110.00 7101.00 160.00 X21149.0 GR7100.8 0.01 7095.83 50.00 7095.83 110.00 7100.83 160.00 X21149.0 GR7100.5 0.01 7095.67 50.00 7095.67 110.00 7100.83 160.00 X21149.0 GR7100.5 0.01 7095.84 49.53 7095.84 109.22 7100.48 158.75 X1 1190 4.0 0.01 158.75 5.00 5.00 5.00 5.00 X21149.0 GR7100.5 0.01 7095.48 49.53 7095.48 109.22 7100.48 158.75 X1 1190 4.0 0.01 158.25 5.00 5.00 5.00 5.00 X21149.0 GR7100.5 0.01 7095.47 49.06 7095.47 108.44 7100.47 157.50 X1 1180 4.0 0.01 158.25 5.00 5.00 5.00 5.00 5.00 X21149.0 GR7100.5 0.01 7095.47 49.06 7095.47 108.44 7100.47 157.50 X1 1180 4.0 0.01 155.05 5.00 5.00 5.00 5.00 5.00 5.00 X21149.0 GR7100.4 0.01 153.75 5.00 5.00 5.00 5.00 5.00 5.0								
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GR7101.8			0.01	160.00	100.00	100.00 100.0	0	
X1 1950			7096 78	50.00	7006 78	110 00 7101 7	160 00	
X21149.0 GR7101.7 X1 1850 4.0 0.01 7096.67 X1 10.00 7101.67 X1 1850 X21149.0 GR7101.6 CR7101.6								
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X21149.0 GR7101.6 0.01 7096.56 50.00 7096.56 110.00 7101.56 160.00 X11800.0 4.0 0.01 160.00 100.00 100.00 100.00 X21149.0 GR7101.5 4.0 0.01 160.00 100.00 100.00 100.00 X21149.0 GR7101.3 4.0 0.01 160.00 100.00 100.00 100.00 X21149.0 GR7101.3 0.01 7096.33 50.00 7096.33 110.00 7101.33 160.00 X21149.0 GR7101.2 0.01 7096.17 50.00 7096.17 110.00 7101.17 160.00 X21149.0 GR7101.2 0.01 7096.17 50.00 7096.17 110.00 7101.17 160.00 X21149.0 GR7101.0 4.0 0.01 160.00 100.00 100.00 100.00 X21149.0 GR7101.0 4.0 0.01 160.00 100.00 100.00 100.00 X21149.0 GR7100.8 0.01 7095.83 50.00 7095.83 110.00 7101.00 160.00 X21149.0 GR7100.7 0.01 7095.83 50.00 7095.83 110.00 7100.83 160.00 X21149.0 GR7100.7 0.01 7095.67 50.00 7095.67 110.00 7100.67 160.00 X21149.0 GR7100.7 0.01 7095.67 50.00 7095.67 110.00 7100.67 160.00 X21149.0 GR7100.5 0.01 7095.87 50.00 7095.67 110.00 7100.67 160.00 X21149.0 GR7100.5 0.01 7095.84 49.53 7095.48 109.22 7100.48 158.75 X1 1190 4.0 0.01 158.75 5.00 5.00 5.00 5.00 X21149.0 GR7100.5 0.01 7095.47 49.06 7095.47 108.44 7100.47 157.50 X1 1185 4.0 0.01 156.25 5.00 5.00 5.00 5.00 X21149.0 GR7100.5 0.01 7095.45 48.59 7095.45 107.66 7100.45 156.25 X1 1180 4.0 0.01 155.00 5.00 5.00 5.00 X21149.0 GR7100.5 0.01 7095.45 48.59 7095.45 107.66 7100.45 156.25 X1 1180 4.0 0.01 155.00 5.00 5.00 5.00 5.00 X21149.0 GR7100.4 0.01 7095.44 48.13 7095.44 106.88 7100.44 155.00 X21149.0 GR7100.4 0.01 7095.44 48.13 7095.44 106.88 7100.44 155.00 X21149.0 GR7100.4 0.01 7095.44 48.13 7095.44 106.88 7100.44 155.00 X21149.0 GR7100.4 0.01 7095.44 48.13 7095.44 106.88 7100.44 155.00 X21149.0 GR7100.4 0.01 1550.75 5.00 5.00 5.00 5.00 5.00 5.00 X21149.0 GR7100.4 0.01 1550.75 5.00 5		0.01	7096.67	50.00	7096.67	110.00 7101.6	160.00	
GR7101.6			0.01	160.00	50.00	50.00 50.0	ס	
X11800.0 X1149.0 X11			7006 56	E0 00	7006 56	110 00 7101 5	160.00	
X21149.0 GR7101.5 X1 1700 X1 1700 X21149.0 GR7101.3 X1 1700 X21149.0 GR7101.3 0.01 7096.33 50.00 7096.33 110.00 7101.33 160.00 X1 1600 X21149.0 GR7101.2 GR7101.2 GR7101.2 0.01 7096.17 50.00 7096.17 110.00 7101.33 160.00 X21149.0 GR7101.0 GR7101.0 GR7101.0 GR7101.0 GR7101.0 GR7101.0 GR7101.0 GR7101.0 GR7101.0 GR7101.0 GR7101.0 GR7101.0 GR7101.0 GR7101.0 GR7101.0 GR7100.1 GR7100.8 0.01 7095.83 50.00 7096.00 110.00 7101.00 160.00 X21149.0 GR7100.7 GR7100.8 0.01 7095.83 50.00 7095.83 110.00 7100.83 160.00 X21149.0 GR7100.7 GR7100.7 GR7100.7 GR7100.7 GR7100.5 GR7100.6 GR710								
GR7101.5			0.01	100.00	100.00	100.00 100.0	•	
X21149.0 GR7101.3	GR7101.5		7096.50	50.00	7096.50	110.00 7101.50	160.00	
GR7101.3			0.01	160.00	100.00	100.00 100.00)	
X1 1600			3000 00	50.00	2006 22			
X21149.0 GR7101.2 1500 4.0 0.01 160.00 100.0								
GR7101.2			0.01	100.00	100.00	100.00 100.00	,	
X21149.0 GR7101.0 V1 1400 X1 1400 X1 1400 X21149.0 GR7100.8 GR7100.8 X1 1300 X21149.0 GR7100.7 GR7100.7 GR7100.7 GR7100.7 GR7100.8 GR7100.8 GR7100.8 GR7100.7 GR7100.7 GR7100.7 GR7100.7 GR7100.7 GR7100.7 GR7100.7 GR7100.5 GR7100.5 GR7100.5 CR7100.5 CR7100.6 CR7100.			7096.17	50.00	7096.17	110.00 7101.17	160.00	
GR7101.0			0.01	160.00	100.00	100.00 100.00)	
X1 1400			7005 00	FO 00	7006 00	110 00 7101 00		
X21149.0 GR7100.8 X1 1300 X21149.0 GR7100.7 GR7100.7 GR7100.7 GR7100.7 X11200.0 X21149.0 GR7100.5 X21149.0 GR7100.5 X21149.0 GR7100.5 GR7100.5 X21149.0 GR7100.5 GR7100.5 X21149.0 GR7100.5 GR7100.5 X21149.0 GR7100.5 X21149.0 GR7100.5 GR7100.5 X21149.0 GR7100.6 X21149.0 GR7100.4 X2149.0 GR7100.4 X21149.0 GR7100.5 X2114								
GR7100.8		4.0	0.01	100.00	100.00	100.00 100.00	,	
X1 1300		0.01	7095.83	50.00	7095.83	110.00 7100.83	160.00	
GR7100.7			. 0.01	160.00	100.00	100.00 100.00)	
X11200.0								
X21149.0 GR7100.5 0.01 7095.50 50.00 7095.50 110.00 7100.50 160.00 X1 1195 4.0 0.01 158.75 5.00 5.00 5.00 X21149.0 GR7100.5 0.01 7095.48 49.53 7095.48 109.22 7100.48 158.75 X1 1190 4.0 0.01 157.50 5.00 5.00 X21149.0 GR7100.5 X1 1185 4.0 0.01 156.25 5.00 5.00 5.00 X21149.0 GR7100.5 X1 1180 4.0 0.01 155.00 GR7100.5 X1 1180 4.0 0.01 155.00 GR7100.5 X1 1180 4.0 0.01 7095.45 48.59 7095.45 107.66 7100.45 156.25 X1 1180 4.0 0.01 7095.44 48.59 7095.45 107.66 7100.45 156.25 X1 1180 X21149.0 GR7100.4 0.01 7095.44 48.13 7095.44 106.88 7100.44 155.00 X1 1175 4.0 0.01 153.75 5.00 5.00 5.00								
GR7100.5			0.01	100.00	3.00	3.00 5.00		
X21149.0 GR7100.5			7095.50	50.00	7095.50	110.00 7100.50	160.00	
GR7100.5			0.01	158.75	5.00	5.00 5.00)	
X1 1190			2005 .0		2005 40			
X21149.0 GR7100.5 0.01 7095.47 49.06 7095.47 108.44 7100.47 157.50 X1 1185 4.0 0.01 156.25 5.00 5.00 5.00 GR7100.5 0.01 7095.45 48.59 7095.45 107.66 7100.45 156.25 X1 1180 4.0 0.01 155.00 5.00 5.00 X21149.0 GR7100.4 0.01 7095.44 48.13 7095.44 106.88 7100.44 155.00 X1 1175 4.0 0.01 153.75 5.00 5.00 5.00								
GR7100.5			0.01	137.30	5.00	3.00		
X1 1185			7095.47	49.06	7095.47			
GR7100.5 0.01 7095.45 48.59 7095.45 107.66 7100.45 156.25 X1 1180 4.0 0.01 155.00 5.00 5.00 5.00 X21149.0 GR7100.4 0.01 7095.44 48.13 7095.44 106.88 7100.44 155.00 X1 1175 4.0 0.01 153.75 5.00 5.00 5.00			0.01	156.25	5.00	5.00 5.00		
X1 1180			7005 15	40.50	7005 15	107 66 7100 17	155.05	
X21149.0 GR7100.4 0.01 7095.44 48.13 7095.44 106.88 7100.44 155.00 X1 1175 4.0 0.01 153.75 5.00 5.00 5.00								
GR7100.4 0.01 7095.44 48.13 7095.44 106.88 7100.44 155.00 X1 1175 4.0 0.01 153.75 5.00 5.00 5.00		4.0	0.01	155.00	3.00	3.00		
X1 1175 4.0 0.01 153.75 5.00 5.00 5.00							155.00	
X21149.0		4.0	0.01	153.75	5.00	5.00 5.00		
	AZ1149.0							

TABLE F-12. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL N (continued).

GR7100.4 X1 1170		7095.42 0.01		7095.42 5.00			153.75
X21149.0 GR7100.4 X1 1165	0.01	7095.41 0.01		7095.41 5.00		7100.41	152.50
X21149.0 GR7100.4 X1 1160	0.01	7095.39 0.01	46.72	7095.39 5.00	104.53	7100.39	151.25
X21149.0 GR7100.4	0.01	7095.38	46.25	7095.38	103.75	7100.38	150.00
X1 1155 X21149.0 GR7100.4		0.01		5.00 7095.36		5.00 7100.36	148.75
X1 1150 X21149.0	4.0	0.01	147.50	5.00	5.00		147.50
GR7100.3 X1 1145 X21149.0	4.0	7095.34	146.25		5.00	5.00	
GR7100.3 X1 1140 X21149.0		7095.33 0.01		7095.33 5.00		7100.33 5.00	146.25
GR7100.3 X1 1135		7095.31 0.01		7095.31 5.00		7100.31 5.00	145.00
X21149.0 GR7100.3 X1 1130		7095.30 0.01		7095.30 5.00		7100.30 5.00	143.75
X21149.0 GR7100.3 X1 1125		7095.28 0.01		7095.28 5.00		7100.28	142.50
X21149.0 GR7100.3 X1 1120		7095.27 0.01		7095.27 5.00		7100.27	141.25
X21149.0 GR7100.3 X1 1115	0.01	7095.25	42.50	7095.25		7100.25	140.00
X21149.0 GR7100.2	0.01	7095.23	42.03	7095.23	96.72	7100.23	
X1 1110 X21149.0 GR7100.2		7095.22		5.00 7095.22	95.94	5.00 7100.22	137.50
X1 1105 X21149.0 GR7100.2		7095.20		5.00		5.00 7100.20	136.25
X1 1100 X21149.0	4.0	0.01	135.00	5.00	5.00	5.00	
GR7100.2 X1 1095 X21149.0	4.0		133.75		5.00		135.00
GR7100.2 X1 1090 X21149.0		7095.17 0.01		7095.17 5.00	93.59 5.00	7100.17	133.75
GR7100.2 X1 1085 X21149.0		7095.16 0.01		7095.16 5.00		7100.16 5.00	132.50
GR7100.1 X1 1080		7095.14 0.01		7095.14 5.00		7100.14 5.00	131.25
X21149.0 GR7100.1 X1 1075		7095.13 0.01		7095.13 5.00	91.25 5.00	7100.13	130.00
X21149.0 GR7100.1 X1 1070	0.01	7095.11	38.28 127.50	7095.11 5.00	90.47 5.00	7100.11	128.75
X21149.0 GR7100.1 X1 1065	0.01	7095.09	37.81 126.25	7095.09 5.00	89.69 5.00	7100.09	127.50
X21149.0 GR7100.1	0.01	7095.08	37.34	7095.08	88.91	7100.08	126.25
X1 1060 X21149.0 GR7100.1		0.01 7095.06	125.00 36.88	5.00 7095.06	5.00 88.13	5.00 7100.06	125.00
X1 1055 X21149.0 GR7100.0	4.0 0.01	0.01 7095.05	123.75 36.41	5.00 7095.05	5.00 87.34	5.00	123.75
X1 1050 X21149.0 GR7100.0	4.0	0.01 7.095.03	122.50	5.00	5.00	5.00	
X1 1045 X21149.0	4.0	0.01	121.25	5.00	5.00	5.00	122.50
GR7100.0 X1 1040	0.01 4.0	7095.02 0.01	35.47 120.00	7095.02 5.00	85.78 5.00	7100.02 5.00	121.25

TABLE F-12. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL N (continued).

V01140 0					
X21149.0 GR7100.0	0.01 7095.00	35.00	7095.00	85.00 7100.00	120.00
X1 1035		118.75			
X21149.0					
GR7100.0 X1 1030	0.01 7094.98 4.0 0.01	34.53 117.50		84.22 7099.98 5.00 5.00	118.75
X21149.0	4.0 0.01	117.50	3.00	5.00 5.00	
GR7100.0	0.01 7094.97	34.06	7094.97	83.44 7099.97	117.50
X1 1025	4.0 0.01	116.25	5.00	5.00 5.00	
X21149.0					
GR7100.0 X1 1020	0.01 7094.95 4.0 0.01		7094.95		116.25
X21149.0	4.0 0.01	113.00	5.00	3.00 3.00	
GR7099.9	0.01 7094.94	33.13	7094.94	81.88 7099.94	115.00
X1 1015	4.0 0.01	113.75	5.00	5.00 5.00	
X21149.0					
GR7099.9 X1 1010	0.01 7094.92 4.0 0.01	112.50		81.09 7099.92 5.00 5.00	113.75
X21149.0	4.0 0.01	112.50	5.00	3.00 5.00	
GR7099.9	0.01 7094.91	32.19	7094.91	80.31 7099.91	112.50
X1 1005	4.0 0.01	111.25	5.00	80.31 7099.91 5.00 5.00	
X21149.0					
GR7099.9	0.01 7094.89		7094.89		111.25
X1 1000 X21149.0	4.0 0.01	110.00	5.00	5.00 5.00	
GR7099.9	0.01 7094.88	31.25	7094.88	78.75 7099.88	110.00
X1 995	4.0 0.01	108.75			
X21149.0					
GR7099.9 X1 990	0.01 7094.86	30.78	7094.86 5.00	77.97 7099.86 5.00 5.00	108.75
X21149.0	4.0 0.01	107.50	5.00	5.00 5.00	
GR7099.8	0.01 7094.84	30.31	7094.84	77.19 7099.84	107.50
X1 985	4.0 0.01	106.25	7094.84 5.00	5.00 5.00	
X21149.0					
GR7099.8 X1 980	0.01 7094.83 4.0 0.01	29.84	7094.83 5.00	76.41 7099.83 5.00 5.00	106.25
X21149.0	4.0 0.01	103.00	3.00	3.00 3.00	
	0.01 7094.81	29.38	7094.81	75.63 7099.81	105.00
X1 975	4.0 0.01	103.75	5.00	5.00 5.00	
X21149.0					
GR7099.8 X1 970	0.01 7094.80 4.0 0.01		7094.80	74.84 7099.80	103.75
X21149.0	4.0 0.01	102.50	5.00	5.00 5.00	
GR7099.8	0.01 7094.78	28.44	7094.78	74.06 7099.78	102.50
X1 965				5.00 5.00	
X21149.0					
GR7099.8	0.01 7094.77		7094.77	73.28 7099.77	101.25
X1 960 X21149.0	4.0 0.01	100.00	5.00	5.00 5.00	
	0.01 7094.75	27.50	7094.75	72.50 7099.75	100.00
X1 955	4.0 0.01		5.00		
X21149.0					
GR7099.7	0.01 7094.73	27.03	7094.73 5.00	71.72 7099.73	98.75
X1 950 X21149.0	4.0 0.01	97.50	5.00	5.00 5.00	
GR7099.7	0.01 7094.72	26.56	7094.72	70.94 7099.72	97.50
X1 945	4.0 0.01	96.25			
X21149.0					
GR7099.7	0.01 7094.70		7094.70		96.25
X1 940 X21149.0	4.0 0.01	95.00	5.00	5.00 5.00	
GR7099.7	0.01 7094.69	25.63	7094.69	69.38 7099.69	95.00
X1 935	4.0 0.01	93.75	5.00	5.00 5.00	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
X21149.0					
GR7099.7	0.01 7094.67		7094.67	68.59 7099.67	93.75
X1 930 X21149.0	4.0 0.01	92.50	5.00	5.00 5.00	
GR7099.7	0.01 7094.66	24 69	7094.66	67.81 7099.66	92.50
X1 925	4.0 0.01	91.25			22.30
X21149.0					
GR7099.6	0.01 7094.64		7094.64	67.03 7099.64	91.25
X1 920	4.0 0.01	90.00	5.00	5.00 5.00	
X21149.0 GR7099.6	0.01 7094.63	23 75	7094.63	66.25 7099.63	90.00
X1 915	4.0 0.01	88.75	5.00	5.00 5.00	50.00
X21149.0					
GR7099.6	0.01 7094.61		7094.61	65.47 7099.61	88.75
X1 910	4.0 0.01	87.50	5.00	5.00 5.00	
X21149.0 GR7099.6	0.01 7094.59	22.91	7094.59	64.69 7099.59	87.50
GR. 073.0	0.01 .094.09	22.01	. 554.55	04107 /033.03	07.50

TABLE F-12. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL N (continued).

X1 905 X21149.0	4.0	0.01	86.25	5.00	5.00	5.00	
GR7099.6		7094.58	22.34 85.00	7094.58 5.00	63.91 5.00	7099.58 5.00	86.25
X1 900 X21149.0	4.0						25.00
GR7099.6 X1 895	0.01 4.0	7094.56	21.88 83.75	7094.56 5.00	63.13 5.00	7099.56 5.00	85.00
X21149.0 GR7099.5	0.01	7094.55	21 41	7094.55	62 34	7099.55	83.75
X1 890	4.0						•
X21149.0 GR7099.5	0.01	7094.53	20.94	7094.53	61.56	7099.53	82.50
X1 885 X21149.0	4.0	0.01	81.25	5.00	- 5.00	5.00	
GR7099.5		7094.52	20.47	7094.52	60.78	7099.52	81.25
X12.0 X21149.0	4.0						
GR7099.5 X1 780	0.01 4.0	7094.50	20.00 80.00	7094.50	100.00	7099.50	80.00
X2 899.4 GR7099.3	0.01	7094.32	20.00	7094.32	60.00	7099.32	80.00
X1 680	4.0		80.00		80.00		00.00
X2 649.7 GR7099.1	0.01	7094.14	20.00	7094.14	60.00	7099.14	80.00
X1600.0 X2 450.0	4.0	0.01	80.00	75.00	75.00	75.00	
GR7099.0		7094.00		7094.00		7099.00	80.00
X1525.0 X2 180.0		0.01	80.00		5.00		
GR7098.2 X1 520		7093.16	20.00 64.38	7093.16 5.00	60.00 5.00	7098.16 5.00	80.00
X2 180.0 GR7098.1		7093.15		7093.15	16 16	7098.15	64.38
X1 515	4.0		48.75		2.00		04.50
X2 180.0 GR7098.1	0.01	7093.13	15.83	7093.13		7098.13	48.75
X1513.0 X2 180.0	4.0	0.01	42.50	5.00	5.00	5.00	
GR7098.1		7093.13		7093.13 5.00	27.50 5.00	7098.13 5.00	42.50
X1 508 X2 180.0	4.0	0.01	33.85				
GR7098.1 X1 503	4.0	7093.12	13.08 25.19	7093.12	3.00	7098.12	33.85
X2 180.0 GR7098.1	0.01	7093.11	11 15	7093.11	14.04	7098.11	25.19
X1500.0	4.0	0.01	20.00		25.00	25.00	20.17
X2 180.0 GR7098.1	0.01	7093.10	10.00	7093.10	10.00	7098.10	20.00
X13.0 X2 180.0	4.0	0.01	20.00	25.00	25.00	25.00	
GR7098.0		7093.05		7093.05		7098.05	20.00
X1450.0 X2 180.0	4.0		20.00		5.00		
GR7098.0 X1445.0	0.01	7093.00	10.00 25.00	7093.00 5.00	10.00 5.00	7098.00 5.00	20.00
X2 180.0 GR7097.8		7092.80		7092.80	12.50	7097.80	25.00
X1440.0	4.0	0.01	30.00		10.00		23.00
X2 180.0 GR7097.5	0.01	7092.50	15.00	7092.50	15.00	7097.50	30.00
X1430.0 X2 180.0	4.0	0.01	35.00	10.00	10.00	10.00	
GR7097.0		7092.00		7092.00		7097.00	35.00
X1420.0 X2 180.0	4.0	0.01		2.00		2.00	
GR7096.5 X1 418	0.01 4.0	7091.50		7091.50		7096.50	40.00
X2 180.0 GR7096.4	0 01	7091.40		7091.40	26 50	7096.40	42.00
X1 416	4.0		44.00	2.00	2.00		72.00
X2 180.0 GR7096.3	0.01	7091.30	16.00	7091.30	28.00	7096.30	44.00
X1 414 X2 180.0	4.0	0.01		2.00	2.00	2.00	
GR7096.2 X1 412		7091.20		7091.20		7096.20	46.00
X2 180.0	4.0	0.01					
GR7096.1 X1410.0	0.01 4.0	7091.10	17.00 50.00	7091.10		7096.10	48.00
X2 180.0							

TABLE F-12. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL N (continued).

GR7096.0 X1 408	0.01	7091.00	17.50 52.00	7091.00	32.50 2.00	7096.00 2.00	
X2 180.0							
GR7095.8	0.01	7090.B4	18.00	7090.84	34.00	7095.84	52.00
X1 406	4.0	0.01	54.00	2.00	2.00	2.00	
X2 180.0							
GR7095.7	0.01	7090.68	18.50	7090.68	35.50	7095.68	54.00
X1 404	4.0	0.01	56.00	2.00	2.00		
X2 180.0		0.01	50.00	2.00	• • • • • • • • • • • • • • • • • • • •	2.00	
	0.01	7090 52	10 00	7090 52	37 00	7095 52	56.00
X1 402	0.01	7090.52 0.01	E0.00	3.00	37.00	3 00	30.00
X2 180.0	4.0	0.01	36.00	2.00	2.00	2.00	
AZ 180.0	0 01	7000 36	3.0 E0	3000 36	30 50	7005 26	E 0 00
GR7095.4	0.01	7090.36	19.50	1090.36	36.30	1093.30	58.00
X1400.0	4.0	0.01	60.00	5.00	5.00	5.00	
X2 180.0							
GR7095.2	0.01	7090.20	20.00	7090.20	40.00	7095.20	60.00
X1395.0	4.0	0.01	65.00	10.00	10.00	10.00	
X2 180.0							
GR7095.0	0.01	7090.00	20.00	7090.00	45.00	7095.00	65.00
X14.0	4.0	7090.00 0.01	65.00	20.00	20.00	20.00	•
X2 180.0							
GR7093.9	0.01	7088.90	20.00	7088.90	45.00	7093.90	65.00
X1 365	4.0	7088.90 0.01	65.00	15.00	15.00	15.00	
X2 180.0							
GR7092.2	0.01	7087.24	20.00	7087.24	45.00	7092.24	65.00
X1350.0	4.0	7087.24	65.00	20.00	20.00	20.00	
X2 180.0	• • •			2	•	,	
	0.01	7086.00	20.00	7086.00	45.00	7091.00	65.00
X1 330	4 0	7086.00 0.01	65.00	20.00	20.00	20.00	
X2 180.0	4.0	0.01	05.00	20.00	20.00	20.00	
	0 01	7005 10	20.00	7005 10	45 00	7000 10	65.00
X1 310	0.01	7085.10 0.01	20.00	7083.10	20.00	7090.10	65.00
X2 100 0	4.0	0.01	65.00	20.00	20.00	20.00	
X2 180.0	0 01	7004 00	00.00	7004 00	45.00	7000 00	
GR7089.2	0.01	7084.20 0.01	20.00	7084.20	45.00	7089.20	65.00
	4.0	0.01	65.00	20.00	20.00	20.00	
X2 180.0							
GR7088.3	0.01	7083.30 0.01	20.00	7083.30	45.00	7088.30	65.00
	4.0	0.01	65.00	20.00	20.00	20.00	
X2 180.0							
GR7087.4	0.01	7082.40 0.01	.20.00	7082.40	45.00	7087.40	65.00
X1250.0	4.0	0.01	65.00	10.00	10.00	10.00	
X2 180.0							
GR7086.5	0.01	7081.50	20.00	7081.50	45.00	7086.50	65.00
X1 240	4.0	0.01	66.67	10.00	10.00	10.00	
X2 180.0	•••	0.02		10.00	20.00	10.00	
	0.01	7081.00	20.00	7081 00	46 67	7086.00	66.67
X1 230	4.0			10.00			00.07
X2 180.0	4.0	0.01	00.55	10.00	10.00	10.00	
GR7085.5	0.01	7080.50	20.00	7080.50	40 33	7085.50	60 33
	0.01	7000.30	70.00	7000.30	30.00	7083.30	68.33
X1220.0	4.0	0.01	70.00	20.00	20.00	20.00	
X2 180.0							
GR7085.0		7080.00					70.00
X1 200	4.0	0.01	70.00	10.00	10.00	10.00	
X2 212.7							
		7079.33				7084.33	70.00
X1190.0	4.0	0.01	70.00	30.00	30.00	30.00	
X2 229.0							
GR7084.0	0.01	7079.00	20.00	7079.00	50.00	7084.00	70.00
X1140.0	4.0	0.01	70.00	30.00	30.00	30.00	
X2 229.0							
GR7083.3	0.01	7078.25	20.00	7078.25	50.00	7083.25	70.00
X15.0	4.0	0.01	70.00	20.00	20.00	20.00	
X2 229.0							
GR7082.5	0.01	7077.50	20.00	7077.50	50.00	7082.50	70.00
X1 110	4.0	0.01	70.00	10.00	10.00	10.00	
X2 229.0							
GR7081.6	0.01	7076.63	20.00	7076.63	50.00	7081.63	70.00
X1100.0	4.0	0.01	70.00	20.00		20.00	,0.00
X2 229.0	4.0	0.01	70.00	20.00	20.00	20.00	
	0.01	7076.20	20 00	7076 20	50 00	7001 20	70.00
GR7081.2				7076.20	50.00	7081.20	70.00
X180.0	4.0	0.01	70.00				
X2 229.0		7075 05	00.00	2025 65	E 0 00	3000 00	
	0.01	7075.00	20.00	7075.00	50.00	7080.00	70.00
EJ							
ER							

TABLE F-13. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL O.

T1 T2	PATHFINE CHANNEL	DER SHIRLE O	Y BASIN	TAILING	S RECLAM	<u>I</u> ATION		
ТЗ J1	PMF - SI	JBCRITICAL	,	-1.0)	•	583 D	7100.0
J2 -1	C	-1.0					503.5	. 200.0
NC 0.035	0.035 4.0	0.035		0.3	ı		•	
X2 583.0)							
GR7100.6 X19	0.01	7096.80	16.00 52.00	7096.80	36.00	7100.80 15.00	52.00	
X2 583.0	i .							
GR7101.0 X1100	0.01	7097.00 0.01	16.00	7097.00	36.00	7101.00	52.00	
X2 583.0	ı							
GR7102.0 X1 150	0.01	7098.00				7102.00		
X2 583.0								
GR7102.7 X1250		7098.67	16.00	7098.67	36.00	100.00	52.00	
X2 583.0								
GR7104.0 X1 310	0.01	7100.00	16.00	7100.00	36.00	7104.00	52.00	
X2 583.0								
GR7104.4 X1410	0.01	7100.38	16.00	7100.38	36.00	7104.38	52.00	
X2 583.0								
GR7105.0 X1 411	0.01	7101.00	16.00	7101.00	36.00	7105.00	52.00	
X2 574.8								
GR7104.0 X1 414	0.01	7101.00	16.00	7101.00	36.50	7104.00	52.46	
X2 550.2								
GR7104.0 X1 417		7101.02 0.01	16.00	7101.02	38.00	7104.02 3.00	53.84	
X2 525.6								
GR7104.0 X1420	0.01	7101.03	16.00	7101.03		7104.03	55.22	
X2 501.0						3.00		
GR7104.0 X1460	0.01	7101.05 0.01			37.00 40.00	7104.05	48.70	
X2 501.0								
GR7104.3 X1 465	0.01	7101.25 0.01	12.00	7101.25	37.00	7104.25	48.70	
X2 501.0								
GR7104.3 X1 470	0.01	7101.27 0.01					48.73	
X2 501.0				5.00	5.00	5.00		
GR7104.3 X1 475	0.01	7101.30				7104.30	48.76	
X2 501.0				5.00		5.00		
GR7104.3 X1 480	0.01	7101.33	12.00	7101.33	37.00	7104.33	48.79	
X2 501.0	4.0	0.01	40.02	5.00	5.00	5.00		
GR7104.4 X1 485		7101.35		7101.35	37.00	7104.35	48.82	
X2 501.0		0.01	40.03	3.00	5.00	5.00		
GR7104.4 X1 490		7101.38				7104.38 5.00	48.85	
X2 501.0					5.00	5.00		
GR7104.4 X1 495	0.01 4.0	7101.40	12.00 48.91	7101.40 5.00	37.00 5.00	7104.40	48.88	
X2 501.0								
GR7104.4 X1 500	0.01 4.0	7101.42	12.00 48.94	7101.42		7104.42 5.00	48.91	
X2 501.0						3.00		
GR7104.5 X1 505	0.01 4.0	7101.45	12.00 48.97	7101.45		7104.45 5.00	48.94	
X2 501.0								
GR7104.5 X16	0.01 4.0	7101.48		7101.48		7104.48	48.97	
X2 501.0								
GR7104.5 X1610	0.01 4.0			7101.50	37.00 100.00	7104.50	49.00	
X2 501.0								
GR7105.0 X1 620	0.01 4.0		12.00 49.00	7102.00	37.00 10.00		49.00	,
X2 501.0								
GR7105.0 X1 720	0.01 4.0	7102.05	12.00	7102.05	37.00 100.00	7105.05	49.00	
X2 501.0				100.00	100.00	100.00		

TABLE F-13. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL O (continued).

GR7105.5	0.01	7102.52	12.00	7102.52	37.00	7105.52	49.00	
X1820	4.0	0.01	49.00	100.00	100.00	100.00		
X2 501.0								
GR7106.0	0.01	7103.00	12.00	7103.00	37.00	7106.00	49.00	
X1 830	4.0	0.01	49.00	10.00	10.00	10.00		
X2 501.0								
GR7106.0	0.01	7103.05	12.00	7103.05	37.00	7106.05	49.00	
X1 930	4.0	0.01	49.00	100.00	100.00	100.00		
X2 501.0								
GR7106.5	0.01	7103.54	12.00	7103.54	37.00	7106.54	49.00	
X1 1030	4.0	0.01	49.00	100.00	100.00	100.00		
X2 501.0								
GR7107.0		7104.02					49.00	
X1 1130	4.0	0.01	49.00	100.00	100.00	100.00		
X2 501.0								
GR7107.5		7104.51				7107.51	49.00	
X11230	4.0	0.01	49.00	100.00	100.00	100.00		
X2 501.0								
	0.01	7105.00	12.00	7105.00	37.00	7108.00	49.00	
EJ								
ER								

TABLE F-14. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL O.

T1	PATHFIND	ER SHIRLE	EY BASIN	TAILING	s reclam	ATION		
T2	CHANNEL	o - INCLU	JDES CRO				5-9	
T3 J1	PMF - SU	PERCRITIO	1.0	-1.0			501.0	7108.4
J2 -1		-1.0						
NC 0.035 X11230	0.035	0.035	49.00	100.00	100.00	100.00		
X2 501.0		0.01	49.00	100.00	100.00	100.00		
GR7108.0		7105.00						
X1 1130 X2 501.0		0.01	49.00	100.00	100.00	100.00		
GR7107.5		7104.51	12.00	7104.51	37.00	7107.51	49.00	
X1 1030		0.01	49.00	100.00	100.00	100.00		
X2 501.0		7104.02	12.00	7104.02	37.00	7107.02	49.00	
X1 930	4.0	0.01	49.00	100.00	100.00	100.00		
X2 501.0 GR7106.5		7103.54	12.00	7102 64	37 00	7106 54	49.00	
X1 830		0.01				10.00		
X2 501.0)							
GR7106.0 X1820		7103.05 0.01				7106.05	49.00	
X2 501.0)							
GR7106.0	0.01	7103.00	12.00	7103.00	37.00	7106.00	49.00	
X1 720 X2 501.0		0.01	49.00	100.00	100.00	100.00		
GR7105.5	0.01	7102.52				7105.52	49.00	
X1 620 X2 501.0		0.01	49.00	10.00	10.00	10.00		
GR7105.0		7102.05	12.00	7102.05	37.00	7105.05	49.00	
X1610	4.0	0.01						
X2 501.0		7102.00	12.00	7102 00	37 00	7105 00	49.00	
X16		0.01		5.00			49.00	
X2 501.0								
GR7104.5 X1 505		7101.50	12.00 48.97			7104.50 5.00	49.00	
X2 501.0		****			••••	•,••		
GR7104.5		7101.48				7104.48	48.97	
X1 500 X2 501.0	4.0	0.01	40.94	5.00	5.00	5.00		
GR7104.5	0.01	7101.45				7104.45	48.94	
X1 495 X2 501.0		0.01	48.91	5.00	5.00	5.00		
		7101.42	12.00	7101.42	37.00	7104.42	48.91	
X1 490 X2 501.0		0.01	48.88	5.00	5.00	5.00		
GR7104.4		7101.40	12.00	7101.40	37.00	7104.40	48.98	
X1 485	4.0		48.85					
X2 501.0 GR7104.4		7101.38	12.00	7101 39	37 00	7104.38	48.85	
X1 480		0.01					40.00	
X2 501.0		7101 25				2.0. 25	40.00	
GR7104.4 X1 475		7101.35		5.00		7104.35 5.00	48.82	
X2 501.0							1	
GR7104.3 X1 470	0.01 4.0	7101.33		7101.33	37.00 5.00	7104.33	48.79	
X2 501.0								
GR7104.3 X1 465		7101.30		7101.30		7104.30	48.76	
X2 501.0				5.00	5.00	5.00		
GR7104.3	0.01	7101.27				7104.27	48.73	
X1460 X2 501.0		0.01	48.70	40.00	40.00	40.00		
GR7104.3	0.01	7101.25	12.00	7101.25	37.00	7104.25	48.70	
X1420 X2 501.0		0.01	48.70	3.00	3.00	3.00		
GR7104.0	0.01	7101.05	12.00	7101.05	37.00	7104.05	48.70	
X1 417	4.0	0.01	55.22	3.00	3.00	3.00		
X2 525.6 GR7104.0		7101.03	16.00	7101.03	39 50	7104 03	55.22	
X1 414	4.0			3.00				
X2 550.2	0.01	7101 02	16.00	7101 00	20 00	7104 00	E2 04	
X1 411	4.0	7101.02	52.46	1.00	1.00	1.00	53.84	
X2 574.8							_	
GR7104.0	0.01	7101.00	16.00 52.00	7101.00	36.50	7104.00	52.46	
X2 583.0			-2.00		200.00	200.00		

TABLE F-14. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL O (continued).

GR7105.0	0.01	7101.00	16.00	7101.00	36.00	7105.00	52.00
X1 310	4.0	0.01	52.00	60.00	60.00	60.00	
X2 583.0			•				
GR7104.4	0.01	7100.38	16.00	7100.38	36.00	7104.38	52.00
X1250	4.0	0.01	52.00	100.00	100.00	100.00	
X2 583.0							
GR7104.0	0.01	7100.00	16.00	7100.00	36.00	7104.00	52.00
X1 150	4.0	0.01	52.00	50.00	50.00	50.00	
X2 583.0							
GR7102.7	0.01	7098.67	16.00	7098.67	36.00	7102.67	52.00
X1100	4.0	0.01	52.00	45.00	45.00	45.00	
X2 583.0							
GR7102.0	0.01	7098.00	16.00	7098.00	36.00	7102.00	52.00
X19	4.0	0.01	52.00	15.00	15.00	15.00	
X2 583.0							
GR7101.0	0.01	7097.00	16.00	7097.00	36.00	7101.00	52,00
X140	4.0	0.01	52.00	5.			
X2 583.0							
GR7100.8	0.01	7096.80	16.00	7096.80	36.00	7100.80	52.00
EJ							
ER							

TABLE F-15. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL P.

T1 T2		ER SHIRLE	Y BASIN	TAILINGS	RECLAM	AT10N		
T3	CHANNEL	r BCRITICAL						
J1	PMF - 30	PCKITICAL		-1.0			583.0	7104 0
J2 -1	0	-1.0		-1.0			363.0	7104.0
NC 0.035				0.3				
X1410	4.0	0.01	44.00					
X2 583.0						2101 00		
GR7104.0		7101.00		7101.00		7104.00	44.00	
X1 10	4.0	0.01	44.00	10.00	10.00	10.00		
X2 446.3								
GR7104.3		7101.33		7101.33		7104.33	44.00	
X130	4.0	0.01	44.00	20.00	20.00	20.00		
X2 173.0	H							
GR7105.0	0.01	7102.00	12.00	7102.00	32.00	7105.00	44.00	
X18	4.0	0.01	44.00	20.00	20.00	20.00		
X2 173.0	l .							
GR7105.5	0.01	7102.50	12.00	7102.50	32.00	7105.50	44.00	
X170	4.0	0.01	44.00	20.00	20.00	20.00		
X2 173.0	į.							
GR7106.0	0.01	7103.00	12.00	7103.00	32.00	7106.00	44.00	
X1110	4.0	0.01	44.00	40.00	40.00	40.00		
X2 173.0	r							
GR7107.0	0.01	7104.00	12.00	7104.00	32.00	7107.00	44.00	
X1160	4.0		44.00		50.00			
X2 173.0								
GR7108.0		7105.00	12.00	7105.00	32.00	7108.00	44.00	
X1200		0.01		40.00	40.00			
X2 173.0		****						
GR7106.5		7105.50	12.00	7105.50	32.00	7108 50	44.00	
X1240		0.01		40.00	40.00		11.00	
X2 173.0		0.01		40.00	40.00	40.00		
GR7109.0		7106.00	12.00	7106.00	32 00	7100 00	44.00	
X1300		0.01		60.00			44.00	
X2 173.0		0.01	44.00	60.00	80.00	60.00		
GR7109.5		7106.50	10.00	7106.50	20.00	7100 50	44.00	
X17						7109.50	44.00	
		0.01	44.00	70.00	70.00	70.00		
X2 173.0		71.07.00	10.00	7107 00	20.00	3110.00		
GR7110.0		7107.00				7110.00	44.00	
X1450	4.0	0.01	44.00	80.00	80.00	80.00		
X2 173.0								
GR7110.5		7107.50		7107.50		7110.50	44.00	
X1540	4.0	0.01	44.00	90.00	90.00	90.00		
X2 173.0								
GR7111.0		7108.00		7108.00		7111.00	44.00	
X1600	4.0	0.01	44.00	60.00	60.00	60.00		
X2 173.0								
GR7111.5		7108.50		7108.50		7111.50	44.00	
X1670	4.0	0.01	44.00	70.00	70.00	70.00		
X2 173.0		_						
GR7112.0		7109.00				7112.00	44.00	
X1760	4.0	0.01	44.00	90.00	90.00	90.00		
X2 173.0								
		7109.50			32.00	7112.50	44.00	
X1840	4.0	0.01	44.00	80.00	80.00	80.00		
X2 173.0		*						
GR7113.0	0.01	7110.00	12.00	7110.00	32.00	7113.00	44.00	
EJ								
FR								

TABLE F-16. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL P.

T1 T2	PATHFIND CHANNEL		Y BASIN	TAILINGS	RECLAM	ATION		
T3	PMF - SU	PERCRITIO	AL					•
J1			1.0	-1.0			173.0	7112.4
J2 -1	0	-1.0						
NC 0.035	0.035	0.035	0.1	0.3				
X1840	4.0	0.01	44.00	80.00	80.00	80.00		
X2 173.0								
GR7113.0	0.01	7110.00	12.00	7110.00	32.00	7113.00	44.00	*
X1760	4.0	0.01	44.00	90.00	90.00	90.00		
X2 173.0		•••						
GR7112.5		7109.50	12.00	7109.50	32 00	7112 50	44.00	
X1670	4.0	0.01		70.00	70.00	70.00	44.00	
X2 173.0		0.01	44.00	70.00	,0.00	,0.00		
GR7112.0		7109.00	12.00	7109.00	33 00	7112 00	44.00	
							44.00	
X1600	4.0	0.01	44.00	60.00	60.00	60.00		
X2 173.0								
GR7111.5				7108.50			44.00	
X1540	4.0	0.01	44.00	90.00	90.00	90.00		
X2 173.0								
GR7111.0	0.01	7106.00	12.00	7108.00	32.00	7111.00	44.00	
X1450	4.0	0.01	44.00	80.00	80.00	80.00		
X2 173.0								
GR7110.5	0.01	7107.50	12.00	7107.50	32.00	7110.50	44.00	
X17	4.0	0.01	44.00	70.00	70.00	70.00		
X2 173.0								
GR7110.0		7107.00	12.00	7107.00	32.00	7110.00	44.00	
X1300				60.00	60.00			
X2 173.0		0.02						
GR7109.5		7106.50	12 00	7106.50	32 00	7109.50	44.00	
X1240	4.0		44.00	40.00	40.00	40.00	44.00	
X2 173.0		0.01	44.00	40.00	40.00	40.00		
		7106.00	12 00	7106 00	22.00	7100 00	44 00	
GR7109.0				7106.00		7109.00	44.00	
X1200	4.0	0.01	44.00	40.00	40.00	40.00		
X2 173.0		7105 50	10.00	7105 50	20.00	7100 50	44.00	
GR7108.5		7105.50		7105.50		7108.50	44.00	
X1160	4.0	0.01	44.00	50.00	50.00	30.00		
X2 173.0								
GR7108.0				7105.00			44.00	•
X1110	4.0	0.01	44.00	40.00	40.00	40.00		
X2 173.0						_		
GR7107.0		7104.00				7107.00	44.00	
X170	4.0	0.01	44.00	20.00	20.00	20.00		
X2 173.0								
GR7106.0		7103.00		7103.00		7106.00	44.00	
X18	4.0	0.01	44.00	20.00 /	20.00	20.00		
X2 173.0								
GR7105.5	0.01	7102.50	12.00	7102.50	32.00	7105.50	44.00	
X130	4.0	0.01	44.00	20.00	20.00	20.00		
X2 173.0								
GR7105.0	0.01	7102.00	12.00	7102.00	32.00	7105.00	44.00	
X1 10	4.0	0.01	44.00	10.00	10.00	10.00		
X2 446.3								
GR7104.3	0.01	7101.33	12.00	7101.33	32.00	7104.33	44.00	
X1410	4.0	0.01	44.00					
X2 583.0								
GR7104.0	0.01	7101.00	12 00	7101.00	32.00	7104.00	44.00	
EJ	0.01	. 101.00		0	22.00		11.00	
ER								
-11								

TABLE F-17. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL Q.

						_		
	CHANNEL						H HC5-10	
	PMF - SU			55 556110				
Jl				-1.0			843.0	7063.0
J2 -1 NC 0.035			0.1	0.3				
X190		0.033		0.5				
X2 843.0								
GR7080.8				7060.82		7080.82	190.00	
X1100		0.01	190.00	10.00	10.00	10.00		
X2 843.0 GR7080.9		7060.94	80.00	7060.94	110.00	7080.94	190.00	
X1130				30.00				
X2 843.0						2001 20		
GR7081.3 X1220	0.01		80.00			7081.30 90.00	190.00	
X2 843.0		0.01	130.00	30.00	30.00	30.00		
GR7082.4		7062.38	80.00	7062.38	110.00	7082.38	190.00	
X14.0	4.0	0.01	190.00	10.00	10.00	10.00		
X2 843.0 GR7082.5		7062 50		7062 50	110.00	7082.50	190.00	
X1250				20.00		20.00	190.00	
X2 843.0)							
	0.01					7083.00	190.00	
X1260 X2 833.0		0.01	190.00	10.00	10.00	10.00		
GR7083.2	0.01	7063.24	80.00	7063.24	110.00	7083.24	190.00	
X1270	4.0	0.01	190.00	10.00	10.00	10.00		
X2 B23.0						****		
GR7083.5 X1280				10.00		7083.49	190.00	
X2 813.0		0.01	190.00	10.00	10.00	10.00		
GR7083.7	0.01	7063.73	80.00	7063.73	110.00	7083.73	190.00	
X1290		0.01	190.00	10.00	10.00	10.00		
X2 803.0	0.01	7063 98	80.00	7063.98	110.00	7083.98	190.00	
X1300				10.00			190.00	
X2 792.0								
	0.01					7084.22	190.00	
X1310 X2 782.0	4.0	0.01	190.00	10.00	10.00	10.00		
GR7084.5		7064.47	80.00	7064.47	110.00	7084.47	190.00	
X1320			190.00			10.00		
X2 772.0		2001 20		2001 21		2004 21		
GR7084.7 X1330				10.00		7084.71	190.00	
X2 761.0)							
GR7085.0	0.01	7064.96	80.00	7064.96	110.00	7084.96	190.00	
X1340	4.0	0.01	190.00	10.00	10.00	10.00		
X2 751.0	0.01	7065.20	80.00	7065.20	110.00	7085.20	190.00	
X1 360	4.0	0.01	190.00	20.00	20.00	20.00		
X2 751.0								
GR7086.1 X1 380				7066.12			190.00	
X2 751.0		0.01	150.00	20.00	20.00	20.00		
GR7087.0		7067.04	80.00	7067.04	110.00	7087.04	190.00	
X1 400		0.01	190.00	20.00	20.00	20.00		
X2 751.0 GR7088.0		7067.96	80.00	7067.96	110.00	7087.96	190.00	
X1 420	4.0		190.00		20.00		2,,,,,	
X2 751.0								
GR7088.9				7068.88			190.00	
X1440 X2 751.0	4.0	. 0.01	190.00	20.00	20.00	20.00		
GR7089.8		7069.80	80.00	7069.80	110.00	7089.80	190.00	
X112.0	4.0	0.01	190.00	60.00	60.00	60.00		
X2 751.0 GR7091.3		7071.30	80.00	7071.30	110 00	7091.30	190.00	
X1540	4.0	0.01	190.00	40.00		40.00	190.00	
X2 751.0	1							
GR7092.3		7072.30		7072.30		7092.30	190.00	
X1 590 X2 751.0	4.0	0.01	190.00	50.00	50.00	50.00		
GR7093.6		7073.56	80.00	7073.56	110.00	7093.56	190.00	
X1 690	4.0	0.01		100.00				
X2 751.0 GR7096.1		7076 00	90.00	7076.08	110.00	2006 00	100 00	
X1790	4.0					7096.08	190.00	
X2 751.0								

TABLE F-17. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL Q (continued).

GR7098.6 X1 870				7078.60 80.00		7098.60 80.00	
X2 751.0 GR7100.6 X1970		7080.60		7080.60		7100.60 100.00	
X2 751.0 GR7103.1 X1 980		7083.10 0.01		7083.10		7103.10 10.00	190.00
X2 746.9 GR7103.4 X11080		7083.35 0.01		7083.35 100.00		7103.35 100.00	190.00
X2 706.0 GR7105.9 X1 1095		7085.90 0.01		7085.90 15.00		7105.90 15.00	190.00
X2 706.0 GR7107.3 X1 1115 X2 706.0	4.0	7087.31		7087.31 20.00		7107.31 20.00	190.00
GR7109.2 X1 1135 X2 706.0	0.01	7089.18 0.01		7089.18 20.00		7109.18 20.00	190.00
GR7111.1 X1 1155 X2 706.0	0.01	7091.05 0.01	80.00 190.00	7091.05 20.00	110.00 20.00	7111.05	190.00
GR7112.9 X11175 X2 706.0		7092.93 0.01		7092.93 20.00			190.00
GR7114.8 X11180 X2 706.0	0.01 4.0			7094.80 5.00			190.00
GR7114.8 X11195 X2 706.0		7094.81		7094.81 15.00			190.00
GR7114.9 X11215 X2 706.0				7094.85		7114.85	170.00
GR7114.9 X1 1220 X2 706.0	3.0	7094.90 0.01	146.67	7114.90 5.00	5.00		
GR7114.9 X11230 X2 706.0	3.0	0.01	120.00	7114.93	10.00		
GR7115.0 X1 1240 X2 706.0	3.0	0.01	110.00		10.00	10.00	
GR7115.0 X11250 X2 706.0	3.0	7095.00	100.00		110.00		
GR7115.0 X11270 X2 706.0	3.0		100.00		20.00	20.00	
GR7115.0 X1 1275 X2 706.0	3.0		96.00	7115.00	5.00	5.00	
GR7115.0 X1 1285 X2 706.0	3.0	0.01		10.00	96.00	10.00	
GR7115.0 X111.0 X2 706.0	4.0	0.01		10.00	88.00	10.00	
GR7115.0 X11320 X2 706.0	4.0	7095.00 0.01	80.00	7095.00 25.00	25.00	7115.00 25.00	80.00
GR7115.0 X1 1325 X2 706.0	4.0	7095.00	102.00		5.00	7115.00 5.00	80.00
GR7115.0 X1 1330 X2 706.0	4.0		124.00		5.00	7115.00 5.00	102.00
GR7115.0 X1 1335 X2 706.0	4.0	7095.00 0.01	146.00	5.00	5.00	7115.00 5.00	124.00
GR7115.0 X1 1340 X2 706.0	4.0	7095.00 0.01	168.00	7095.00 5.00	5.00	5.00	146.00
GR7115.0 X11345 X2 706.0	0.01 4.0	7095.00 0.01	190.00	7095.00 5.00	5.00	7115.00 5.00	168.00
GR7115.0 X11348	0.01	7095.00 0.01	80.00 190.00	7095.00 3.00	110.00 3.00	7115.00 3.00	190.00

TABLE F-17. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL Q (continued).

X2 700.0 GR7115.3 X11352	0.01	7095.34	80.00	7095.34 4.00	110.00	7115.34	190.00
X2 650.0	4.0	0.01	170.00	4.00	1.00		
	0.01	7095.80	80.00	7095.80	110.00	7115.80	190.00
	4.0	0.01	190.00	4.00	4.00	4.00	
X2 600.0	0 01	7096 26	RO OO	7096.26	110.00	7116.26	190.00
GR7116.3 X11360				4.00			
X2 550.0							
				7096.72			190.00
	4.0	0.01	190.00	4.00	4.00	4.00	
X2 500.0 GR7117.2	0 01	7097 18	80.00	7097.18	110.00	7117.18	190.00
X11368	4.0	0.01	190.00	4.00	4.00	4.00	
X2 450.0							
	0.01	7097.64	80.00	7097.64	110.00	7117.64	190.00
X11372 X2 400.0	4.0	0.01	190.00	4.00	4.00	4.00	
	0.01	7098.10	80.00	7098,10	110.00	7118.10	190.00
				4.00			
X2 480.0							
GR7118.5 X11380	0.01	7098.55	80.00	7098,55	110.00	7118.55	190.00
X2 460.0	4.0	0.01	190.00	4.00	4.00	4.00	
GR7119.0	0.01	7099.01	80.00	7099.01	110.00	7119.01	190.00
X11384	4.0	0.01	190.00	4.00	4.00	4.00	
X2 440.0							
GR7119.5 X11388				7099.47		4.00	190.00
X2 420.0	4.0	0.01	190.00	4.00	4.00	3.00	
GR7119.9	0.01	7099.93	80.00	7099.93	110.00	7119.93	190.00
	4.0	0.01	190.00	4.00	4.00	4.00	
X2 400.0	0 01	7100 20	80.00	7100.39	110.00	7120 20	190.00
GR7120.4 X11396	4.0	0.01	190.00	4.00	4.00	4.00	190.00
X2 300.0	•••	****					
GR7120.9				7100.85			190.00
	4.0	0.01	190.00	4.00	4.00	4.00	
X2 290.0 GR7121.3	0 01	7101 31	80.00	7101.31	110.00	7121 31	190 00
X11404				4.00			150.00
X2 280.0							
GR7121.8	0.01	7101.77	80.00	7101.77	110.00	7121.77	190.00
X11408 X2 270.0	4.0	0.01	190.00	4.00	4.00	4.00	
GR7122.2	0.01	7102.22	80.00	7102.22	110.00	7122.22	190.00
X11412	4.0	0.01	190.00	4.00	4.00	4.00	
X2 260.0							
				7102.68			190.00
X11416 X2 250.0	4.0	0.01	190.00	4.00	4.00	4.00	
GR7123.1	0.01	7103.14	80.00	7103.14	110.00	7123.14	190.00
X11420	4.0	0.01	190.00	4.00	4.00	4.00	
X2 243.0 GR7123.6	0.01	7107 60	90 00	7107 60	110 00	7122 60	100.00
X11427	4.0	0.01	190.00	7.00	7.00	7123.60	190.00
Y2 243 0							
GR7125.0	0.01	7105.05	80.00	7105.05	110.00	7125.05	190.00
X11434 X2 243.0	4.0	0.01	190.00	7.00	7.00	7.00	
GR7126.5	0.01	7106.50	80.00	7106.50	110.00	7126.50	190.00
X11444	4.0	0.01	190.00	10.00	10.00	10.00	
X2 243.0		7.00					
GR7128.1 X11454	0.01 4.0	7108.15	80.00	7108.15	110.00	7128.15	190.00
X2 243.0	4.0	0.01	±50.00	10.00	10.00	10.00	
GR7129.8	0.01	7109.80	80.00	7109.80	110.00	7129.80	190.00
X1 1455	4.0	0.01	189.05	1.00	1.00	1.00	
X2 243.0	0.01	7109.80	80.00	7100 00	100.05	7120 00	100 05
GR7129.8 X11475	4.0		170.00	7109.80	20.00	7129.80 20.00	189.05
X2 243.0		2.01	,		-5.00	_5.00	
GR7129.9		7109.90		7109.90		7129.90	170.00
X11485. X2 243.0	3.0	0.01	160.00	10.00	10.00	10.00	
GR7130.0	0.01	7110.00	80.00	7130.00	160.00		
X11490	3.0	0.01	144.00	5.00	5.00	5.00	
X2 243.0		5.4. 7 =:					
GR7130.0	0.01	7110.01	72.00	/130.01	144.00		

TABLE F-17. HEC-2 SUBCRITICAL INPUT FILE FOR CHANNEL Q (continued).

X11499	3.0	0.01	120.00	9.00	9.00	9.00	
X2 243.0							
GR7130.0	0.01	7110.03	60.00	7130.03	120.00		
X1 1500	3.0	0.01	119.02	1.00	1.00	1.00	
X2 243.0							
GR7130.0	0.01	7110.03	59.51	7130.03	119.02		
X1 1510	3.0				10.00		
X2 243.0							
GR7130.0	0.01	7110.05	54.63	7130.05	109.27		
X1 1520	3.0				10.00		
X2 243.0	2.0	0.01		10.00	10,00	10.00	
GR7130.1	0.01	7110.07	10 76	7130.07	99.51		
X1 1530	3.0			10.00	10.00	10.00	
X2 243.0	3.0	0.01	09.70	10.00	10.00	10.00	
GR7130.1	0 01	7110.08	44 00	7130.08	89.76		
X110.0	4.0	0.01	80.00	10.00	10.00	10.00	
X2 243.0					10.00	7120	
GR7130.1		7110.10		7110.10		7130.10	80.00
X1 1543	4.0	0.01	87.50	3.00	3.00	3.00	
X2 243.0							
GR7130.1		7110.10		7110.10		7130.10	87.50
X1 1548	4.0	0.01	100.00	5.00	5.00	5.00	
X2 243.0							
GR7130.1		7110.11		7110.11		7130.11	100.00
X1 1553	4.0	0.01	112.50	5.00	5.00	5.00	
X2 243.0							
GR7130.1			53.68			7130.12	112.50
X1 1558	4.0	0.01	125.00	5.00	5.00	5.00	
X2 243.0							
GR7130.1	0.01	7110.12	58.95	7110.12	66.05	7130.12	125.00
X1 1563	4.0	0.01	137.50	5.00	5.00	5.00	
X2 243.0							
GR7130.1	0.01	7110.13	64.21	7110.13	73.29	7130.13	137.50
X1 1568	4.0		150.00		5.00		-
X2 243.0							
GR7130.1	0.01	7110.14	69.47	7110.14	80.53	7130.14	150.00
X1 1573	4.0		162.50	5.00	5.00		
X2 243.0		0.01	102.00	5.50	0.00	2.50	
GR7130.1	0.01	7110 14	74.74	7110 14	97 76	7130.14	162.50
X11578	4.0	0.01			5.00	5.00	102.50
X2 243.0	4.0	0.01	1/3.00	3,00	5.00	5.00	
GR7130.1	0.01	7110 15	90.00	7110 15	96 00	7130.15	175.00
X11590	4.0			12.00			1/5.00
	4.0	0.01	130.00	14.00	12.00	12.00	
X2 243.0	0.01	7110 00	00.00	7110 20	110.00	7130.20	100.00
GR7130.2	0.01	/110.20	80.00	1110.20	110.00	,130.20	190.00
EJ							
ER							

TABLE F-18. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL Q.

	CHANNEL		UDES CRO				GH HC5-10	
T3 J1	PMF - S	UPERCRITI	1.0	-1.0			243.0	7118.0
J2 -1		0 ~1.0		• •				
NC 0.035 X11590	0.03			12.00		12.00		
X2 243.0								
GR7130.2 X11578	0.03	1 7110.20 0 0.01		7110.20		7130.20 5.00		
X2 243.0		0.01	173.00	3.00	3.00	3.00		
GR7130.1		7110.15		7110.15		7130.15		
X1 1573 X2 243.0	4.0	0.01	162.50	5.00	5.00	5.00		
GR7130.1	0.0	7110.14		7110.14		7130.14	162.50	
X1 1568 X2 243.0	4.0	0.01	150.00	5.00	5.00	5.00		
GR7130.1		7110.14	69.47	7110.14	80.53	7130.14	150.00	
X1 1563	4.0	0.01	137.50	5.00	5,00	5.00		
X2 243.0 GR7130.1		7110.13	64.21	7110.13	73.29	7130.13	137.50	
X1 1558	4.0			5.00				
X2 243.0 GR7130.1		7110.12	50 05	7110 12	66 05	7130.12	125.00	
X1 1553	4.0						123.00	
X2 243.0		7110 10		7110 10				
GR7130.1 X1 1548	4.0	7110.12		7110.12 5.00		7130.12 5.00	112.50	•
X2 243.0								
GR7130.1 X1 1543	0.01 4.0	7110.11		7110.11		7130.11	100.00	
X2 243.0		0.01	67.50	3.00	3.00	3.00		
GR7130.1		7110.10		7110.10		7130.10	87.50	
X110.0 X2 243.0	4.0	0.01	80.00	10.00	10.00	10.00		
GR7130.1		7110.10		7110.10		7130.10	80.00	
X1 1530 X2 243.0	3.0	0.01	89.76	10.00	10.00	10.00		
GR7130.1	0.01	7110.08	44.88	7130.08	89.76			
X1 1520	3.0	0.01	99.51	10.00	10.00	10.00		
X2 243.0 GR7130.1	0.01	7110.07	49.76	7130.07	99.51			
X1 1510	3.0			10.00	10.00	10.00		
X2 243.0 GR7130.0		7110.05	54 63	7130 05	100.03			
X1 1500	3.0			7130.05	109.27	1.00	•	
X2 243.0								
GR7130.0 X11499	3.0	7110.03		7130.03 9.00	119.02 9.00	9.00		
X2 243.0					•	3100		
GR7130.0 X11490		7110.03			120.00	E 00		
X2 243.0	3.0	0.01	144.00	5.00	3.00	5.00		
GR7130.0 X11485		7110.01		7130.01	144.00			
X2 243.0	3.0	0.01	160.00	10.00	10.00	10.00		
GR7130.0		7110.00		7130.00	160.00			
X11475 X2 243.0	4.0				20.00	20.00		
GR7129.9		7109.90				7129.90	170.00	
X1 1455 X2 243.0	4.0	0.01	189.05	1.00	1.00	1.00		
GR7129.8	0.01	7109.80	80.00	7109.80	109.05	7129.80	189.05	
X11454 X2 243.0	4.0	0.01	190.00	10.00	10.00	10.00		
GR7129.8	0.01	7109.80	80.00	7109.80	110.00	7129.80	190.00	
X11444	4.0	0.01	190.00	10.00				
X2 243.0 GR7128.1	0.01	7108.15	80.00	7108.15	110 00	7128.15	190.00	
X11434	4.0			7.00			_,	
X2 243.0 GR7126.5	0.01	7106.50	90.00	7106 50	110 00	7106 50	100.00	
X11427	4.0	0.01	190.00	7.00	7.00	7126.50	190.00	
X2 243.0								
GR7125.0 X11420	4.0	7105.05	80.00 190.nn	/105.05 4.00	110.00	7125.05	190.00	
X2 243.0								-
GR7123.6 X11416	0.01 4.0	7103.60	80.00	7103.60	110.00	7123.60	190.00	
X2 250.0				4.00	4.00	4.00		

TABLE F-18. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL Q (continued).

GR7123.1 X11412 X2 260.0		7103.14		7103.14		7123.14	190.00
GR7122.7 X11408 X2 270.0	0.01 4.0			7102.68 4.00		7122.68 4.00	190.00
GR7122.2 X11404 X2 280.0	0.01 4.0	7102.22 0.01		7102.22 4.00		7122.22 4.00	190.00
GR7121.8 X11400	0.01	7101.77 0.01		7101.77 4.00		7121.77 4.00	190.00
X2 290.0 GR7121.3 X11396 X2 300.0	0.01 4.0			7101.31		7121.31 4.00	190.00
GR7120.9 X11392 X2 400.0		7100.85				7120.85 4.00	190.00
GR7120.4 X11388 X2 420.0				7100.39 4.00		7120.39 4.00	190.00
GR7119.9 X11384 X2 440.0				7099.93 4.00		7119.93 4.00	190.00
GR7119.5 X11380 X2 460.0	0.01 4.0			7099.47 4.00		7119.47 4.00	190.00
GR7119.0 X11376 X2 480.0	0.01 4.0	7099.01		7099.01 4.00		7119.01	190.00
GR7118.5 X11372 X2 400.0	0.01 4.0	7098.55 0.01	80.00 190.00	7098.55 4.00		7118.55 4.00	190.00
GR7118.1 X11368 X2 450.0	4.0	0.01	190.00		4.00		
GR7117.6 X11364 X2 500.0	4.0	0.01	190.00	4.00	110.00	7117.64	190.00
X11360 X2 550.0	4.0	7097.18	190.00	4.00	4.00	7117.18	190.00
GR7116.7 X11356 X2 600.0	4.0		190.00	4.00	4.00	7116.72	190.00
GR7116.3 X11352 X2 650.0	4.0		190.00	4.00	4.00		190.00
X11348 X2 700.0	4.0		190.00	7095.80 3.00	3.00	7115.80 · 3.00	190.00
GR7115.3 X11345 X2 706.0	4.0	7095.34	190.00		5.00	7115.34 5.00	190.00
GR7115.0 X1 1340 X2 706.0	. 4.0	7095.00	168.00	7095.00 5.00	5.00	7115.00 5.00	190.00
GR7115.0 X1 1335 X2 706.0	4.0	7095.00	146.00	5.00	5.00	7115.00	168.00
GR7115.0 X1 1330 X2 706.0	4.0	7095.00	124.00	7095.00	5.00	7115.00	146.00
GR7115.0 X1 1325 X2 706.0	4.0	7095.00	102.00	7095.00	5.00	7115.00	124.00
GR7115.0 X11320 X2 706.0	4.0	7095.00	80.00	7095.00	25.00	7115.00 25.00	102.00
GR7115.0 X111.0 X2 706.0	4.0	7095.00	80.00	7095.00	10.00	7115.00	80.00
GR7115.0 X1 1285 X2 706.0	3.0	7095.00 0.01	88.00	7095.00 10.00	40.00 10.00	7115.00	80.00
GR7115.0 X1 1275 X2 706.0	3.0	7095.00 0.01	96.00	7115.00 5.00	5.00	5.00	
GR7115.0 X11270	0.01 3.0	7095.00 0.01		7115.00 20.00	96.00 20.00	20.00	

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TABLE F-18. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL Q (continued).

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X2 706.0	0.01	7095 00	E0 00	7115 00	100 00		٠.
GR7115.0 X11250	3.0		100.00	7115.00			
X2 706.0							
GR7115.0		7095.00		7115.00			
X1 1240	3.0	0.01	110.00	10.00	10.00	10.00	
X2 706.0	0.01	7095.00	65 00	7115.00	110.00		
GR7115.0 X11230	3.01	0.01	120.00	10.00	10.00		
X2 706.0	5.0	0.01	120.00	10.00	20.00	20.00	
GR7115.0	0.01	7095.00	60.00	7115.00	120.00		
X1 1220	3.0	0.01	146.67	5.00	5.00	5.00	
X2 706.0							
GR7114.9		7094.93 0.01		7114.93			
X11215 X2 706.0	3.0	0.01	160.00	20.00	20.00	20.00	
GR7114.9	0.01	7094.90	80.00	7114.90	160.00		
X11195	4.0	0.01	170.00	15.00	15.00	15.00	
X2 706.0							
GR7114.9		7094.85		7094.85		7114.85	170.00
X11180	4.0	0.01	190.00	5.00	5.00	5.00	
X2 706.0 GR7114.8	0 01	7094.81	90.00	7094.81	110 00	7114 91	190.00
X11175		0.01		20.00			190.00
X2 706.0	7			••••			
GR7114.8	0.01	7094.80	80.00	7094.80			190.00
X1 1155	4.0	0.01	190.00	20.00	20.00	20.00	
X2 706.0		7000					
				7092.93		7112.93	190.00
X1 1135 X2 706.0	4.0	0.01	190.00	20.00	20.00	20.00	
GR7111.1	0.01	7091.05	80.00	7091.05	110.00	7111.05	190.00
X1 1115		0.01		20.00			130.00
X2 706.0							
GR7109.2	0.01	7089.18	80.00	7089.18		7109.18	190.00
X1 1095	4.0	0.01	190.00	15.00	15.00	15.00	
X2 706.0	0.01	7007 21	00.00	7007 31	110 00	7107 31	100.00
GR7107.3 X11080	4.0	7087.31		7087.31		7107.31	190.00
X11080 X2 706.0	4.0	0.01	190.00	100.00	100.00	100.00	
GR7105.9	0.01	7085.90	80.00	7085.90	110.00	7105.90	190.00
X1 980	4.0		190.00		10.00		
X2 746.9							
GR7103.4		7083.35		7083.35		7103.35	190.00
X1970 X2 751.0	4.0	0.01	190.00	100.00	100.00	100.00	
GR7103.1	0 01	7083.10	80.00	7083.10	110.00	7103.10	190.00
X1 870	4.0			80.00		80.00	130.00
X2 751.0							
		7080.60		7080.60		7100.60	190.00
X1790	4.0	0.01	190.00	100.00	100.00	100.00	
X2 751.0 GR7098.6	. 0 01	7078.60	80.00	7078.60	110.00	7098.60	190.00
X1 690		0.01		100.00		100.00	130.00
X2 751.0	• • •			20000			
GR7096.1	0.01	7076.08	80.00	7076.08	110.00	7096.08	190.00
X1 590	4.0	0.01	190.00	50.00	50.00	50.00	
X2 751.0	0 01	7072 56	00.00	7073.56	110 00	2002 56	100.00
GR7093.6 X1540				40.00		7093.56	190.00
X2 751.0	4.0	0.01	130.00	40.00	10.00	40.00	
GR7092.3	0.01	7072.30	80.00	7072.30	110.00	7092.30	190.00
X112.0	4.0	0.01	190.00	60.00	60.00	60.00	
X2 751.0							
GR7091.3 X1440		7071.30		7071.30		7091.30	190.00
X2 751.0	4.0	0.01	190.00	20.00	20.00	20.00	
GR7089.8	0.01	7069.80	80.00	7069.80	110.00	7089.80	190.00
X1 420	4.0	0.01	190.00	20.00	20.00	20.00	
X2 751.0							
GR7088.9		7068.88		7068.88		7088.88	190.00
X1 400	4.0	0.01	190.00	20.00	20.00	20.00	
X2 751.0	0.01	7067 00	. 00 00	2067 06	110 00	7007 05	100.00
GR7088.0 X1 380	4.0	7067.96	190.00	7067.96 20.00	20.00	7087.96 20.00	190.00
X2 751.0	4.0	0.01	150.00	20.00	20.00	20.00	
GR7087.0	0.01	7067.04	80.00	7067.04	110.00	7087.04	190.00
X1 360	4.0		190.00	20.00	20.00	20.00	
X2 751.0							
GR7086.1	0.01	7066.12	80.00	7066.12	110.00	7086.12	190.00

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TABLE F-18. HEC-2 SUPERCRITICAL INPUT FILE FOR CHANNEL Q (continued).

	•		1.00				
X1340	4.0	0.01	190.00	10.00	10.00	10.00	
X2 751.0							
GR7085.2	0.01	7065.20	80.00	7065.20	110.00	7085.20	190.00
X1330	4.0	0.01	190.00	10.00	10.00	10.00	
X2 761.0			7 -				
GR7085.0	0.01	7064.96	80.00	7064.96	110.00	7084.96	190.00
X1320	4.0		190.00		10.00		
X2:772.0		4.					
GR7084.7		7064.71	80.00	7064.71	110.00	7084.71	190.00
X1310		0.01		10.00		10.00	130.00
X2 782.0	•		-,	20.00	10.00	10.00	
GR7084.5	0.01	7064 47	80 00	7064.47	110 00	7084.47	190.00
X1300		0.01		10.00		10.00	190.00
X2 792.0	1.0	0.01	190.00	10.00	10.00	10.00	
GR7084.2	0 01	7064: 22	. 80 00	7064 22	110.00	7084.22	100 00
X1290						10.00	190.00
X2 803.0		0.01	130,00	. 10.00	10.00	10.00	
GR7084.0	0.01	7063.00	90.00	.7062 00	110 00	7083.98	190.00
	4.01	7003.96	100.00	14003.90	110.00		190.00
X1280	4.0	. 0.01	130.00	10.00	10.00	10.00	
X2 813.0	0.01	7060 70					
GR7083.7	0.01	1063.13	80.00	1063.13		7083.73	190.00
		0.01	190.00	10.00	10.00	10.00	
X2 023.0			· 227 11				
GR7083.5	0.01	7063.49	80.00	7063.49		7083.49	190.00
X1260		0.01	190.00	10.00	10.00	10.00	
X2 833.0		1.7					
GR7083.2				7063.24	110.00	7083.24	190.00
		0.01	190.00	20.00	20.00	20.00	
X2 843.0							
.GR7083.0		7063,00	80.00	7063.00		7083.00	190.00
X14.0	4.0	0.01	190.00	10.00	10.00	10.00	
X2 843.0							
GR7082.5		7062.50		7062.50	110.00	7082.50	190.00
X1220	4.0	0.01	190.00	90.00	90.00	90.00	
X2 843.0							
GR7082.4		7,062.38			110.00	7082.38	190.00
X1130	4.0	0.01	190.00	30.00	30.00	30.00	
X2 843.0							
GR7081.3		7061.30	80.00	7061.30	110.00	7081.30	190.00
X1100	4.0	0.01	190.00	10.00	10.00	10.00	
X2 843.0							
GR7080.9	, 0.01	7060.94	80.00	7060.94	110.00	7080.94	190.00
X190		0.01					
X2 843.0							
GR7080.8	0.01	7060.82	80.00	7060.82	110.00	7080.82	190.00
EJ						-	
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