

**WESTERN NUCLEAR, INC.**

**SHERWOOD PROJECT**  
P. O. BOX 392 • WELLPINIT, WASHINGTON 99040 • (509) 747-2082

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DEC 20 1999



DIV. OF RADIATION PROTECTION  
XXXXXXXX 258-4521

December 17, 1999

Mr. Gary Robertson  
Washington Department of Health  
Waste Management Section  
Division of Radiation Protection  
P.O. Box 47827  
Airdustrial Park, Building 5  
Olympia, Washington 98504-7827

**RE: Radioactive Materials License WN-I0133-1; License Conditions 22 and 29;  
Request for license termination: submission of discussion and calculation  
to demonstrate: 1) the erosional stability provided by the existing vegetation  
on the reclaimed impoundment; and 2) the level of conservatism inherent in  
in riprap sizing analyses for the diversion channel.**

Dear Mr. Robertson:

As a result of our discussions during our November 30 meeting in Spokane and during a telephone conversation on December 6 regarding issues of erosional stability of the reclaimed impoundment, please find attached the discussions and calculations requested by the Washington Department of Health (WDOH). These attachments provide additional support for prior submittals which have demonstrated the stability of the reclaimed tailing impoundment structure.

Should have any questions regarding the attachments or need additional information, please contact me at your earliest convenience.

Sincerely

Brad K. DeWaard, Resident Agent

/bd  
Attachments

cc: L. J. Corte, Esq. L. L. Miller (SMI)  
E. M. Schern H. W. Shaver, Esq. (S&L)  
WNI Central File (L.C. 22 & 29)

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OSP

Prepared For:  
**WESTERN NUCLEAR, INC.**  
**SHERWOOD PROJECT**  
**Wellpinit, WA**

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**RESPONSE TO**  
**WASHINGTON DEPARTMENT OF HEALTH**  
**VERBAL QUESTIONS OF DECEMBER 6, 1999**

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Prepared By:  
**Shepherd Miller, Inc.**  
**3801 Automation Way, Suite 100**  
**Fort Collins, Colorado 80525**



**SHEPHERD MILLER**  
INCORPORATED

**DECEMBER 1999**

DEC 20 1999

A telephone conference call was held on December 6, 1999 to discuss all remaining issues that need to be addressed before the Sherwood license termination process can be completed. Two issues were raised. The specific request along with Western Nuclear's response for each of the request is provided below.

**Issue 1. Provide calculations and discussion to show that the current vegetation on the surface and margins is sufficient for the next few years until the vegetation reaches the required cover to provide erosional stability for the PMP event. The department suggests that the erosional stability be evaluated for 100 year, 1000 year and 10,000 year precipitation events. The Washington Department of Ecology – Dam Safety Section (WDOE-DSS) has calculated various precipitation events for the site, which has been transmitted to WNI.**

The Monitoring and Stabilization Plan (MSP) final report was submitted to the WDOH in the "Request For License Termination – Final Data Report" dated November 15, 1999. The MSP final report concluded that the total vegetation on the surface was 35.8% (at the lower 80% confidence level – excluding the pond area which is inherently stable). This is essentially the same as the target success level of 36%. While perennial species cover did not increase each of the previous three years, the plant community is maturing as anticipated. Much of the decline in perennial species from 1998 to 1999 is attributable to the biennial life cycle of sweet clover and does not reflect any deficiency in the revegetation effort nor indicate some unanticipated natural failure or control mechanism.

Similarly, the margin areas (excluding the inherently stable areas with shallow quartz monzonite bedrock) have vegetation that is very close to the required levels (Total vegetation of 37.6% versus the target level of 39%). The perennial species on the margins have increased each of the last 3 years.

Therefore, while the vegetation on the surface and margins do not strictly meet the numerical success criteria, the values are extremely close to the numerical targets. Additionally, the diversity of perennial species including the abundance of trees and shrubs on the surface and

margins indicate that the plant community will continue to develop, and vegetation at levels much greater than the target levels will be achieved in the next few years.

As requested, an erosional stability analysis was performed using precipitation events less than the PMP but with very unlikely probability of occurring during the next several years. It needs to be understood that the analysis using precipitation events less than the PMP is not intended to evaluate the long-term performance of the cover. This evaluation is to show that the cover and margins are erosionally stable with the current vegetation cover for events that are highly unlikely in the interim until the vegetation meets the previously approved numerical criteria. Given that the current cover measurements are only a few percentage points away from the target levels and that the diversity of species indicates that the site is progressing very well, the site should have cover values well in excess of the numerical standards within a year or two.

Erosional stability analyses were performed on the cover and margins using precipitation events for the 100-yr, 1000-yr and the 10,000-yr storm events as supplied by the Washington Department of Ecology – Dam Safety Section (via letter dated December 16, 1999). The 6-hour storm event was used since it was previously determined that the 6-hour PMP was the critical storm event for the site. The peak intensity for each storm event was estimated using the depth-duration curve developed for the PMP. The peak intensity is 1.56 times the total precipitation for the design storm, therefore each of the precipitation events was multiplied by 1.56 to determine the peak intensities. A conservative flow concentration factor of 3.0 was used for all the erosional stability calculations. This factor is considered conservative because overland flow conditions can typically be characterized as sheet flow. Sheet flow concentration factors typically range between 2 and 2.5. Visual observations of the site indicate that flow concentration is not occurring and therefore a concentration factor of 3 is very conservative.

The same techniques used in the previous analyses were used to determine the required vegetative cover for each of the storm events. The analyses are attached and are summarized in Table 1.

As can be seen, the existing vegetal coverage for the impoundment margins and cover is able to provide erosional protection for the 100-yr, 1000-yr, and 10,000 yr precipitation events. The

impoundment margin cover does not require any vegetal cover to be erosionally stable for the 100-yr, 1000-yr, and 10,000-yr precipitation. The impoundment margins require a maximum of 34 percent vegetal cover to be erosionally stable for the 10,000-yr precipitation event.

**Table 1 Summary of Vegetal Coverage Required for Erosional Stability**

	100 Year Storm	1,000 Year Storm	10,000 Year Storm
<b>Margins</b>			
Storm Event (in)	2.19	3.13	4.31
Peak Intensity (in/hr)	3.43	4.90	6.75
% Vegetal Cover Required	32-33	33	33-34
Existing Vegetal Cover (%)	37.5	37.5	37.5
Vegetal Cover Target (%)	39	39	39
<b>Cover</b>			
Storm Event (in)	2.19	3.13	4.31
Peak Intensity (in/hr)	3.43	4.90	6.75
% Vegetal Cover Required	0	0	0
Existing Vegetal Cover (%)	35.8	35.8	35.8
Vegetal Cover Target (%)	36	36	36

**Issue 2: To assist the WDOH with determining the level of conservatism inherent in the analyses used to size the riprap in the diversion channel, provide calculations for required riprap sizing using the 1000-yr and the 10,000-yr precipitation events as calculated by the WDOE-DSS.**

The diversion channels were designed to be erosionally stable during the conveyance of the runoff from the Possible Maximum Precipitation (PMP) event. Two different scenarios were evaluated. First, the channel dimensions and riprap sizing was calculated assuming no vegetation in the channel. This leads to a situation with high velocities and low flow depths. This scenario was used to determine the rock size.

The other scenario that was evaluated was for the situation with trees in the channel. This scenario leads to conditions with low velocities and high water levels. The overall depth of the channel was determined from the scenario.

The performance of the channel using 1000-yr and 10,000-yr precipitation events was previously analyzed for the scenario with trees. This analysis was submitted to WDOH in the Sherwood Project Response to WDOH Comments on the December 1994 Tailing Reclamation Plan (dated August 1995). The results of those analyses were summarized in the August 1995 report (attached) and showed that the 1000-yr and the 10,000-yr event lead to flow conditions that are much less than the PMP and the diversion channel has from 3.5 feet of freeboard at the upstream end of the channel to 10.8 feet near the outlet for the 10,000 year event.

An evaluation of the diversion channel was also performed using the assumption that the channel would not have trees. The PMP analysis was used to design the rock sizing for this scenario and the appropriately sized rock was placed and documented in the Tailing Reclamation Plan Completion Report (dated June, 1997). The evaluation using the 100-yr, 1000-yr, and the 10,000-year precipitation events was performed using the same analytical techniques and input parameters as used in the original PMP analysis. The results of the analysis indicates that much smaller rock would be required for the 100-yr, 1000-yr, and 10,000-yr events. The evaluation is attached and results are summarized on Table 2.

**Table 2 – Scenario With No Trees (Roughness Coefficient  $n = 0.031$ )**

Diversion Channel Reach	Channel Flow (cfs)	Flow Depth (ft)	Maximum Velocity (fps)	Calculated Riprap Size (in)	Design Riprap Size (in)
<b>100 Year Storm Event</b>					
Above A	21	1.26	3.14	0.25	3
A-B	26	1.35	3.38	0.25	3
B-C	93	2.24	4.43	0.50	3
C-D	191	2.69	6.10	1.00	5
D-E	219	3.80	3.68	0.25	3
E-F	254	4.00	3.87	0.25	3
F-G	309	4.34	4.08	0.50	3
G-H	312	4.34	4.08	0.50	3
Below H	312	4.08	4.57	0.25	3

**Table 2 – Scenario With No Trees (Roughness Coefficient n = 0.031) (continued)**

<b>Diversion Channel Reach</b>	<b>Channel Flow (cfs)</b>	<b>Flow Depth (ft)</b>	<b>Maximum Velocity (fps)</b>	<b>Calculated Riprap Size (in)</b>	<b>Design Riprap Size (in)</b>
<b>1,000 Year Storm Event</b>					
Above A	47	1.70	3.86	0.50	3
A-B	58	1.87	3.93	0.50	3
B-C	208	3.03	5.41	0.75	3
C-D	428	3.88	6.89	2.00	5
D-E	490	5.24	4.51	0.50	3
E-F	570	5.49	4.81	0.50	3
F-G	693	5.95	5.07	0.75	3
G-H	698	5.95	5.07	0.75	3
Below H	698	5.67	5.54	0.75	3
<b>10,000 Year Storm Event</b>					
Above A	86	2.13	4.5	0.50	3
A-B	105	2.35	4.55	0.50	3
B-C	377	3.80	6.32	1.00	3
C-D	779	4.87	8.18	2.25	5
D-E	893	6.72	5.19	0.75	3
E-F	1039	7.00	5.61	0.75	3
F-G	1264	7.57	5.93	1.00	3
G-H	1272	7.57	5.93	1.00	3
Below H	1271	7.26	6.39	0.75	3

**MARGIN AND SURFACE STABILITY ANALYSES**



# Design of Soil Covers; Horton/NRC Method (100 YR)

(NRC STP August 1990; Appendix A)  
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# Tailing Impoundment Margins

Step	Parameter	Symbol	Units	Strip A	Strip B	Strip C	Strip D	Strip E	Strip F	Strip G	Strip H	Source
1	soil classification			SW	SW	SW	SW	SW	SW	SW	SW	SMI December 1994; Appendix A
1	plasticity index	PI										
1	75% finer diameter (non-cohesive)	D <sub>75</sub>	in	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	SMI December 1994; Appendix A; Section A.3; Figure 3.13
1	soil grain roughness	n <sub>s</sub>	-	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	Temple 1987, Table 3.3
1	basic allowable effective stress	τ <sub>ab</sub>	psf									Temple 1987, Figure 3.3
1	void ratio	e	-	0.37	0.37	0.37	0.37	0.37	0.37	0.37	0.37	Design of Small Dams 1977, Table 8
1	void ratio correction factor	C <sub>e</sub>	-									Temple 1987, Figure 3.4
1	allowable soil stress	τ <sub>a</sub>	psf	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	Temple 1987, Table 3.3
1	retardance curve index	C <sub>i</sub>	-	6.79	6.79	6.79	6.79	6.79	6.79	6.79	6.79	SMI December 1994; Appendix C: Attachment C
1	cover factor	C <sub>F</sub>	-	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	SMI December 1994; Appendix C; Attachment C
1,2	slope length	L <sub>i</sub>	feet	130	130	190	145	130	110	185	155	SMI December 1994; Drawing 3; Rev 2
1	base elevation	E <sub>A</sub>	feet	2084.0	2079.0	2076.0	2084.0	2083.0	2082.0	2076.0	2078.0	SMI December 1994; Drawing 3; Rev 2
1	top elevation	E <sub>B</sub>	feet	2128.0	2117.0	2114.0	2132.0	2126.0	2118.0	2113.0	2108.0	SMI December 1994; Drawing 3; Rev 2
1	elevation difference	H	feet	44.0	38.0	38.0	48.0	43.0	36.0	37.0	30.0	
1,2	initial slope	S <sub>i</sub>	ft/ft	0.338	0.292	0.200	0.331	0.331	0.327	0.200	0.194	
1	runoff coefficient	C	-	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	Chow, 1988; Table 15.1.1
1	peak intensity	i	in/hr	3.43	3.43	3.43	3.43	3.43	3.43	3.43	3.43	=(PMP/10,000 yr total precip.) x (PMP <sub>max</sub> )
1	drainage basin area	A	acres	0.0030	0.0030	0.0044	0.0033	0.0030	0.0025	0.0042	0.0036	
1,3	flow concentration factor	F	-	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	varied so that S <sub>i</sub> = S <sub>s</sub>
	peak unit discharge (w/ concentration factor)	q <sub>o</sub>	cfs	0.006	0.006	0.009	0.007	0.006	0.005	0.009	0.008	
1	peak unit discharge (w/o concentration factor)	q	cfs	0.019	0.019	0.028	0.021	0.019	0.016	0.027	0.023	NRC STP; Section A.2.4.1
1,4	roughness factor (Manning's n)	n	-	0.035	0.035	0.035	0.035	0.035	0.035	0.035	0.035	Chow, 1959; Table 5-6
1	depth of flow	y	ft	0.014	0.014	0.020	0.015	0.014	0.012	0.020	0.018	NRC STP; Section 2.2; Step 6
1	allowable vegetal stress	τ <sub>va</sub>	psf	5.093	5.093	5.093	5.093	5.093	5.093	5.093	5.093	Temple 1987; Eqn 1.17 and Table 3.1
1	percent vegetal coverage	VC	-	33%	32%	32%	33%	33%	32%	32%	32%	39% = min. cover. (Pg. 40 of 2/96 Monitor & Stable Plan)
1	reduced allowable vegetal stress	τ <sub>var</sub>	psf	0.382	0.255	0.255	0.382	0.382	0.255	0.255	0.255	SMI, Jan 1996; Comment 6
1	maximum allowable shear stress	τ <sub>max</sub>	psf	0.382	0.255	0.255	0.382	0.382	0.255	0.255	0.255	= MAX(τ <sub>a</sub> , τ <sub>var</sub> )
5	peak intensity	i	in/hr	3.43	3.43	3.43	3.43	3.43	3.43	3.43	3.43	copied from above
6	stable slope	S <sub>s</sub>	ft/ft	0.505	0.283	0.204	0.459	0.505	0.326	0.209	0.243	NRC STP; Section 2.1.1
1	initial slope	S <sub>i</sub>	ft/ft	0.338	0.292	0.200	0.331	0.331	0.327	0.200	0.194	copied from step 1

initial to stable slope comparison

Stable    Stable    Stable    Stable    Stable    Stable    Stable    Stable    Stable    Stable    Is S<sub>i</sub> <= S<sub>s</sub> ?

# Design of Soil Covers; Horton/NRC Method (1000 YR)

# Tailing Impoundment Margins

(NRC STP August 1990; Appendix A)  
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Step	Parameter	Symbol	Units	Strip A	Strip B'	Strip C	Strip D	Strip E	Strip F	Strip G	Strip H	Source
1	soil classification			SW	SW	SW	SW	SW	SW	SW	SW	SMI December 1994; Appendix A
1	plasticity index	PI										
1	75% finer diameter (non-cohesive)	D <sub>75</sub>	in	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	SMI December 1994; Appendix A; Section A.3; Figure 3.13
1	soil grain roughness	n <sub>s</sub>	-	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	Temple 1987, Table 3.3
1	basic allowable effective stress	τ <sub>ab</sub>	psf									Temple 1987, Figure 3.3
1	void ratio	e	-	0.37	0.37	0.37	0.37	0.37	0.37	0.37	0.37	Design of Small Dams 1977, Table 8
1	void ratio correction factor	C <sub>e</sub>	-									Temple 1987, Figure 3.4
1	allowable soil stress	τ <sub>a</sub>	psf	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	Temple 1987, Table 3.3
1	retardance curve index	C <sub>i</sub>	-	6.79	6.79	6.79	6.79	6.79	6.79	6.79	6.79	SMI December 1994; Appendix C; Attachment C
1	cover factor	C <sub>F</sub>	-	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	SMI December 1994; Appendix C; Attachment C
1,2	slope length	L <sub>1</sub>	feet	130	130	190	145	130	110	185	155	SMI December 1994; Drawing 3; Rev 2
1	base elevation	E <sub>A</sub>	feet	2084.0	2079.0	2076.0	2084.0	2083.0	2082.0	2076.0	2078.0	SMI December 1994; Drawing 3; Rev 2
1	top elevation	E <sub>B</sub>	feet	2128.0	2117.0	2114.0	2132.0	2126.0	2118.0	2113.0	2108.0	SMI December 1994; Drawing 3; Rev 2
1	elevation difference	H	feet	44.0	38.0	38.0	48.0	43.0	36.0	37.0	30.0	
1,2	initial slope	S <sub>i</sub>	ft/ft	0.338	0.292	0.200	0.331	0.331	0.327	0.200	0.194	
1	runoff coefficient	C	-	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	Chow, 1988; Table 15.1.1
1	peak intensity	i	in/hr	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9	=(PMP/10,000 yr total precip.) x (PMP <sub>max</sub> )
1	drainage basin area	A	acres	0.0030	0.0030	0.0044	0.0033	0.0030	0.0025	0.0042	0.0036	
1,3	flow concentration factor	F	-	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	varied so that S <sub>i</sub> = S <sub>s</sub>
	peak unit discharge (w/ concentration factor)	q <sub>o</sub>	cfs	0.009	0.009	0.013	0.010	0.009	0.008	0.013	0.011	
1	peak unit discharge (w/o concentration factor)	q	cfs	0.027	0.027	0.040	0.030	0.027	0.023	0.039	0.032	NRC STP; Section A.2.4.1
1,4	roughness factor (Manning's n)	n	-	0.035	0.035	0.035	0.035	0.035	0.035	0.035	0.035	Chow, 1959; Table 5-6
1	depth of flow	y	ft	0.017	0.018	0.025	0.018	0.017	0.015	0.024	0.022	NRC STP; Section 2.2; Step 6
1	allowable vegetal stress	τ <sub>va</sub>	psf	5.093	5.093	5.093	5.093	5.093	5.093	5.093	5.093	Temple 1987; Eqn 1.17 and Table 3.1
1	percent vegetal coverage	VC	-	33%	33%	33%	33%	33%	33%	33%	33%	39% = min. cover. (Pg. 40 of 2/96 Monitor & STable Plan)
1	reduced allowable vegetal stress	τ <sub>var</sub>	psf	0.382	0.382	0.382	0.382	0.382	0.382	0.382	0.382	SMI, Jan 1996; Comment 6
1	maximum allowable shear stress	τ <sub>max</sub>	psf	0.382	0.382	0.382	0.382	0.382	0.382	0.382	0.382	= MAX(τ <sub>a</sub> , τ <sub>var</sub> )
5	peak intensity	i	in/hr	4.9	4.9	4.9	4.9	4.9	4.9	4.9	4.9	copied from above
6	stable slope	S <sub>s</sub>	ft/ft	0.372	0.372	0.268	0.338	0.372	0.429	0.275	0.320	NRC STP; Section 2.1.1
1	initial slope	S <sub>i</sub>	ft/ft	0.338	0.292	0.200	0.331	0.331	0.327	0.200	0.194	copied from step 1

initial to stable slope comparison

Stable Stable Stable Stable Stable Stable Stable Stable Stable Is S<sub>i</sub> <= S<sub>s</sub> ?

# Design of Soil Covers; Horton/NRC Method (10,000 YR)

# Tailing Impoundment Margins

(NRC STP August 1990; Appendix A)

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Step	Parameter	Symbol	Units	Strip A	Strip B'	Strip C	Strip D	Strip E	Strip F	Strip G	Strip H	Source
1	soil classification			SW	SW	SW	SW	SW	SW	SW	SW	SMI December 1994; Appendix A
1	plasticity index	PI										
1	75% finer diameter (non-cohesive)	D <sub>75</sub>	in	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	SMI December 1994; Appendix A; Section A.3; Figure 3.13
1	soil grain roughness	n <sub>s</sub>	-	0.016	0.016	0.016	0.016	0.016	0.016	0.016	0.016	Temple 1987, Table 3.3
1	basic allowable effective stress	τ <sub>ab</sub>	psf									Temple 1987, Figure 3.3
1	void ratio	e	-	0.37	0.37	0.37	0.37	0.37	0.37	0.37	0.37	Design of Small Dams 1977, Table 8
1	void ratio correction factor	C <sub>e</sub>	-									Temple 1987, Figure 3.4
1	allowable soil stress	τ <sub>a</sub>	psf	0.020	0.020	0.020	0.020	0.020	0.020	0.020	0.020	Temple 1987, Table 3.3
1	retardance curve index	C <sub>1</sub>	-	6.79	6.79	6.79	6.79	6.79	6.79	6.79	6.79	SMI December 1994; Appendix C: Attachment C
1	cover factor	C <sub>F</sub>	-	0.60	0.60	0.60	0.60	0.60	0.60	0.60	0.60	SMI December 1994; Appendix C: Attachment C
1,2	slope length	L <sub>1</sub>	feet	130	130	190	145	130	110	185	155	SMI December 1994; Drawing 3; Rev 2
1	base elevation	E <sub>A</sub>	feet	2084.0	2079.0	2076.0	2084.0	2083.0	2082.0	2076.0	2078.0	SMI December 1994; Drawing 3; Rev 2
1	top elevation	E <sub>B</sub>	feet	2128.0	2117.0	2114.0	2132.0	2126.0	2118.0	2113.0	2108.0	SMI December 1994; Drawing 3; Rev 2
1	elevation difference	H	feet	44.0	38.0	38.0	48.0	43.0	36.0	37.0	30.0	
1,2	initial slope	S <sub>i</sub>	ft/ft	0.338	0.292	0.200	0.331	0.331	0.327	0.200	0.194	
1	runoff coefficient	C	-	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	Chow, 1988; Table 15.1.1
1	peak intensity	i	in/hr	6.75	6.75	6.75	6.75	6.75	6.75	6.75	6.75	=(PMP/10,000 yr total precip.) x (PMP <sub>max</sub> )
1	drainage basin area	A	acres	0.0030	0.0030	0.0044	0.0033	0.0030	0.0025	0.0042	0.0036	
1,3	flow concentration factor	F	-	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	
	peak unit discharge (w/ concentration factor)	q <sub>o</sub>	cfs	0.012	0.012	0.018	0.014	0.012	0.011	0.018	0.015	
1	peak unit discharge (w/o concentration factor)	q	cfs	0.037	0.037	0.055	0.042	0.037	0.032	0.053	0.045	NRC STP; Section A.2.4.1
1,4	roughness factor (Manning's n)	n	-	0.035	0.035	0.035	0.035	0.035	0.035	0.035	0.035	Chow, 1959; Table 5-6
1	depth of flow	y	ft	0.020	0.021	0.030	0.022	0.020	0.019	0.029	0.027	NRC STP; Section 2.2; Step 6
1	allowable vegetal stress	τ <sub>va</sub>	psf	5.093	5.093	5.093	5.093	5.093	5.093	5.093	5.093	Temple 1987; Eqn 1.17 and Table 3.1
1	percent vegetal coverage	VC	-	34%	33%	33%	34%	34%	33%	33%	34%	39% = min. cover. (Pg. 40 of 2/96 Monitor & STable Plan)
1	reduced allowable vegetal stress	τ <sub>var</sub>	psf	0.509	0.382	0.382	0.509	0.509	0.382	0.382	0.509	SMI, Jan 1996; Comment 6
1	maximum allowable shear stress	τ <sub>max</sub>	psf	0.509	0.382	0.382	0.509	0.509	0.382	0.382	0.509	= MAX(τ <sub>a</sub> , τ <sub>var</sub> )
5	peak intensity	i	in/hr	6.75	6.75	6.75	6.75	6.75	6.75	6.75	6.75	copied from above
6	stable slope	S <sub>s</sub>	ft/ft	0.426	0.282	0.204	0.388	0.426	0.326	0.209	0.366	NRC STP; Section 2.1.1
1	initial slope	S <sub>i</sub>	ft/ft	0.338	0.292	0.200	0.331	0.331	0.327	0.200	0.194	copied from step 1

initial to stable slope comparison

Stable Stable Stable Stable Stable Stable Stable Stable Is S<sub>i</sub> <= S<sub>s</sub> ?

# Design of Soil Covers; Horton/NRC Method (100 yr)

# Impoundment Surface

(NRC STP August 1990; Appendix A)

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Step	Parameter	Symbol	Units	Strip A	Strip B	Strip C	Basin 6	Basin 7	Source
1	soil classification			SW	SW	SW	SW	SW	SMI December 1994; Appendix A
1	plasticity index	PI							
1	75% finer diameter (non-cohesive)	D <sub>75</sub>	in	0.070	0.070	0.070	0.070	0.070	SMI December 1994; Appendix G; Section G.A.5
1	soil grain roughness	n <sub>s</sub>	-	0.016	0.016	0.016	0.016	0.016	Temple 1987, Table 3.3
1	basic allowable effective stress	τ <sub>ab</sub>	psf						Temple 1987, Figure 3.3
1	void ratio	e	-	0.37	0.37	0.37	0.37	0.37	Design of Small Dams 1977, Table 8
1	void ratio correction factor	C <sub>e</sub>	-						Temple 1987, Figure 3.4
1	allowable soil stress	τ <sub>a</sub>	psf	0.028	0.028	0.028	0.028	0.028	Temple 1987, Table 3.3
1	retardance curve index	C <sub>i</sub>	-	6.79	6.79	6.79	6.79	6.79	SMI December 1994; Appendix C: Attachment C
1	cover factor	C <sub>F</sub>	-	0.60	0.60	0.60	0.60	0.60	SMI December 1994; Appendix C; Attachment C
1,2	slope length	L <sub>1</sub>	feet	1978	2195	2517	1310	970	SMI December 1994; Drawing 3; Rev 2
1	base elevation	E <sub>A</sub>	feet	2071.0	2069.0	2070.0	2068.0	2075.0	SMI December 1994; Drawing 3; Rev 2
1	top elevation	E <sub>B</sub>	feet	2079.0	2080.0	2076.5	2120.0	2115.0	SMI December 1994; Drawing 3; Rev 2
1	elevation difference	H	feet	8.0	11.0	6.5	52.0	40.0	
1,2	initial slope	S <sub>i</sub>	ft/ft	0.0040	0.0050	0.0026	0.0397	0.0412	
1	runoff coefficient	C	-	0.58	0.58	0.58	0.61	0.61	Chow, 1988; Table 15.1.1, slopes less than 2%
1	peak intensity	i	in/hr	3.43	3.43	3.43	3.43	3.43	=(PMP/10,000 yr total precip.) x (PMP <sub>max</sub> )
1	drainage basin area	A	acres	0.0454	0.0504	0.0578	0.0301	0.0223	
1,3	flow concentration factor	F	-	3.0	3.0	3.0	3.0	3.0	varied so that S <sub>i</sub> = S <sub>s</sub>
	peak unit discharge (w/ concentration factor)	q <sub>o</sub>	cfs	0.090	0.100	0.115	0.063	0.047	
1	peak unit discharge (w/o concentration factor)	q	cfs	0.271	0.301	0.345	0.189	0.140	NRC STP; Section A.2.4.1
1,4	roughness factor (Manning's n)	n	-	0.031	0.031	0.031	0.031	0.031	TRP 12/94, Appendix G, Section G.A-4
1	depth of flow	y	ft	0.234	0.234	0.309	0.095	0.078	NRC STP; Section 2.2; Step 6
1	allowable vegetal stress	τ <sub>va</sub>	psf	5.093	5.093	5.093	5.093	5.093	Temple 1987; Eqn 1.17 and Table 3.1
1	percent vegetal coverage	VC	-	0%	0%	0%	0%	0%	39% = min. cover. (Pg. 40 of 2/96 Monitor & STable Plan)
1	reduced allowable vegetal stress	τ <sub>var</sub>	psf	0.000	0.000	0.000	0.000	0.000	SMI, Jan 1996; Comment 6
1	maximum allowable shear stress	τ <sub>max</sub>	psf	0.028	0.028	0.028	0.028	0.028	= MAX(τ <sub>a</sub> , τ <sub>var</sub> )
5	peak intensity	i	in/hr	3.43	3.43	3.43	3.43	3.43	copied from above
6	stable slope	S <sub>s</sub>	ft/ft	0.001	0.001	0.001	0.002	0.002	NRC STP; Section 2.1.1
1	initial slope	S <sub>i</sub>	ft/ft	0.004	0.005	0.003	0.040	0.041	copied from step 1

initial to stable slope comparison

Stable    Stable    Stable    Unstable    Unstable    Is S<sub>i</sub> <= S<sub>s</sub> ?

# Design of Soil Covers; Horton/NRC Method (1000 YR)

# Impoundment Surface

(NRC STP August 1990; Appendix A)  
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Step	Parameter	Symbol	Units	Strip A	Strip B	Strip C	Basin 6	Basin 7	Source
1	soil classification			SW	SW	SW	SW	SW	SMI December 1994; Appendix A
1	plasticity index	PI							
1	75% finer diameter (non-cohesive)	D <sub>75</sub>	in	0.070	0.070	0.070	0.070	0.070	SMI December 1994; Appendix G; Section G.A.5
1	soil grain roughness	n <sub>s</sub>	-	0.016	0.016	0.016	0.016	0.016	Temple 1987, Table 3.3
1	basic allowable effective stress	τ <sub>ab</sub>	psf						Temple 1987, Figure 3.3
1	void ratio	e	-	0.37	0.37	0.37	0.37	0.37	Design of Small Dams 1977, Table 8
1	void ratio correction factor	C <sub>e</sub>	-						Temple 1987, Figure 3.4
1	allowable soil stress	τ <sub>a</sub>	psf	0.028	0.028	0.028	0.028	0.028	Temple 1987, Table 3.3
1	retardance curve index	C <sub>r</sub>	-	6.79	6.79	6.79	6.79	6.79	SMI December 1994; Appendix C: Attachment C
1	cover factor	C <sub>F</sub>	-	0.60	0.60	0.60	0.60	0.60	SMI December 1994; Appendix C; Attachment C
1,2	slope length	L <sub>1</sub>	feet	1978	2195	2517	1310	970	SMI December 1994; Drawing 3; Rev 2
1	base elevation	E <sub>A</sub>	feet	2071.0	2069.0	2070.0	2068.0	2075.0	SMI December 1994; Drawing 3; Rev 2
1	top elevation	E <sub>B</sub>	feet	2079.0	2080.0	2076.5	2120.0	2115.0	SMI December 1994; Drawing 3; Rev 2
1	elevation difference	H	feet	8.0	11.0	6.5	52.0	40.0	
1,2	initial slope	S <sub>i</sub>	ft/ft	0.0040	0.0050	0.0026	0.0397	0.0412	
1	runoff coefficient	C	-	0.58	0.58	0.58	0.61	0.61	Chow, 1988; Table 15.1.1, slopes less than 2%
1	peak intensity	i	in/hr	4.9	4.9	4.9	4.9	4.9	=(PMP/10,000 yr total precip.) x (PMP <sub>max</sub> )
1	drainage basin area	A	acres	0.0454	0.0504	0.0578	0.0301	0.0223	
1,3	flow concentration factor	F	-	3.0	3.0	3.0	3.0	3.0	varied so that S <sub>i</sub> = S <sub>s</sub>
	peak unit discharge (w/ concentration factor)	q <sub>o</sub>	cfs	0.129	0.143	0.164	0.090	0.067	
1	peak unit discharge (w/o concentration factor)	q	cfs	0.387	0.430	0.493	0.270	0.200	NRC STP; Section A.2.4.1
1,4	roughness factor (Manning's n)	n	-	0.031	0.031	0.031	0.031	0.031	TRP 12/94, Appendix G, Section G.A-4
1	depth of flow	y	ft	0.290	0.289	0.383	0.118	0.097	NRC STP; Section 2.2; Step 6
1	allowable vegetal stress	τ <sub>va</sub>	psf	5.093	5.093	5.093	5.093	5.093	Temple 1987; Eqn 1.17 and Table 3.1
1	percent vegetal coverage	VC	-	0%	0%	0%	32%	32%	39% = min. cover. (Pg. 40 of 2/96 Monitor & STable Plan)
1	reduced allowable vegetal stress	τ <sub>var</sub>	psf	0.000	0.000	0.000	0.255	0.255	SMI, Jan 1996; Comment 6
1	maximum allowable shear stress	τ <sub>max</sub>	psf	0.028	0.028	0.028	0.255	0.255	= MAX(τ <sub>a</sub> , τ <sub>var</sub> )
5	peak intensity	i	in/hr	4.9	4.9	4.9	4.9	4.9	copied from above
6	stable slope	S <sub>s</sub>	ft/ft	0.001	0.001	0.001	0.032	0.042	NRC STP; Section 2.1.1
1	initial slope	S <sub>i</sub>	ft/ft	0.004	0.005	0.003	0.040	0.041	copied from step 1

initial to stable slope comparison

Stable    Stable    Stable    Stable    Stable    Is S<sub>i</sub> <= S<sub>s</sub> ?

# Design of Soil Covers; Horton/NRC Method (10,000-YR)

# Impoundment Surface

(NRC STP August 1990; Appendix A)  
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Step	Parameter	Symbol	Units	Strip A	Strip B	Strip C	Basin 6	Basin 7	Source
1	soil classification			SW	SW	SW	SW	SW	SMI December 1994; Appendix A
1	plasticity index	PI							
1	75% finer diameter (non-cohesive)	D <sub>75</sub>	in	0.070	0.070	0.070	0.070	0.070	SMI December 1994; Appendix G; Section G.A.5
1	soil grain roughness	n <sub>s</sub>	-	0.016	0.016	0.016	0.016	0.016	Temple 1987, Table 3.3
1	basic allowable effective stress	τ <sub>ab</sub>	psf						Temple 1987, Figure 3.3
1	void ratio	e	-	0.37	0.37	0.37	0.37	0.37	Design of Small Dams 1977, Table 8
1	void ratio correction factor	C <sub>e</sub>	-						Temple 1987, Figure 3.4
1	allowable soil stress	τ <sub>a</sub>	psf	0.028	0.028	0.028	0.028	0.028	Temple 1987, Table 3.3
1	retardance curve index	C <sub>t</sub>	-	6.79	6.79	6.79	6.79	6.79	SMI December 1994; Appendix C: Attachment C
1	cover factor	C <sub>F</sub>	-	0.60	0.60	0.60	0.60	0.60	SMI December 1994; Appendix C; Attachment C
1,2	slope length	L <sub>t</sub>	feet	1978	2195	2517	1310	970	SMI December 1994; Drawing 3; Rev 2
1	base elevation	E <sub>A</sub>	feet	2071.0	2069.0	2070.0	2068.0	2075.0	SMI December 1994; Drawing 3; Rev 2
1	top elevation	E <sub>B</sub>	feet	2079.0	2080.0	2076.5	2120.0	2115.0	SMI December 1994; Drawing 3; Rev 2
1	elevation difference	H	feet	8.0	11.0	6.5	52.0	40.0	
1,2	initial slope	S <sub>i</sub>	ft/ft	0.0040	0.0050	0.0026	0.0397	0.0412	
1	runoff coefficient	C	-	0.58	0.58	0.58	0.61	0.61	Chow, 1988; Table 15.1.1, slopes less than 2%
1	peak intensity	i	in/hr	6.75	6.75	6.75	6.75	6.75	=(PMP/10,000 yr total precip.) x (PMP <sub>max</sub> )
1	drainage basin area	A	acres	0.0454	0.0504	0.0578	0.0301	0.0223	
1,3	flow concentration factor	F	-	3.0	3.0	3.0	3.0	3.0	varied so that S <sub>i</sub> = S <sub>s</sub>
	peak unit discharge (w/ concentration factor)	q <sub>o</sub>	cfs	0.178	0.197	0.226	0.124	0.092	
1	peak unit discharge (w/o concentration factor)	q	cfs	0.533	0.592	0.679	0.371	0.275	NRC STP; Section A.2.4.1
1,4	roughness factor (Manning's n)	n	-	0.031	0.031	0.031	0.031	0.031	TRP 12/94, Appendix G, Section G.A-4
1	depth of flow	y	ft	0.351	0.351	0.464	0.143	0.118	NRC STP; Section 2.2; Step 6
1	allowable vegetal stress	τ <sub>va</sub>	psf	5.093	5.093	5.093	5.093	5.093	Temple 1987; Eqn 1.17 and Table 3.1
1	percent vegetal coverage	VC	-	0%	0%	0%	33%	33%	39% = min. cover. (Pg. 40 of 2/96 Monitor & STable Plan)
1	reduced allowable vegetal stress	τ <sub>var</sub>	psf	0.000	0.000	0.000	0.382	0.382	SMI, Jan 1996; Comment 6
1	maximum allowable shear stress	τ <sub>max</sub>	psf	0.028	0.028	0.028	0.382	0.382	= MAX(τ <sub>a</sub> , τ <sub>var</sub> )
5	peak intensity	i	in/hr	6.75	6.75	6.75	6.75	6.75	copied from above
6	stable slope	S <sub>s</sub>	ft/ft	0.001	0.001	0.001	0.044	0.057	NRC STP; Section 2.1.1
1	initial slope	S <sub>i</sub>	ft/ft	0.004	0.005	0.003	0.040	0.041	copied from step 1

initial to stable slope comparison

Stable    Stable    Stable    Stable    Stable    Is S<sub>i</sub> <= S<sub>s</sub> ?

**SHERWOOD PROJECT  
RESPONSES TO WDOH COMMENTS ON THE  
DECEMBER 1994 TAILING RECLAMATION PLAN**

Prepared for

Western Nuclear, Inc.  
Sherwood Mine  
Wellpinit, Washington

Prepared by

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August 1995

***SMI***  

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***Shepherd Miller, Inc.***

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**Q: Question No. 1 from the July 06, 1995 meeting in Seattle - The Washington State Department of Ecology (WDOE) has developed a procedure for estimating precipitation values for extreme rainfall events. The procedure is described in WDOE publication, Dam Safety Guidelines, Technical Note. No. 3, Design Storm Construction. This publication should be used to estimate the 1000-year rainfall event for comparison with the PMP used for design of the diversion channel.**

### **3.7 Values of Extreme Rainfall Events**

Criterion 6 of WAC 246-252-030 requires that the reclamation design be effective for 1000 years. To meet this criterion, the diversion channel was designed to safely convey runoff from the Probable Maximum Precipitation (PMP) event, which is 11.5 inches of precipitation in a 6-hour period. For comparison with this rainfall event, precipitation estimates were made using the criteria from the WDOE publication. These are shown in Table 3.9. Flood discharges for recurrence intervals of 1000 and 10,000 years were then estimated using the Corps of Engineers' HEC-1 computer model and HEC-2 was used to estimate flow depths and velocities for the two flood events. (The HEC-1 and HEC-2 models are discussed in detail in Appendix D of the 12/94 TRP). The results of this analysis, are summarized in Tables 3.10 and 3.11. Table 3.10 shows that riprap on the side slopes of the diversion channel is from 4.4 feet to 11.0 feet higher in elevation within the channel than required for the 10,000-year flood. For the 1000-year flood, the riprap is from 5.0 feet to 13.4 feet higher than required. Table 3.11 shows that the maximum design flood discharge is at least an order of magnitude larger for the PMF than it is for the 1000-year flood and almost 6 times larger than the 10,000-year flood.



TABLE 3.9

Projections of Design Precipitation for Eastern Washington, Region 1 (Sherwood site)

Annual exceedance probability, AEP	Return period	Frequency factor, Ki	Precipitation estimate for selected AEP (inches)	Design precipitation (inches)
1/2	2 yr	-0.187	0.704	0.81
1/10	10 yr	1.23	1.056	1.21
1/25	25 yr	2.04	1.257	1.45
1/100	100 yr	3.39	1.593	1.83
step 1	500 yr	5.21	2.045	2.35
step 2	1000 yr	6.1	2.266	2.61
step 3	step 3	7.72	2.668	3.07
step 4	10,000 yr	9.55	3.123	3.59
step 5	step 5	11.62	3.637	4.18
step 6	100,000 yr	13.96	4.218	4.85
step 7	step 7	16.61	4.876	5.61
step 8	1,000,000 yr	19.6	5.619	6.46

Mean annual ppt: 20 inches  
duration of interest: 6 hours

X2p(fig3) = 0.8 inches  
Cv(fig5a) = 0.331  
T3(fig6a) = 0.21  
K2(tabB2) = -0.187  
Xbar(eqn2) = 0.750

Source of Information:

Washington State Dept. of Ecology. Dam safety Guidelines.

Technical Note 3: Design Storm Construction. April 1993.

TABLE 3.10

COMPARISON OF DISCHARGES, WATER DEPTHS, AND FLOW VELOCITIES FOR THE PMF, 10,000 YEAR FLOOD AND THE 1000 YEAR FLOOD.

7/18/95 Revised 8/17/95

STATIONS	ELEVATION TOP OF RIPRAP 1)	PROBABLE MAXIMUM FLOOD			10,000 YEAR FLOOD 3)			1000 YEAR FLOOD 3)		
		DISCHARGE (CFS)	WATER LEVEL ELEVATION 2) (FT)	DIFFERENCE BETWEEN TOP OF RIPRAP AND PMF WATER ELEVATION (FT)	DISCHARGE (CFS)	WATER LEVEL ELEVATION 2) (FT)	DIFFERENCE BETWEEN TOP OF RIPRAP AND WATER ELEVATION (FT)	DISCHARGE (CFS)	WATER LEVEL ELEVATION 2) (FT)	DIFFERENCE BETWEEN TOP OF RIPRAP AND WATER ELEVATION (FT)
88 + 00	2133.50	357	2132.40	1.10	62	2129.94	3.56	32	2129.47	4.03
86 + 00	2133.00	357	2131.65	1.35	62	2128.87	4.13	32	2128.33	4.67
84 + 00	2131.50	357	2130.38	1.12	62	2127.49	4.01	32	2126.91	4.59
82 + 00	2130.50	428	2129.04	1.46	76	2126.10	4.40	40	2125.51	4.99
80 + 00	2129.00	428	2127.65	1.35	76	2124.60	4.40	40	2124.02	4.98
78 + 00	2127.50	428	2126.50	1.00	76	2123.11	4.39	40	2122.52	4.98
76 + 00	2127.00	428	2125.77	1.23	76	2121.61	5.39	40	2121.02	5.98
74 + 00	2126.50	428	2125.39	1.11	76	2120.15	6.35	40	2119.52	6.98
72 + 00	2126.50	428	2125.22	1.28	76	2119.15	7.35	40	2118.11	8.39
70 + 00	2126.50	428	2125.13	1.37	76	2118.86	7.64	40	2117.61	8.89
68 + 00	2126.00	1557	2124.53	1.47	272	2117.96	8.04	141	2116.56	9.44
66 + 00	2125.50	1557	2124.13	1.37	272	2117.27	8.23	141	2115.73	9.77
64 + 00	2124.50	3247	2123.15	1.35	559	2116.20	8.30	290	2114.64	9.86
62 + 00	2123.50	3247	2121.99	1.51	559	2114.70	8.80	290	2113.14	10.36
60 + 00	2122.00	3247	2120.90	1.10	559	2113.24	8.76	290	2111.64	10.36
58 + 00	2121.50	3247	2120.02	1.48	559	2111.85	9.65	290	2110.16	11.34
56 + 00	2120.50	3247	2119.36	1.14	559	2110.69	9.81	290	2108.77	11.73
54 + 00	2120.00	3247	2118.94	1.06	559	2109.95	10.05	290	2107.81	12.19
52 + 00	2120.00	3247	2118.61	1.39	559	2109.51	10.49	290	2107.31	12.69
50 + 00	2119.50	3725	2118.26	1.24	641	2109.11	10.39	332	2106.90	12.60
48 + 00	2119.00	3725	2117.90	1.10	641	2108.70	10.30	332	2106.47	12.53
46 + 00	2119.00	3725	2117.54	1.46	641	2108.29	10.71	332	2106.30	12.70
44 + 00	2118.50	4333	2117.20	1.30	746	2107.92	10.58	386	2105.64	12.86
42 + 00	2118.00	4333	2116.76	1.24	746	2107.47	10.53	386	2105.20	12.80
40 + 00	2117.50	4333	2116.37	1.13	746	2107.02	10.48	386	2104.74	12.76
38 + 00	2117.00	4333	2115.93	1.07	746	2106.55	10.45	386	2104.25	12.75
36 + 00	2117.00	4333	2115.51	1.49	746	2106.09	10.91	386	2103.77	13.23
34 + 00	2116.50	4333	2115.10	1.40	746	2105.64	10.86	386	2103.30	13.20
32 + 00	2116.00	4333	2114.71	1.29	746	2105.20	10.80	386	2102.84	13.16
30 + 00	2115.50	4333	2114.32	1.18	746	2104.78	10.72	386	2102.40	13.10
28 + 00	2115.00	4333	2113.95	1.05	746	2104.38	10.62	386	2101.97	13.03
26 + 00	2115.00	4333	2113.59	1.41	746	2104.00	11.00	386	2101.58	13.42
24 + 00	2114.50	4333	2113.24	1.26	746	2103.63	10.87	386	2101.22	13.28
22 + 00	2114.00	5297	2112.89	1.11	907	2103.19	10.81	470	2100.70	13.30
20 + 00	2113.50	5297	2112.34	1.16	907	2102.72	10.78	470	2100.32	13.18
18 + 00	2113.00	5326	2111.86	1.14	913	2102.22	10.78	473	2099.82	13.18
16 + 00	2112.50	5326	2111.35	1.15	913	2101.72	10.78	473	2099.32	13.18
14 + 00	2112.00	5326	2110.85	1.15	913	2101.22	10.78	473	2098.82	13.18
12 + 00	2111.50	5326	2110.35	1.15	913	2100.73	10.77	473	2098.33	13.17
10 + 00	2111.00	5326	2109.85	1.15	913	2100.23	10.77	473	2097.83	13.17
8 + 00	2110.50	5325	2109.35	1.15	912	2099.73	10.77	473	2097.34	13.16
6 + 00	2110.00	5325	2108.83	1.17	912	2099.21	10.79	473	2096.81	13.19
4 + 00	2109.50	5325	2108.33	1.17	912	2098.71	10.79	473	2096.31	13.19
2 + 00	2109.00	5325	2107.83	1.17	912	2098.21	10.79	473	2095.81	13.19
0 + 00	2108.50	5325	2107.32	1.18	912	2097.71	10.79	473	2095.31	13.19

- 1) TOP OF RIPRAP ELEVATIONS WERE DETERMINED BY ADDING SUPERELEVATION AND 1 FOOT OF FREEBOARD TO THE HEC-2 ANALYSIS THAT INCLUDED TREES GROWING IN CHANNEL ON 1.5 FEET OF SEDIMENT. THE VALUES WERE THEN ROUNDED UP TO THE NEAREST 0.5 FOOT. VALUES OF SUPERELEVATION ARE SHOWN IN TABLE 3.3.
- 2) WATER LEVEL ELEVATIONS ARE FROM THE HEC-2 ANALYSIS PRESENTED IN ATTACHMENT 2.
- 3) DISCHARGES WERE DETERMINED USING PRECIPITATION AMOUNTS ESTIMATED USING WDOE'S TECHNICAL NOTE 3, AND HEC-1.

**TABLE 3.11                      COMPARISON OF FLOOD DISCHARGES FOR  
THE 1000-YEAR, 10,000-YEAR AND PMP**

RECURRENCE INTERVAL	6-HOUR PRECIPITATION	MAXIMUM FLOOD DISCHARGE
1000-YEAR	2.61 INCHES	473 CFS
10,000-YEAR	3.59 INCHES	912 CFS
PMP	11.5 INCHES	5325 CFS

**Q:     Question No. 2 from the July 06, 1995 meeting in Seattle - Is it feasible to construct sediment traps (pits) at each tributary to intercept and hold sediment before it enters the diversion channel?**

### **3.8 Construction of Sediment Traps**

The design of the diversion channel presented in the 12/94 TRP will be modified based on the concerns of the State of Washington. The new design will take into account sediment deposition, in addition to vegetation growing in the channel, superelevation, and freeboard. The sediment deposition analysis presented in section 1.3 of this response report indicates that very little sediment will be deposited in the channel. The maximum depth of deposited sediment will be 1.5 feet and this will occur during the PMF. Since the channel design is being revised to accommodate sediment deposition, it is concluded that sediment traps are not necessary to augment the additional conservatism inherent to the revised diversion channel design.

**Q:     Question No. 3 from the July 06, 1995 meeting in Seattle - Does HEC-6 consider the effect of the tributaries, which are much steeper than the diversion channel, dropping their sediment loads as they enter the much flatter diversion channel and forming deltas?**

### **3.9 Deltas at Intersections of Tributaries and Diversion Channel**

The design of the tributary confluences, discussed in Appendix E to the 12/94 TRP, minimizes the potential for the formation of deltas by 1) widening the confluences to reduce flow velocities before flood flows enter the diversion channel, 2) regrading each confluence such that the base of the confluence and bed of the diversion

**HEC-1 ANALYSES FOR 100-YR, 1,000-YR, AND 10,000-YR EVENTS**

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   MAY 1991                       *
*   VERSION 4.0.1E                 *
*   Lahey F77L-EM/32 version 5.01  *
*   Dodson & Associates, Inc.      *
*   RUN DATE 12/08/99 TIME 14:49:38 *
*****
    
```

```

*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET             *
* DAVIS, CALIFORNIA 95616      *
* (916) 551-1748               *
*****
    
```

```

X X XXXXXXXX XXXXX X
X X X X X X X X
X X X X X X X
XXXXXXXXXXXX X XXXXX X
X X X X X X X
X X X X X X X
X X XXXXXXXX XXXXX XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW. THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Input File Name: P:\317\TASK12\HEC1\EAST_100.IH1
2 ID DATE:12-08-99
3 ID HEC-1 100 YEAR RUNOFF STUDY, SHERWOOD TAILINGS, WESTERN WASHINGTON
4 ID EAST DIVERSION CHANNEL MODIFIED TO REDUCE RIPRAP SIZE
5 ID CURVE NUMBER = 78
6 ID DESIGNED FOR 100 YEAR PRECIPITATION, 2.19 in. IN 6 HOURS WITH SCS TYPE II DI
7 ID ANTECEDENT MOISTURE CONDITION III
*DIAGRAM
8 IT 1 0 0000 540 0 0
9 IN 3 0 0
10 IO 5 0
*
11 KK B1 RUNOFF FROM BASIN 1
12 KO 21
13 PB 2.19
14 PC .0000 0.0024 0.0042 0.0062 0.0086 0.0110 0.0126 0.0148 0.0172 0.0198
15 PC .0230 0.0254 0.0278 0.0302 0.0326 0.0350 0.0374 0.0404 0.0432 0.0458
16 PC .0490 0.0514 0.0544 0.0572 0.0598 0.0630 0.0662 0.0694 0.0726 0.0760
17 PC .0800 0.0840 0.0880 0.0920 0.0960 0.1000 0.1048 0.1102 0.1158 0.1214
18 PC .1270 0.1326 0.1388 0.1452 0.1516 0.1580 0.1652 0.1730 0.1822 0.1928
19 PC .2040 0.2192 0.2374 0.2622 0.2998 0.3590 0.5174 0.6230 0.6802 0.7050
20 PC .7250 0.7450 0.7608 0.7736 0.7848 0.7960 0.8032 0.8104 0.8176 0.8246
21 PC .8310 0.8374 0.8438 0.8498 0.8554 0.8610 0.8658 0.8706 0.8750 0.8790
22 PC .8830 0.8870 0.8910 0.8950 0.8990 0.9030 0.9070 0.9104 0.9140 0.9178
23 PC .9210 0.9250 0.9284 0.9316 0.9348 0.9380 0.9412 0.9438 0.9466 0.9496
24 PC .9520 0.9552 0.9578 0.9606 0.9636 0.9660 0.9684 0.9708 0.9732 0.9756
25 PC .9780 0.9804 0.9828 0.9852 0.9874 0.9890 0.9914 0.9938 0.9958 0.9976
26 PC 1.000
27 BA 0.050
28 LS 0 78
29 UD .288
    
```

```

*
30  KK  R-AB ROUTE REACH AB
31  KO                               21
32  RM   4 .0586 .399
*
33  KK  B2 RUNOFF FROM BASIN 2
34  KO                               21
35  BA   .023
36  LS   0 78
37  UD  0.097
*
38  KK  NB COMBINE ROUTED REACH AB WITH BASIN B2 AT NODE B
39  KO                               21
40  HC   2
*
41  KK  R-BC ROUTE REACH BC
42  KO                               21
43  RM   1 .018 .387
    HEC-1 INPUT

```

1

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

44  KK  B3 RUNOFF FROM BASIN 3
45  KO                               21
46  BA  .162
47  LS   0 78
48  UD  .297
*
49  KK  NC COMBINE ROUTED REACH BC WITH BASIN B3 AT NODE C
50  KO                               21
51  HC   2
*
52  KK  R-CD ROUTE REACH CD
53  KO                               21
54  RM   2 .0348 .431
*
55  KK  B4 RUNOFF FROM BASIN 4
56  KO                               21
57  BA  .354
58  LS   0 78
59  UD  0.467
*
60  KK  ND COMBINE ROUTED REACH CD WITH BASIN B4 AT NODE D
61  KO                               21
62  HC   2
*
63  KK  R-DE ROUTED REACH DE
64  KO                               21
65  RM   2 .0343 .411
*
66  KK  B5 RUNOFF FROM BASIN 5
67  KO                               21
68  BA  0.086
69  LS   0 78
70  UD  0.266
*
71  KK  NE COMBINE ROUTED REACH DE WITH BASIN B5 AT NODE E
72  KO                               21
73  HC   2
*
74  KK  R-EF ROUTED REACH EF

```

75 KO 21  
 76 RM 4 .0658 .416  
 \*

1

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

77 KK B6 RUNOFF FROM BASIN 6  
 78 KO 21  
 79 BA 0.111  
 80 LS 0 78  
 81 UD 0.311  
 \*

82 KK NF COMBINE ROUTED REACH EF WITH BASIN B6 AT NODE F  
 83 KO 21  
 84 HC 2  
 \*

85 KK R-FG ROUTE REACH FG  
 86 KO 21  
 87 RM 1 .0085 .416  
 \*

88 KK B7 RUNOFF FROM BASIN 7  
 89 KO 21  
 90 BA 0.278  
 91 LS 0 78  
 92 UD 0.647  
 \*

93 KK NG COMBINE ROUTED REACH FG WITH BASIN B7 AT NODE G  
 94 KO 21  
 95 HC 2  
 \*

96 KK R-GH ROUTE REACH GH  
 97 KO 21  
 98 RM 2 .0371 .417  
 \*

99 KK B8 RUNOFF FROM BASIN 8  
 100 KO 21  
 101 BA .020  
 102 LS 0 78  
 103 UD 0.112  
 \*

104 KK NH COMBINE ROUTED REACH GH WITH BASIN B8 AT NODE H  
 105 KO 21  
 106 HC 2  
 \*

107 KK R-HI ROUTE REACH HI  
 108 KO 21  
 109 RM 2 .030 .417  
 110 ZZ

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT

LINE (V) ROUTING (→) DIVERSION OR PUMP FLOW

NO. (.) CONNECTOR (←) RETURN OF DIVERTED OR PUMPED FLOW

11 B1  
 V  
 V  
 30 R-AB

```

33 . . . B2
. . .
38 NB.....
V
V
41 R-BC
.
.
44 . . . B3
. . .
49 NC .....
V
V
52 R-CD
.
.
55 . . . B4
. . .
60 ND.....
V
V
63 R-DE
.
.
66 . . . B5
. . .
71 NE.....
V
V
74 R-EF
.
.
77 . . . B6
. . .
82 NF.....
V
V
85 R-FG
.
.
88 . . . B7
. . .
93 NG.....
V
V
96 R-GH
.
.
99 . . . B8
. . .
104 NH.....
V
V
107 R-HI

```

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*                               *
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
ENGINEERS *
*   MAY 1991   *
*   VERSION 4.0.1E   *

```

```

*****
*                               *
* U.S. ARMY CORPS OF
HYDROLOGIC ENGINEERING CENTER *
*   609 SECOND STREET   *

```



\* Lahey F77L-EM/32 version 5.01 \*  
\* Dodson & Associates, Inc. \*  
\* RUN DATE 12/08/99 TIME 14:49:38 \*  
\*\*\*\*\*

\* DAVIS, CALIFORNIA 95616 \*  
\* (916) 551-1748 \*  
\*\*\*\*\*

Input File Name: P:\317\TASK12\HEC1\EAST\_100.IH1  
DATE:12-08-99  
HEC-1 100 YEAR RUNOFF STUDY, SHERWOOD TAILINGS, WESTERN WASHINGTON  
EAST DIVERSION CHANNEL MODIFIED TO REDUCE RIPRAP SIZE  
CURVE NUMBER = 78  
DESIGNED FOR 100 YEAR PRECIPITATION, 2.19 in. IN 6 HOURS WITH SCS TYPE II DI  
ANTECEDENT MOISTURE CONDITION III

10 IO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA  
NMIN 1 MINUTES IN COMPUTATION INTERVAL  
IDATE 1 0 STARTING DATE  
ITIME 0000 STARTING TIME  
NQ 540 NUMBER OF HYDROGRAPH ORDINATES  
NDDATE 1. 0 ENDING DATE  
NDTIME 0859 ENDING TIME  
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.02 HOURS  
TOTAL TIME BASE 8.98 HOURS

ENGLISH UNITS  
DRAINAGE AREA SQUARE MILES  
PRECIPITATION DEPTH INCHES  
LENGTH, ELEVATION FEET  
FLOW CUBIC FEET PER SECOND  
STORAGE VOLUME ACRE-FEET  
SURFACE AREA ACRES  
TEMPERATURE DEGREES FAHRENHEIT

\*\*\*\*\*

\*\*\*\*\*  
\* \*  
11 KK \* B1 \* RUNOFF FROM BASIN 1  
\* \*  
\*\*\*\*\*

12 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

```

*****
*      *
30 KK * R-AB * ROUTE REACH AB
*      *
*****

```

```

31 KO OUTPUT CONTROL VARIABLES
    IPRNT 5 PRINT CONTROL
    IPLOT 0 PLOT CONTROL
    QSCAL 0. HYDROGRAPH PLOT SCALE
    IPNCH 0 PUNCH COMPUTED HYDROGRAPH
    IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
    ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
    ISAV2 540 LAST ORDINATE PUNCHED OR SAVED
    TIMINT 0.017 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*      *
33 KK * B2 * RUNOFF FROM BASIN 2
*      *
*****

```

```

34 KO OUTPUT CONTROL VARIABLES
    IPRNT 5 PRINT CONTROL
    IPLOT 0 PLOT CONTROL
    QSCAL 0. HYDROGRAPH PLOT SCALE
    IPNCH 0 PUNCH COMPUTED HYDROGRAPH
    IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
    ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
    ISAV2 540 LAST ORDINATE PUNCHED OR SAVED
    TIMINT 0.017 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*      *
38 KK * NB * COMBINE ROUTED REACH AB WITH BASIN B2 AT NODE B
*      *
*****

```

```

39 KO OUTPUT CONTROL VARIABLES
    IPRNT 5 PRINT CONTROL
    IPLOT 0 PLOT CONTROL
    QSCAL 0. HYDROGRAPH PLOT SCALE
    IPNCH 0 PUNCH COMPUTED HYDROGRAPH
    IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
    ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
    ISAV2 540 LAST ORDINATE PUNCHED OR SAVED
    TIMINT 0.017 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*      *
41 KK * R-BC * ROUTE REACH BC
*      *

```

```

*****
42 KO  OUTPUT CONTROL VARIABLES
      IPRNT    5 PRINT CONTROL
      IPLOT    0 PLOT CONTROL
      QSCAL    0. HYDROGRAPH PLOT SCALE
      IPNCH    0 PUNCH COMPUTED HYDROGRAPH
      IOUT     21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2    540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT   0.017 TIME INTERVAL IN HOURS

```

\*\*\*\*\*

```

*****
*      *
44 KK  *  B3  *  RUNOFF FROM BASIN 3
*      *
*****

```

```

45 KO  OUTPUT CONTROL VARIABLES
      IPRNT    5 PRINT CONTROL
      IPLOT    0 PLOT CONTROL
      QSCAL    0. HYDROGRAPH PLOT SCALE
      IPNCH    0 PUNCH COMPUTED HYDROGRAPH
      IOUT     21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2    540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT   0.017 TIME INTERVAL IN HOURS

```

\*\*\*\*\*

```

*****
*      *
49 KK  *  NC  *  COMBINE ROUTED REACH BC WITH BASIN B3 AT NODE C
*      *
*****

```

```

50 KO  OUTPUT CONTROL VARIABLES
      IPRNT    5 PRINT CONTROL
      IPLOT    0 PLOT CONTROL
      QSCAL    0. HYDROGRAPH PLOT SCALE
      IPNCH    0 PUNCH COMPUTED HYDROGRAPH
      IOUT     21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2    540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT   0.017 TIME INTERVAL IN HOURS

```

\*\*\*\*\*

```

*****
*      *
52 KK  *  R-CD *  ROUTE REACH CD
*      *
*****

```

```

53 KO  OUTPUT CONTROL VARIABLES
      IPRNT    5 PRINT CONTROL

```

IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
 \* \*  
 55 KK \* B4 \* RUNOFF FROM BASIN 4  
 \* \*  
 \*\*\*\*\*

56 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
 \* \*  
 60 KK \* ND \* COMBINE ROUTED REACH CD WITH BASIN B4 AT NODE D  
 \* \*  
 \*\*\*\*\*

61 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
 \* \*  
 63 KK \* R-DE \* ROUTED REACH DE  
 \* \*  
 \*\*\*\*\*

64 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT

ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
 \* \*  
 66 KK \* B5 \* RUNOFF FROM BASIN 5  
 \* \*  
 \*\*\*\*\*

67 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
 \* \*  
 71 KK \* NE \* COMBINE ROUTED REACH DE WITH BASIN B5 AT NODE E  
 \* \*  
 \*\*\*\*\*

72 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
 \* \*  
 74 KK \* R-EF \* ROUTED REACH EF  
 \* \*  
 \*\*\*\*\*

75 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

```
*****
*      *
77 KK *  B6 *  RUNOFF FROM BASIN 6
*      *
*****
```

```
78 KO  OUTPUT CONTROL VARIABLES
      IPRNT  5 PRINT CONTROL
      IPLOT  0 PLOT CONTROL
      QSCAL  0. HYDROGRAPH PLOT SCALE
      IPNCH  0 PUNCH COMPUTED HYDROGRAPH
      IOUT   21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1  1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2  540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT 0.017 TIME INTERVAL IN HOURS
```

\*\*\*\*\*

```
*****
*      *
82 KK *  NF *  COMBINE ROUTED REACH EF WITH BASIN B6 AT NODE F
*      *
*****
```

```
83 KO  OUTPUT CONTROL VARIABLES
      IPRNT  5 PRINT CONTROL
      IPLOT  0 PLOT CONTROL
      QSCAL  0. HYDROGRAPH PLOT SCALE
      IPNCH  0 PUNCH COMPUTED HYDROGRAPH
      IOUT   21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1  1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2  540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT 0.017 TIME INTERVAL IN HOURS
```

\*\*\*\*\*

```
*****
*      *
85 KK *  R-FG *  ROUTE REACH FG
*      *
*****
```

```
86 KO  OUTPUT CONTROL VARIABLES
      IPRNT  5 PRINT CONTROL
      IPLOT  0 PLOT CONTROL
      QSCAL  0. HYDROGRAPH PLOT SCALE
      IPNCH  0 PUNCH COMPUTED HYDROGRAPH
      IOUT   21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1  1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2  540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT 0.017 TIME INTERVAL IN HOURS
```

\*\*\*\*\* WARNING \*\*\*\*\* POSSIBLE INSTABILITIES IN THE MUSKINGUM ROUTING FOR REACH R-FG.  
 REDUCE NSTPS OR DECREASE YOUR COMPUTATION INTERVAL (FIRST FIELD OF THE IT RECORD).

\*\*\*\*\*

```

*****
*
88 KK * B7 * RUNOFF FROM BASIN 7
*
*****
    
```

```

89 KO OUTPUT CONTROL VARIABLES
      IPRNT 5 PRINT CONTROL
      IPLOT 0 PLOT CONTROL
      QSCAL 0. HYDROGRAPH PLOT SCALE
      IPNCH 0 PUNCH COMPUTED HYDROGRAPH
      IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2 540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT 0.017 TIME INTERVAL IN HOURS
    
```

\*\*\*\*\*

```

*****
*
93 KK * NG * COMBINE ROUTED REACH FG WITH BASIN B7 AT NODE G
*
*****
    
```

```

94 KO OUTPUT CONTROL VARIABLES
      IPRNT 5 PRINT CONTROL
      IPLOT 0 PLOT CONTROL
      QSCAL 0. HYDROGRAPH PLOT SCALE
      IPNCH 0 PUNCH COMPUTED HYDROGRAPH
      IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2 540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT 0.017 TIME INTERVAL IN HOURS
    
```

\*\*\*\*\*

```

*****
*
96 KK * R-GH * ROUTE REACH GH
*
*****
    
```

```

97 KO OUTPUT CONTROL VARIABLES
      IPRNT 5 PRINT CONTROL
      IPLOT 0 PLOT CONTROL
      QSCAL 0. HYDROGRAPH PLOT SCALE
      IPNCH 0 PUNCH COMPUTED HYDROGRAPH
      IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2 540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT 0.017 TIME INTERVAL IN HOURS
    
```

\*\*\*\*\*

```

*****
*           *
99 KK *   B8 *   RUNOFF FROM BASIN 8
*           *
*****
    
```

```

100 KO   OUTPUT CONTROL VARIABLES
        IPRNT   5 PRINT CONTROL
        IPLOT   0 PLOT CONTROL
        QSCAL   0. HYDROGRAPH PLOT SCALE
        IPNCH   0 PUNCH COMPUTED HYDROGRAPH
        IOUT    21 SAVE HYDROGRAPH ON THIS UNIT
        ISAV1   1 FIRST ORDINATE PUNCHED OR SAVED
        ISAV2   540 LAST ORDINATE PUNCHED OR SAVED
        TIMINT  0.017 TIME INTERVAL IN HOURS
    
```

\*\*\*\*\*

```

*****
*           *
104 KK *   NH *   COMBINE ROUTED REACH GH WITH BASIN B8 AT NODE H
*           *
*****
    
```

```

105 KO   OUTPUT CONTROL VARIABLES
        IPRNT   5 PRINT CONTROL
        IPLOT   0 PLOT CONTROL
        QSCAL   0. HYDROGRAPH PLOT SCALE
        IPNCH   0 PUNCH COMPUTED HYDROGRAPH
        IOUT    21 SAVE HYDROGRAPH ON THIS UNIT
        ISAV1   1 FIRST ORDINATE PUNCHED OR SAVED
        ISAV2   540 LAST ORDINATE PUNCHED OR SAVED
        TIMINT  0.017 TIME INTERVAL IN HOURS
    
```

\*\*\*\*\*

```

*****
*           *
107 KK *   R-HI *  ROUTE REACH HI
*           *
*****
    
```

```

108 KO   OUTPUT CONTROL VARIABLES
        IPRNT   5 PRINT CONTROL
        IPLOT   0 PLOT CONTROL
        QSCAL   0. HYDROGRAPH PLOT SCALE
        IPNCH   0 PUNCH COMPUTED HYDROGRAPH
        IOUT    21 SAVE HYDROGRAPH ON THIS UNIT
        ISAV1   1 FIRST ORDINATE PUNCHED OR SAVED
        ISAV2   540 LAST ORDINATE PUNCHED OR SAVED
        TIMINT  0.017 TIME INTERVAL IN HOURS
    
```

1

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

TIME OF OPERATION +	PEAK STATION	TIME OF FLOW	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN STAGE	MAXIMUM MAX STAGE
			PEAK 6-HOUR	24-HOUR	72-HOUR		



+	HYDROGRAPH AT B1	21. 3.15	3.	2.	2.	0.05
+	ROUTED TO R-AB	21. 3.22	3.	2.	2.	0.05
+	HYDROGRAPH AT B2	18. 2.93	1.	1.	1.	0.02
+	2 COMBINED AT NB	26. 3.18	5.	3.	3.	0.07
+	ROUTED TO R-BC	26. 3.20	5.	3.	3.	0.07
+	HYDROGRAPH AT B3	67. 3.17	10.	7.	7.	0.16
+	2 COMBINED AT NC	93. 3.17	15.	10.	10.	0.23
+	ROUTED TO R-CD	93. 3.22	15.	10.	10.	0.23
+	HYDROGRAPH AT B4	108. 3.37	23.	15.	15.	0.35
+	2 COMBINED AT ND	191. 3.28	38.	25.	25.	0.59
+	ROUTED TO R-DE	191. 3.32	38.	25.	25.	0.59
+	HYDROGRAPH AT B5	38. 3.13	5.	4.	4.	0.09
+	2 COMBINED AT NE	219. 3.28	43.	29.	29.	0.68
+	ROUTED TO R-EF	218. 3.35	43.	29.	29.	0.68
+	HYDROGRAPH AT B6	44. 3.18	7.	5.	5.	0.11
+	2 COMBINED AT NF	254. 3.33	50.	34.	34.	0.79
+	ROUTED TO R-FG	254. 3.33	50.	34.	34.	0.79
+	HYDROGRAPH AT B7	67. 3.58	18.	12.	12.	0.28
+	2 COMBINED AT NG	309. 3.37	68.	45.	45.	1.06
+	ROUTED TO R-GH	309. 3.40	68.	45.	45.	1.06
+	HYDROGRAPH AT B8	15. 2.95	1.	1.	1.	0.02
+	2 COMBINED AT NH	312. 3.40	69.	46.	46.	1.08
+	ROUTED TO R-HI	312. 3.43	69.	46.	46.	1.08

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   MAY 1991 *
*   VERSION 4.0.1E *
*   Lahey F77L-EM/32 version 5.01 *
*   Dodson & Associates, Inc. *
* RUN DATE 12/08/99 TIME 14:52:11 *
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 551-1748 *
*****

```

```

X X XXXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXXXXXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW. THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Input File Name: P:\317\TASK12\HEC1\EAST1000.IH1
2 ID DATE:12-08-99
3 ID HEC-1 1000 YEAR RUNOFF STUDY, SHERWOOD TAILINGS, WESTERN WASHINGTON
4 ID EAST DIVERSION CHANNEL MODIFIED TO REDUCE RIPRAP SIZE
5 ID CURVE NUMBER = 78
6 ID DESIGNED FOR 1000 YEAR PRECIPITATION, 3.13 in. IN 6 HOURS WITH SCS TYPE II DI
7 ID ANTECEDENT MOISTURE CONDITION III
*DIAGRAM
8 IT 1 0 0000 540 0 0
9 IN 3 0 0
10 IO 5 0
*
11 KK B1 RUNOFF FROM BASIN 1
12 KO 21
13 PB 3.13
14 PC .0000 0.0024 0.0042 0.0062 0.0086 0.0110 0.0126 0.0148 0.0172 0.0198
15 PC .0230 0.0254 0.0278 0.0302 0.0326 0.0350 0.0374 0.0404 0.0432 0.0458
16 PC .0490 0.0514 0.0544 0.0572 0.0598 0.0630 0.0662 0.0694 0.0726 0.0760
17 PC .0800 0.0840 0.0880 0.0920 0.0960 0.1000 0.1048 0.1102 0.1158 0.1214
18 PC .1270 0.1326 0.1388 0.1452 0.1516 0.1580 0.1652 0.1730 0.1822 0.1928
19 PC .2040 0.2192 0.2374 0.2622 0.2998 0.3590 0.5174 0.6230 0.6802 0.7050
20 PC .7250 0.7450 0.7608 0.7736 0.7848 0.7960 0.8032 0.8104 0.8176 0.8246
21 PC .8310 0.8374 0.8438 0.8498 0.8554 0.8610 0.8658 0.8706 0.8750 0.8790
22 PC .8830 0.8870 0.8910 0.8950 0.8990 0.9030 0.9070 0.9104 0.9140 0.9178
23 PC .9210 0.9250 0.9284 0.9316 0.9348 0.9380 0.9412 0.9438 0.9466 0.9496
24 PC .9520 0.9552 0.9578 0.9606 0.9636 0.9660 0.9684 0.9708 0.9732 0.9756
25 PC .9780 0.9804 0.9828 0.9852 0.9874 0.9890 0.9914 0.9938 0.9958 0.9976
26 PC 1.000
27 BA 0.050
28 LS 0 78
29 UD .288
*

```

30 KK R-AB ROUTE REACH AB  
 31 KO 21  
 32 RM 4 .0586 .399  
 \*  
 33 KK B2 RUNOFF FROM BASIN 2  
 34 KO 21  
 35 BA .023  
 36 LS 0 78  
 37 UD 0.097  
 38 KK NB COMBINE ROUTED REACH AB WITH BASIN B2 AT NODE B  
 39 KO 21  
 40 HC 2  
 41 KK R-BC ROUTE REACH BC  
 42 KO 21  
 43 RM 1 .018 .387  
 HEC-1 INPUT

PAGE 2

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

44 KK B3 RUNOFF FROM BASIN 3  
 45 KO 21  
 46 BA .162  
 47 LS 0 78  
 48 UD .297  
 49 KK NC COMBINE ROUTED REACH BC WITH BASIN B3 AT NODE C  
 50 KO 21  
 51 HC 2  
 52 KK R-CD ROUTE REACH CD  
 53 KO 21  
 54 RM 2 .0348 .431  
 \*  
 55 KK B4 RUNOFF FROM BASIN 4  
 56 KO 21  
 57 BA .354  
 58 LS 0 78  
 59 UD 0.467  
 \*  
 60 KK ND COMBINE ROUTED REACH CD WITH BASIN B4 AT NODE D  
 61 KO 21  
 62 HC 2  
 \*  
 63 KK R-DE ROUTED REACH DE  
 64 KO 21  
 65 RM 2 .0343 .411  
 \*  
 66 KK B5 RUNOFF FROM BASIN 5  
 67 KO 21  
 68 BA 0.086  
 69 LS 0 78  
 70 UD 0.266  
 \*  
 71 KK NE COMBINE ROUTED REACH DE WITH BASIN B5 AT NODE E  
 72 KO 21  
 73 HC 2  
 \*  
 74 KK R-EF ROUTED REACH EF  
 75 KO 21

1

76 RM 4 .0658 .416

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

77 KK B6 RUNOFF FROM BASIN 6
78 KO 21
79 BA 0.111
80 LS 0 78
81 UD 0.311

82 KK NF COMBINE ROUTED REACH EF WITH BASIN B6 AT NODE F
83 KO 21
84 HC 2

85 KK R-FG ROUTE REACH FG
86 KO 21
87 RM 1 .0085 .416

88 KK B7 RUNOFF FROM BASIN 7
89 KO 21
90 BA 0.278
91 LS 0 78
92 UD 0.647

93 KK NG COMBINE ROUTED REACH FG WITH BASIN B7 AT NODE G
94 KO 21
95 HC 2

96 KK R-GH ROUTE REACH GH
97 KO 21
98 RM 2 .0371 .417

99 KK B8 RUNOFF FROM BASIN 8
100 KO 21
101 BA .020
102 LS 0 78
103 UD 0.112

104 KK NH COMBINE ROUTED REACH GH WITH BASIN B8 AT NODE H
105 KO 21
106 HC 2

107 KK R-HI ROUTE REACH HI
108 KO 21
109 RM 2 .030 .417
110 ZZ

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW

NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

11 B1
V
V
30 R-AB

33 . B2  
 .  
 .  
 38 NB.....  
 V  
 V  
 41 R-BC  
 .  
 .  
 44 . B3  
 .  
 .  
 49 NC .....  
 V  
 V  
 52 R-CD  
 .  
 .  
 55 . B4  
 .  
 .  
 60 ND.....  
 V  
 V  
 63 R-DE  
 .  
 .  
 66 . B5  
 .  
 .  
 71 NE.....  
 V  
 V  
 74 R-EF  
 .  
 .  
 77 . B6  
 .  
 .  
 82 NF.....  
 V  
 V  
 85 R-FG  
 .  
 .  
 88 . B7  
 .  
 .  
 93 NG.....  
 V  
 V  
 96 R-GH  
 .  
 .  
 99 . B8  
 .  
 .  
 104 NH.....  
 V  
 V  
 107 R-HI

(\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

\*  
 \* FLOOD HYDROGRAPH PACKAGE (HEC-1) \*  
 \* MAY 1991 \*  
 \* VERSION 4.0.1E \*  
 \* Lahey F77L-EM/32 version 5.01 \*  
 \* Dodson & Associates, Inc. \*

\*\*\*\*\*  
 \* U.S. ARMY CORPS OF ENGINEERS \*  
 \* HYDROLOGIC ENGINEERING CENTER \*  
 \* 609 SECOND STREET \*  
 \* DAVIS, CALIFORNIA 95616 \*  
 \* (916) 551-1748 \*

\* RUN DATE 12/08/99 TIME 14:52:11 \*

\*\*\*\*\*

Input File Name: P:\317\TASK12\HEC1\EAST1000.IH1  
DATE:12-08-99  
HEC-1 1000 YEAR RUNOFF STUDY, SHERWOOD TAILINGS, WESTERN WASHINGTON  
EAST DIVERSION CHANNEL MODIFIED TO REDUCE RIPRAP SIZE  
CURVE NUMBER = 78  
DESIGNED FOR 1000 YEAR PRECIPITATION, 3.13 in. IN 6 HOURS WITH SCS TYPE II DI  
ANTECEDENT MOISTURE CONDITION III

10 IO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA  
NMIN 1 MINUTES IN COMPUTATION INTERVAL  
IDATE 1 0 STARTING DATE  
ITIME 0000 STARTING TIME  
NQ 540 NUMBER OF HYDROGRAPH ORDINATES  
NDDATE 1 0 ENDING DATE  
NDTIME 0859 ENDING TIME  
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.02 HOURS  
TOTAL TIME BASE 8.98 HOURS

ENGLISH UNITS  
DRAINAGE AREA SQUARE MILES  
PRECIPITATION DEPTH INCHES  
LENGTH, ELEVATION FEET  
FLOW CUBIC FEET PER SECOND  
STORAGE VOLUME ACRE-FEET  
SURFACE AREA ACRES  
TEMPERATURE DEGREES FAHRENHEIT

\*\*\*\*\*

\*\*\*\*\*  
\* \*  
11 KK \* B1 \* RUNOFF FROM BASIN 1  
\* \*  
\*\*\*\*\*

12 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
\* \*

30 KK \* R-AB \* ROUTE REACH AB  
\*\*\*\*\*

31 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
33 KK \* B2 \* RUNOFF FROM BASIN 2  
\* \*  
\*\*\*\*\*

34 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
38 KK \* NB \* COMBINE ROUTED REACH AB WITH BASIN B2 AT NODE B  
\* \*  
\*\*\*\*\*

39 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
41 KK \* R-BC \* ROUTE REACH BC  
\* \*  
\*\*\*\*\*

42 KO    OUTPUT CONTROL VARIABLES  
IPRNT    5 PRINT CONTROL  
IPLOT    0 PLOT CONTROL  
QSCAL    0. HYDROGRAPH PLOT SCALE  
IPNCH    0 PUNCH COMPUTED HYDROGRAPH  
IOUT    21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2    540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT    0.017 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*            \*  
44 KK \*    B3 \*    RUNOFF FROM BASIN 3  
\*            \*  
\*\*\*\*\*

45 KO    OUTPUT CONTROL VARIABLES  
IPRNT    5 PRINT CONTROL  
IPLOT    0 PLOT CONTROL  
QSCAL    0. HYDROGRAPH PLOT SCALE  
IPNCH    0 PUNCH COMPUTED HYDROGRAPH  
IOUT    21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2    540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT    0.017 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*            \*  
49 KK \*    NC \*    COMBINE ROUTED REACH BC WITH BASIN B3 AT NODE C  
\*            \*  
\*\*\*\*\*

50 KO    OUTPUT CONTROL VARIABLES  
IPRNT    5 PRINT CONTROL  
IPLOT    0 PLOT CONTROL  
QSCAL    0. HYDROGRAPH PLOT SCALE  
IPNCH    0 PUNCH COMPUTED HYDROGRAPH  
IOUT    21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2    540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT    0.017 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\*            \*  
52 KK \*    R-CD \*    ROUTE REACH CD  
\*            \*  
\*\*\*\*\*

53 KO    OUTPUT CONTROL VARIABLES  
IPRNT    5 PRINT CONTROL  
IPLOT    0 PLOT CONTROL  
QSCAL    0. HYDROGRAPH PLOT SCALE



IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
\* \*  
55 KK \* B4 \* RUNOFF FROM BASIN 4  
\* \*  
\*\*\*\*\*

56 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
\* \*  
60 KK \* ND \* COMBINE ROUTED REACH CD WITH BASIN B4 AT NODE D  
\* \*  
\*\*\*\*\*

61 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
\* \*  
63 KK \* R-DE \* ROUTED REACH DE  
\* \*  
\*\*\*\*\*

64 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED

TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

```
*****
*
66 KK * B5 * RUNOFF FROM BASIN 5
*
*****
```

```
67 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.017 TIME INTERVAL IN HOURS
```

\*\*\*\*\*

```
*****
*
71 KK * NE * COMBINE ROUTED REACH DE WITH BASIN B5 AT NODE E
*
*****
```

```
72 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.017 TIME INTERVAL IN HOURS
```

\*\*\*\*\*

```
*****
*
74 KK * R-EF * ROUTED REACH EF
*
*****
```

```
75 KO OUTPUT CONTROL VARIABLES
IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 0 PUNCH COMPUTED HYDROGRAPH
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.017 TIME INTERVAL IN HOURS
```

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
77 KK \* B6 \* RUNOFF FROM BASIN 6  
\* \*  
\*\*\*\*\*

78 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
82 KK \* NF \* COMBINE ROUTED REACH EF WITH BASIN B6 AT NODE F  
\* \*  
\*\*\*\*\*

83 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
85 KK \* R-FG \* ROUTE REACH FG  
\* \*  
\*\*\*\*\*

86 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\* WARNING \*\*\*\* POSSIBLE INSTABILITIES IN THE MUSKINGUM ROUTING FOR REACH R-FG.  
REDUCE NSTPS OR DECREASE YOUR COMPUTATION INTERVAL (FIRST FIELD OF THE IT RECORD).

\*\*\* \*\*

```
*****
*      *
88 KK *  B7 *  RUNOFF FROM BASIN 7
*      *
*****
```

```
89 KO  OUTPUT CONTROL VARIABLES
      IPRNT  5 PRINT CONTROL
      IPLOT  0 PLOT CONTROL
      QSCAL  0. HYDROGRAPH PLOT SCALE
      IPNCH  0 PUNCH COMPUTED HYDROGRAPH
      IOUT   21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1  1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2  540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT 0.017 TIME INTERVAL IN HOURS
```

\*\*\*\*\*

```
*****
*      *
93 KK *  NG *  COMBINE ROUTED REACH FG WITH BASIN B7 AT NODE G
*      *
*****
```

```
94 KO  OUTPUT CONTROL VARIABLES
      IPRNT  5 PRINT CONTROL
      IPLOT  0 PLOT CONTROL
      QSCAL  0. HYDROGRAPH PLOT SCALE
      IPNCH  0 PUNCH COMPUTED HYDROGRAPH
      IOUT   21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1  1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2  540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT 0.017 TIME INTERVAL IN HOURS
```

\*\*\*\*\*

```
*****
*      *
96 KK *  R-GH *  ROUTE REACH GH
*      *
*****
```

```
97 KO  OUTPUT CONTROL VARIABLES
      IPRNT  5 PRINT CONTROL
      IPLOT  0 PLOT CONTROL
      QSCAL  0. HYDROGRAPH PLOT SCALE
      IPNCH  0 PUNCH COMPUTED HYDROGRAPH
      IOUT   21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1  1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2  540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT 0.017 TIME INTERVAL IN HOURS
```

\*\*\*\*\*

```
*****
*      *
```

99 KK \* B8 \* RUNOFF FROM BASIN 8  
 \* \*  
 \*\*\*\*\*

100 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
 \* \*  
 104 KK \* NH \* COMBINE ROUTED REACH GH WITH BASIN B8 AT NODE H  
 \* \*  
 \*\*\*\*\*

105 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
 \* \*  
 107 KK \* R-HI \* ROUTE REACH HI  
 \* \*  
 \*\*\*\*\*

108 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

1

RUNOFF SUMMARY  
 FLOW IN CUBIC FEET PER SECOND  
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK TIME OF AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
		6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT						
+	B1	47.	3.13	7.	4.	4.	0.05

+	ROUTED TO R-AB	47.	3.18	7.	4.	4.	0.05
+	HYDROGRAPH AT B2	40.	2.92	3.	2.	2.	0.02
+	2 COMBINED AT NB	58.	3.15	10.	6.	6.	0.07
+	ROUTED TO R-BC	58.	3.17	10.	6.	6.	0.07
+	HYDROGRAPH AT B3	150.	3.15	21.	14.	14.	0.16
+	2 COMBINED AT NC	208.	3.15	31.	21.	21.	0.23
+	ROUTED TO R-CD	208.	3.18	31.	21.	21.	0.23
+	HYDROGRAPH AT B4	241.	3.35	47.	31.	31.	0.35
+	2 COMBINED AT ND	428.	3.25	77.	52.	52.	0.59
+	ROUTED TO R-DE	427.	3.28	77.	52.	52.	0.59
+	HYDROGRAPH AT B5	86.	3.10	11.	8.	8.	0.09
+	2 COMBINED AT NE	490.	3.25	89.	59.	59.	0.68
+	ROUTED TO R-EF	489.	3.32	89.	59.	59.	0.68
+	HYDROGRAPH AT B6	100.	3.17	15.	10.	10.	0.11
+	2 COMBINED AT NF	570.	3.30	103.	69.	69.	0.79
+	ROUTED TO R-FG	570.	3.30	103.	69.	69.	0.79
+	HYDROGRAPH AT B7	149.	3.55	37.	24.	24.	0.28
+	2 COMBINED AT NG	693.	3.33	140.	93.	93.	1.06
+	ROUTED TO R-GH	692.	3.37	140.	93.	93.	1.06
+	HYDROGRAPH AT B8	33.	2.93	3.	2.	2.	0.02
+	2 COMBINED AT NH	698.	3.37	143.	95.	95.	1.08
+	ROUTED TO R-HI	698.	3.40	143.	95.	95.	1.08

\*\*\* NORMAL END OF HEC-1 \*\*\*

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   MAY 1991 *
*   VERSION 4.0.1E *
*   Lahey F77L-EM/32 version 5.01 *
*   Dodson & Associates, Inc. *
* RUN DATE 12/08/99 TIME 14:55:01 *
*****
    
```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
*   609 SECOND STREET *
*   DAVIS, CALIFORNIA 95616 *
*   (916) 551-1748 *
*****
    
```

```

X X XXXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X X
X X XXXXXXXX XXXXX XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW. THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Input File Name: P:\317\TASK12\HEC1\EST10000.IH1
2 ID DATE:12-08-99
3 ID HEC1 10000 YEAR RUNOFF STUDY, SHERWOOD TAILINGS, WESTERN WASHINGTON
4 ID EAST DIVERSION CHANNEL MODIFIED TO REDUCE RIPRAP SIZE
5 ID CURVE NUMBER = 78
6 ID DESIGNED FOR 10000 YR PRECIPITATION, 4.31 in. IN 6 HOURS WITH SCS TYPE II DI
7 ID ANTECEDENT MOISTURE CONDITION III
*DIAGRAM
8 IT 1 0 0000 540 0 0
9 IN 3 0 0
10 IO 5 0
*
11 KK B1 RUNOFF FROM BASIN 1
12 KO 21
13 PB 4.31
14 PC .0000 0.0024 0.0042 0.0062 0.0086 0.0110 0.0126 0.0148 0.0172 0.0198
15 PC .0230 0.0254 0.0278 0.0302 0.0326 0.0350 0.0374 0.0404 0.0432 0.0458
16 PC .0490 0.0514 0.0544 0.0572 0.0598 0.0630 0.0662 0.0694 0.0726 0.0760
17 PC .0800 0.0840 0.0880 0.0920 0.0960 0.1000 0.1048 0.1102 0.1158 0.1214
18 PC .1270 0.1326 0.1388 0.1452 0.1516 0.1580 0.1652 0.1730 0.1822 0.1928
19 PC .2040 0.2192 0.2374 0.2622 0.2998 0.3590 0.5174 0.6230 0.6802 0.7050
20 PC .7250 0.7450 0.7608 0.7736 0.7848 0.7960 0.8032 0.8104 0.8176 0.8246
21 PC .8310 0.8374 0.8438 0.8498 0.8554 0.8610 0.8658 0.8706 0.8750 0.8790
22 PC .8830 0.8870 0.8910 0.8950 0.8990 0.9030 0.9070 0.9104 0.9140 0.9178
23 PC .9210 0.9250 0.9284 0.9316 0.9348 0.9380 0.9412 0.9438 0.9466 0.9496
24 PC .9520 0.9552 0.9578 0.9606 0.9636 0.9660 0.9684 0.9708 0.9732 0.9756
25 PC .9780 0.9804 0.9828 0.9852 0.9874 0.9890 0.9914 0.9938 0.9958 0.9976
26 PC 1.000
27 BA 0.050
28 LS 0 78
29 UD .288
*
    
```

30 KK R-AB ROUTE REACH AB  
 31 KO 21  
 32 RM 4 .0586 .399  
 \*  
 33 KK B2 RUNOFF FROM BASIN 2  
 34 KO 21  
 35 BA .023  
 36 LS 0 78  
 37 UD 0.097  
 38 KK NB COMBINE ROUTED REACH AB WITH BASIN B2 AT NODE B  
 39 KO 21  
 40 HC 2  
 41 KK R-BC ROUTE REACH BC  
 42 KO 21  
 43 RM 1 .018 .387  
 HEC-1 INPUT

PAGE 2

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

44 KK B3 RUNOFF FROM BASIN 3  
 45 KO 21  
 46 BA .162  
 47 LS 0 78  
 48 UD .297  
 49 KK NC COMBINE ROUTED REACH BC WITH BASIN B3 AT NODE C  
 50 KO 21  
 51 HC 2  
 52 KK R-CD ROUTE REACH CD  
 53 KO 21  
 54 RM 2 .0348 .431  
 \*  
 55 KK B4 RUNOFF FROM BASIN 4  
 56 KO 21  
 57 BA .354  
 58 LS 0 78  
 59 UD 0.467  
 \*  
 60 KK ND COMBINE ROUTED REACH CD WITH BASIN B4 AT NODE D  
 61 KO 21  
 62 HC 2  
 \*  
 63 KK R-DE ROUTED REACH DE  
 64 KO 21  
 65 RM 2 .0343 .411  
 \*  
 66 KK B5 RUNOFF FROM BASIN 5  
 67 KO 21  
 68 BA 0.086  
 69 LS 0 78  
 70 UD 0.266  
 \*  
 71 KK NE COMBINE ROUTED REACH DE WITH BASIN B5 AT NODE E  
 72 KO 21  
 73 HC 2  
 \*  
 74 KK R-EF ROUTED REACH EF  
 75 KO 21  
 76 RM 4 .0658 .416



1

HEC-1 INPUT

PAGE 3

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
77	KK B6 RUNOFF FROM BASIN 6
78	KO 21
79	BA 0.111
80	LS 0 78
81	UD 0.311
	*
82	KK NF COMBINE ROUTED REACH EF WITH BASIN B6 AT NODE F
83	KO 21
84	HC 2
	*
85	KK R-FG ROUTE REACH FG
86	KO 21
87	RM 1 .0085 .416
	*
88	KK B7 RUNOFF FROM BASIN 7
89	KO 21
90	BA 0.278
91	LS 0 78
92	UD 0.647
	*
93	KK NG COMBINE ROUTED REACH FG WITH BASIN B7 AT NODE G
94	KO 21
95	HC 2
	*
96	KK R-GH ROUTE REACH GH
97	KO 21
98	RM 2 .0371 .417
	*
99	KK B8 RUNOFF FROM BASIN 8
100	KO 21
101	BA .020
102	LS 0 78
103	UD 0.112
	*
104	KK NH COMBINE ROUTED REACH GH WITH BASIN B8 AT NODE H
105	KO 21
106	HC 2
	*
107	KK R-HI ROUTE REACH HI
108	KO 21
109	RM 2 .030 .417
110	ZZ

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
11	B1	
	V	
	V	
30	R-AB	
	.	
	.	
33	B2	

```

      .
      .
38   NB.....
      V
      V
41   R-BC
      .
      .
44   .   B3
      .
      .
49   NC.....
      V
      V
52   R-CD
      .
      .
55   .   B4
      .
      .
60   ND.....
      V
      V
63   R-DE
      .
      .
66   .   B5
      .
      .
71   NE.....
      V
      V
74   R-EF
      .
      .
77   .   B6
      .
      .
82   NF.....
      V
      V
85   R-FG
      .
      .
88   .   B7
      .
      .
93   NG.....
      V
      V
96   R-GH
      .
      .
99   .   B8
      .
      .
104  NH.....
      V
      V
107  R-HI

```

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
*   FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   MAY 1991 *
*   VERSION 4.0.1E *
*   Lahey F77L-EM/32 version 5.01 *
*   Dodson & Associates, Inc. *
*   RUN DATE 12/08/99 TIME 14:55:01 *

```

```

*****
*
*   U.S. ARMY CORPS OF ENGINEERS *
*   HYDROLOGIC ENGINEERING CENTER *
*   609 SECOND STREET *
*   DAVIS, CALIFORNIA 95616 *
*   (916) 551-1748 *
*

```

\*\*\*\*\*

\*\*\*\*\*

Input File Name: P:\317\TASK12\HEC1\EST1000.IH1  
DATE:12-08-99  
HEC1 10000 YEAR RUNOFF STUDY, SHERWOOD TAILINGS, WESTERN WASHINGTON  
EAST DIVERSION CHANNEL MODIFIED TO REDUCE RIPRAP SIZE  
CURVE NUMBER = 78  
DESIGNED FOR 10000 YR PRECIPITATION, 4.31 in. IN 6 HOURS WITH SCS TYPE II DI  
ANTECEDENT MOISTURE CONDITION III

10 IO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA  
NMIN 1 MINUTES IN COMPUTATION INTERVAL  
IDATE 1 0 STARTING DATE  
ITIME 0000 STARTING TIME  
NQ 540 NUMBER OF HYDROGRAPH ORDINATES  
NDDATE 1 0 ENDING DATE  
NDTIME 0859 ENDING TIME  
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.02 HOURS  
TOTAL TIME BASE 8.98 HOURS

ENGLISH UNITS  
DRAINAGE AREA SQUARE MILES  
PRECIPITATION DEPTH INCHES  
LENGTH, ELEVATION FEET  
FLOW CUBIC FEET PER SECOND  
STORAGE VOLUME ACRE-FEET  
SURFACE AREA ACRES  
TEMPERATURE DEGREES FAHRENHEIT

\*\*\*\*\*

\*\*\*\*\*  
\* \*  
11 KK \* B1 \* RUNOFF FROM BASIN 1  
\* \*  
\*\*\*\*\*

12 KO OUTPUT CONTROL VARIABLES  
IPRNT 5 PRINT CONTROL  
IPLOT 0 PLOT CONTROL  
QSCAL 0. HYDROGRAPH PLOT SCALE  
IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
\* \*  
30 KK \* R-AB \* ROUTE REACH AB

\* \*  
\*\*\*\*\*

31 KO    OUTPUT CONTROL VARIABLES  
      IPRNT    5 PRINT CONTROL  
      IPLOT    0 PLOT CONTROL  
      QSCAL    0. HYDROGRAPH PLOT SCALE  
      IPNCH    0 PUNCH COMPUTED HYDROGRAPH  
      IOUT     21 SAVE HYDROGRAPH ON THIS UNIT  
      ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED  
      ISAV2    540 LAST ORDINATE PUNCHED OR SAVED  
      TIMINT   0.017 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
33 KK \* B2 \*  
\* \*  
\*\*\*\*\*

34 KO    OUTPUT CONTROL VARIABLES  
      IPRNT    5 PRINT CONTROL  
      IPLOT    0 PLOT CONTROL  
      QSCAL    0. HYDROGRAPH PLOT SCALE  
      IPNCH    0 PUNCH COMPUTED HYDROGRAPH  
      IOUT     21 SAVE HYDROGRAPH ON THIS UNIT  
      ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED  
      ISAV2    540 LAST ORDINATE PUNCHED OR SAVED  
      TIMINT   0.017 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
38 KK \* NB \*  
\* \*  
\*\*\*\*\*

39 KO    OUTPUT CONTROL VARIABLES  
      IPRNT    5 PRINT CONTROL  
      IPLOT    0 PLOT CONTROL  
      QSCAL    0. HYDROGRAPH PLOT SCALE  
      IPNCH    0 PUNCH COMPUTED HYDROGRAPH  
      IOUT     21 SAVE HYDROGRAPH ON THIS UNIT  
      ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED  
      ISAV2    540 LAST ORDINATE PUNCHED OR SAVED  
      TIMINT   0.017 TIME INTERVAL IN HOURS

\*\*\* \*\*

\*\*\*\*\*  
\* \*  
41 KK \* R-BC \*  
\* \*  
\*\*\*\*\*

42 KO    OUTPUT CONTROL VARIABLES

```

IPRNT      5 PRINT CONTROL
IPLOT      0 PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0 PUNCH COMPUTED HYDROGRAPH
IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2      540 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.017 TIME INTERVAL IN HOURS

```

\*\*\*\*\*

```

*****
*      *
44 KK *  B3 *  RUNOFF FROM BASIN 3
*      *
*****

```

```

45 KO      OUTPUT CONTROL VARIABLES
IPRNT      5 PRINT CONTROL
IPLOT      0 PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0 PUNCH COMPUTED HYDROGRAPH
IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2      540 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.017 TIME INTERVAL IN HOURS

```

\*\*\*\*\*

```

*****
*      *
49 KK *  NC *  COMBINE ROUTED REACH BC WITH BASIN B3 AT NODE C
*      *
*****

```

```

50 KO      OUTPUT CONTROL VARIABLES
IPRNT      5 PRINT CONTROL
IPLOT      0 PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0 PUNCH COMPUTED HYDROGRAPH
IOUT       21 SAVE HYDROGRAPH ON THIS UNIT
ISAV1      1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2      540 LAST ORDINATE PUNCHED OR SAVED
TIMINT     0.017 TIME INTERVAL IN HOURS

```

\*\*\*\*\*

```

*****
*      *
52 KK *  R-CD *  ROUTE REACH CD
*      *
*****

```

```

53 KO      OUTPUT CONTROL VARIABLES
IPRNT      5 PRINT CONTROL
IPLOT      0 PLOT CONTROL
QSCAL      0. HYDROGRAPH PLOT SCALE
IPNCH      0 PUNCH COMPUTED HYDROGRAPH

```

IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
 \* \*  
 55 KK \* B4 \* RUNOFF FROM BASIN 4  
 \* \*  
 \*\*\*\*\*

56 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
 \* \*  
 60 KK \* ND \* COMBINE ROUTED REACH CD WITH BASIN B4 AT NODE D  
 \* \*  
 \*\*\*\*\*

61 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

\*\*\*\*\*  
 \* \*  
 63 KK \* R-DE \* ROUTED REACH DE  
 \* \*  
 \*\*\*\*\*

64 KO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 IPNCH 0 PUNCH COMPUTED HYDROGRAPH  
 IOUT 21 SAVE HYDROGRAPH ON THIS UNIT  
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED  
 ISAV2 540 LAST ORDINATE PUNCHED OR SAVED  
 TIMINT 0.017 TIME INTERVAL IN HOURS

\*\*\*\*\*

```

*****
*           *
66 KK *   B5 *   RUNOFF FROM BASIN 5
*           *
*****
    
```

```

67 KO   OUTPUT CONTROL VARIABLES
      IPRNT   5 PRINT CONTROL
      IPLOT   0 PLOT CONTROL
      QSCAL   0. HYDROGRAPH PLOT SCALE
      IPNCH   0 PUNCH COMPUTED HYDROGRAPH
      IOUT    21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1   1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2   540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT  0.017 TIME INTERVAL IN HOURS
    
```

\*\*\*\*\*

```

*****
*           *
71 KK *   NE *   COMBINE ROUTED REACH DE WITH BASIN B5 AT NODE E
*           *
*****
    
```

```

72 KO   OUTPUT CONTROL VARIABLES
      IPRNT   5 PRINT CONTROL
      IPLOT   0 PLOT CONTROL
      QSCAL   0. HYDROGRAPH PLOT SCALE
      IPNCH   0 PUNCH COMPUTED HYDROGRAPH
      IOUT    21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1   1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2   540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT  0.017 TIME INTERVAL IN HOURS
    
```

\*\*\*\*\*

```

*****
*           *
74 KK *   R-EF *   ROUTED REACH EF
*           *
*****
    
```

```

75 KO   OUTPUT CONTROL VARIABLES
      IPRNT   5 PRINT CONTROL
      IPLOT   0 PLOT CONTROL
      QSCAL   0. HYDROGRAPH PLOT SCALE
      IPNCH   0 PUNCH COMPUTED HYDROGRAPH
      IOUT    21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1   1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2   540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT  0.017 TIME INTERVAL IN HOURS
    
```

\*\*\* \*\*

```

*****
*      *
77 KK *  B6 *  RUNOFF FROM BASIN 6
*      *
*****

```

```

78 KO  OUTPUT CONTROL VARIABLES
      IPRNT    5 PRINT CONTROL
      IPLOT    0 PLOT CONTROL
      QSCAL    0. HYDROGRAPH PLOT SCALE
      IPNCH    0 PUNCH COMPUTED HYDROGRAPH
      IOUT     21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2    540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT   0.017 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*      *
82 KK *  NF *  COMBINE ROUTED REACH EF WITH BASIN B6 AT NODE F
*      *
*****

```

```

83 KO  OUTPUT CONTROL VARIABLES
      IPRNT    5 PRINT CONTROL
      IPLOT    0 PLOT CONTROL
      QSCAL    0. HYDROGRAPH PLOT SCALE
      IPNCH    0 PUNCH COMPUTED HYDROGRAPH
      IOUT     21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2    540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT   0.017 TIME INTERVAL IN HOURS

```

\*\*\* \*\*

```

*****
*      *
85 KK *  R-FG *  ROUTE REACH FG
*      *
*****

```

```

86 KO  OUTPUT CONTROL VARIABLES
      IPRNT    5 PRINT CONTROL
      IPLOT    0 PLOT CONTROL
      QSCAL    0. HYDROGRAPH PLOT SCALE
      IPNCH    0 PUNCH COMPUTED HYDROGRAPH
      IOUT     21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2    540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT   0.017 TIME INTERVAL IN HOURS

```

\*\*\*\* WARNING \*\*\*\* POSSIBLE INSTABILITIES IN THE MUSKINGUM ROUTING FOR REACH R-FG.  
 REDUCE NSTPS OR DECREASE YOUR COMPUTATION INTERVAL (FIRST FIELD OF THE IT RECORD).

\*\*\* \*\*



```

*****
*      *
88 KK *  B7 *  RUNOFF FROM BASIN 7
*      *
*****

```

```

89 KO  OUTPUT CONTROL VARIABLES
      IPRNT    5 PRINT CONTROL
      IPLOT    0 PLOT CONTROL
      QSCAL    0. HYDROGRAPH PLOT SCALE
      IPNCH    0 PUNCH COMPUTED HYDROGRAPH
      IOUT     21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2    540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT   0.017 TIME INTERVAL IN HOURS

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*      *
93 KK *  NG *  COMBINE ROUTED REACH FG WITH BASIN B7 AT NODE G
*      *
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94 KO  OUTPUT CONTROL VARIABLES
      IPRNT    5 PRINT CONTROL
      IPLOT    0 PLOT CONTROL
      QSCAL    0. HYDROGRAPH PLOT SCALE
      IPNCH    0 PUNCH COMPUTED HYDROGRAPH
      IOUT     21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2    540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT   0.017 TIME INTERVAL IN HOURS

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*****
*      *
96 KK *  R-GH *  ROUTE REACH GH
*      *
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97 KO  OUTPUT CONTROL VARIABLES
      IPRNT    5 PRINT CONTROL
      IPLOT    0 PLOT CONTROL
      QSCAL    0. HYDROGRAPH PLOT SCALE
      IPNCH    0 PUNCH COMPUTED HYDROGRAPH
      IOUT     21 SAVE HYDROGRAPH ON THIS UNIT
      ISAV1    1 FIRST ORDINATE PUNCHED OR SAVED
      ISAV2    540 LAST ORDINATE PUNCHED OR SAVED
      TIMINT   0.017 TIME INTERVAL IN HOURS

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*      *
99 KK *  B8 *  RUNOFF FROM BASIN 8

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100 KO     OUTPUT CONTROL VARIABLES  
IPRNT     5 PRINT CONTROL  
IPLOT     0 PLOT CONTROL  
QSCAL     0. HYDROGRAPH PLOT SCALE  
IPNCH     0 PUNCH COMPUTED HYDROGRAPH  
IOUT      21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1     1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2     540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT    0.017 TIME INTERVAL IN HOURS

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104 KK \*   NH \*     COMBINE ROUTED REACH GH WITH BASIN B8 AT NODE H  
\*        \*  
\*\*\*\*\*

105 KO     OUTPUT CONTROL VARIABLES  
IPRNT     5 PRINT CONTROL  
IPLOT     0 PLOT CONTROL  
QSCAL     0. HYDROGRAPH PLOT SCALE  
IPNCH     0 PUNCH COMPUTED HYDROGRAPH  
IOUT      21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1     1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2     540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT    0.017 TIME INTERVAL IN HOURS

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\* \*

107 KK \*   R-HI \*    ROUTE REACH HI  
\*        \*  
\*\*\*\*\*

108 KO     OUTPUT CONTROL VARIABLES  
IPRNT     5 PRINT CONTROL  
IPLOT     0 PLOT CONTROL  
QSCAL     0. HYDROGRAPH PLOT SCALE  
IPNCH     0 PUNCH COMPUTED HYDROGRAPH  
IOUT      21 SAVE HYDROGRAPH ON THIS UNIT  
ISAV1     1 FIRST ORDINATE PUNCHED OR SAVED  
ISAV2     540 LAST ORDINATE PUNCHED OR SAVED  
TIMINT    0.017 TIME INTERVAL IN HOURS

1

RUNOFF SUMMARY  
FLOW IN CUBIC FEET PER SECOND  
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK TIME OF AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM TIME OF MAX STAGE	TIME OF
		6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT						
+	B1	86.	3.12	11.	8.	8.	0.05
	ROUTED TO						

+	R-AB	86.	3.18	11.	8.	8.	0.05
+	HYDROGRAPH AT B2	72.	2.92	5.	4.	4.	0.02
+	2 COMBINED AT NB	105.	3.15	17.	11.	11.	0.07
+	ROUTED TO R-BC	105.	3.17	17.	11.	11.	0.07
+	HYDROGRAPH AT B3	273.	3.13	37.	25.	25.	0.16
+	2 COMBINED AT NC	377.	3.13	54.	36.	36.	0.23
+	ROUTED TO R-CD	377.	3.17	54.	36.	36.	0.23
+	HYDROGRAPH AT B4	438.	3.32	81.	54.	54.	0.35
+	2 COMBINED AT ND	779.	3.23	135.	90.	90.	0.59
+	ROUTED TO R-DE	778.	3.27	135.	90.	90.	0.59
+	HYDROGRAPH AT B5	156.	3.10	20.	13.	13.	0.09
+	2 COMBINED AT NE	893.	3.23	155.	104.	104.	0.68
+	ROUTED TO R-EF	891.	3.30	155.	104.	104.	0.68
+	HYDROGRAPH AT B6	182.	3.15	26.	17.	17.	0.11
+	2 COMBINED AT NF	1039.	3.28	181.	121.	121.	0.79
+	ROUTED TO R-FG	1039.	3.28	181.	121.	121.	0.79
+	HYDROGRAPH AT B7	271.	3.53	64.	43.	43.	0.28
+	2 COMBINED AT NG	1264.	3.32	245.	163.	163.	1.06
+	ROUTED TO R-GH	1263.	3.35	245.	163.	163.	1.06
+	HYDROGRAPH AT B8	59.	2.93	5.	3.	3.	0.02
+	2 COMBINED AT NH	1272.	3.35	249.	166.	166.	1.08
+	ROUTED TO R-HI	1271.	3.38	249.	166.	166.	1.08

\*\*\* NORMAL END OF HEC-1 \*\*\*

**HEC-2 ANALYSES FOR 100-YR, 1,000-YR, AND 10,000-YR EVENTS**







XI	6200	6	200	284.99	199.44	200.6	200				
GR	128	115	120.5	200	107.5	239	110.43	254.78	120.5	284.99	
GR	130.5	314.99									
XI	6400	6	200	266.99	205.51	194.59	200				
GR	129	120	119	200	109	230	111.93	245.78	119	266.99	
GR	129	296.99									
QT	1	93									
XI	6600	6	200	266.99	203.03	196.86	200				
GR	130	108	120.5	200	110.5	230	113.43	245.78	120.5	266.99	
GR	130.5	296.99									
XI	6700	6	200	266.99	101.2	98.8	100				
GR	131.25	170	121.25	200	111.25	230	114.18	245.78	121.25	266.99	
GR	131.25	296.99									
XI	6800	6	200	266.99	101.19	98.8	100				
GR	132	170	122	200	112	230	114.93	245.78	122	266.99	
GR	132	296.99									
XI	6900	6	200	266.99	99.68	100.32	100				
GR	132.75	170	122.75	200	112.75	230	115.68	245.78	122.75	266.99	
GR	132.75	296.99									
QT	1	26									
XI	7000	6	200	266.99	99.67	100.33	100				
GR	133.5	170	123.5	200	113.5	230	116.43	245.78	123.5	266.99	
GR	133.5	296.99									
XI	7100	6	200	266.99	99.84	100.16	100				
GR	134.25	170	124.25	200	114.25	230	117.18	245.78	124.25	266.99	
GR	134.25	296.99									
XI	7200	6	200	266.99	99.83	100.16	100				
GR	135	170	125	200	115	230	117.93	245.78	125	266.99	
GR	135	296.99									
XI	7300	6	200	254.99	98.84	101.17	100				
GR	133.75	170	123.75	200	115.75	224	118.68	239.78	123.75	254.99	
GR	133.75	284.99									
XI	7400	6	200	254.99	98.85	101.17	100				
GR	134.5	170	124.5	200	116.5	224	119.43	239.78	124.5	254.99	
GR	134.5	284.99									
XI	7500	6	200	254.99	99.47	100.52	100				
GR	135.25	170	125.25	200	117.25	224	120.18	239.78	125.25	254.99	
GR	135.25	284.99									
XI	7600	6	200	254.99	99.46	100.53	100				
GR	136	170	126	200	118	224	120.93	239.78	126	254.99	
GR	136	284.99									
XI	7800	6	200	254.99	196.29	203.71	200				



GR 137.5 170 127.5 200 119.5 224 122.43 239.78 127.5 254.99  
 GR 137.5 284.99

X1 8000 6 200 254.99 199.4 200.6 200  
 GR 139 170 129 200 121 224 123.93 239.78 129 254.99  
 GR 139 284.99

X1 8200 7 200 254.99 192.62 203.75 200  
 GR 135 75 130.5 100 130.5 200 122.5 224 125.43 239.78  
 GR 130.5 254.99 140.5 284.99

QT 1 21  
 X1 8400 6 200 254.99 191.51 203.39 200  
 GR 142 135 132 200 124 224 126.93 239.78 132 254.99  
 GR 142 284.99

X1 8600 6 200 254.99 210.12 190.02 200  
 GR 139 135 133.5 200 125.5 224 128.43 239.78 133.5 254.99  
 GR 143.5 284.99

X1 8800 8 211 254.99 198.47 201.87 200  
 GR 136 37 134 98 130 180 128 211 127 224  
 GR 129.93 239.78 135 254.99 145 284.99

SECNO DEPTH CWSEL CRIWS WSELK EG HV HL OLOSS L-BANK ELEV  
 Q QLOB QCH QROB ALOB ACH AROB VOL TWA R-BANK ELEV  
 TIME VLOB VCH VROB XNL XNCH XNR WTN ELMIN SSTA  
 SLOPE XLOBL XLCH XLOBR ITRIAL IDC ICONT CORAR TOPWID ENDST

\*PROF 1

CRITICAL DEPTH TO BE CALCULATED AT ALL CROSS SECTIONS

CCHV= .100 CEHV= .300

\*SECNO .000  
 .000 4.34 91.34 90.18 100.48 91.60 .26 .00 .00 101.00  
 312.0 .0 312.0 .0 .0 76.6 .0 .0 .0 101.00  
 .00 .00 4.07 .00 .000 .031 .000 .000 87.00 228.98  
 .002465 0. 0. 0. 0 17 7 .00 33.03 262.01

\*SECNO 200.000  
 200.000 4.33 91.83 90.68 .00 92.09 .26 .50 .00 101.50  
 312.0 .0 312.0 .0 .0 76.4 .0 .4 .2 101.50  
 .01 .00 4.08 .00 .000 .031 .000 .000 87.50 229.00  
 .002485 202. 200. 198. 0 19 0 .00 32.99 261.99

\*SECNO 400.000  
 400.000 4.33 92.33 91.18 .00 92.59 .26 .50 .00 102.00  
 312.0 .0 312.0 .0 .0 76.3 .0 .7 .3 102.00  
 .03 .00 4.09 .00 .000 .031 .000 .000 88.00 229.01  
 .002496 203. 200. 197. 0 19 0 .00 32.97 261.98

\*SECNO 600.000  
 600.000 4.33 92.83 91.68 .00 93.09 .26 .50 .00 102.50  
 312.0 .0 312.0 .0 .0 76.3 .0 1.1 .5 102.50  
 .04 .00 4.09 .00 .000 .031 .000 .000 88.50 229.01  
 .002496 200. 200. 200. 0 19 0 .00 32.97 261.98

\*SECNO 700.000

700.000	4.08	93.08	92.18	.00	93.40	.32	.29	.02	103.00
312.0	.0	312.0	.0	.0	68.2	.0	1.2	.5	103.00
.05	.00	4.57	.00	.000	.031	.000	.000	89.00	229.76
.003395	101.	100.	99.	0	19	0	.00	31.48	261.23

\*SECNO 800.000

800.000	4.44	93.44	92.18	.00	93.68	.24	.27	.01	103.00
312.0	.0	312.0	.0	.0	80.0	.0	1.4	.6	103.00
.05	.00	3.90	.00	.000	.031	.000	.000	89.00	228.67
.002188	101.	100.	99.	2	15	0	.00	33.64	262.32

\*SECNO 1000.000

1000.000	4.39	93.89	92.68	.00	94.13	.25	.45	.00	103.50
312.0	.0	312.0	.0	.0	78.0	.0	1.8	.8	103.50
.07	.00	4.00	.00	.000	.031	.000	.000	89.50	228.85
.002346	196.	200.	204.	2	19	0	.00	33.28	262.14

\*SECNO 1200.000

1200.000	4.36	94.36	93.18	.00	94.61	.26	.48	.00	104.00
312.0	.0	312.0	.0	.0	76.9	.0	2.1	.9	104.00
.08	.00	4.05	.00	.000	.031	.000	.000	90.00	228.95
.002436	197.	200.	203.	1	19	0	.00	33.09	262.04

\*SECNO 1400.000

1400.000	4.35	94.85	93.68	.00	95.10	.26	.49	.00	104.50
312.0	.0	312.0	.0	.0	76.7	.0	2.5	1.1	104.50
.10	.00	4.07	.00	.000	.031	.000	.000	90.50	228.97
.002457	200.	200.	200.	1	19	0	.00	33.05	262.02

\*SECNO 1600.000

1600.000	4.34	95.34	94.18	.00	95.60	.26	.49	.00	105.00
312.0	.0	312.0	.0	.0	76.5	.0	2.8	1.2	105.00
.11	.00	4.08	.00	.000	.031	.000	.000	91.00	228.99
.002476	204.	200.	196.	0	19	0	.00	33.01	262.00

\*SECNO 1800.000

1800.000	4.34	95.84	94.68	.00	96.09	.26	.50	.00	105.50
312.0	.0	312.0	.0	.0	76.4	.0	3.2	1.4	105.50
.12	.00	4.08	.00	.000	.031	.000	.000	91.50	229.00
.002485	204.	200.	196.	0	19	0	.00	32.99	261.99

\*SECNO 2000.000

2000.000	4.33	96.33	95.17	.00	96.59	.25	.49	.00	106.00
309.0	.0	309.0	.0	.0	76.3	.0	3.5	1.5	106.00
.14	.00	4.05	.00	.000	.031	.000	.000	92.00	229.00
.002443	206.	200.	194.	0	19	0	.00	32.98	261.99

\*SECNO 2300.000

2300.000	4.53	97.03	95.67	.00	97.24	.22	.65	.00	106.50
309.0	.0	309.0	.0	.0	82.9	.0	4.1	1.7	106.50
.16	.00	3.73	.00	.000	.031	.000	.000	92.50	228.42
.001945	303.	300.	297.	2	19	0	.00	34.16	262.57

\*SECNO 2500.000

2500.000	4.43	97.43	95.95	.00	97.59	.16	.34	.01	107.00
254.0	.0	254.0	.0	.0	79.7	.0	4.4	1.9	107.00
.18	.00	3.19	.00	.000	.031	.000	.000	93.00	228.70
.001466	198.	200.	202.	2	14	0	.00	33.58	262.29

\*SECNO 2600.000

2600.000	4.07	97.57	96.45	.00	97.79	.22	.18	.02	107.50
254.0	.0	254.0	.0	.0	67.9	.0	4.6	2.0	107.50
.18	.00	3.74	.00	.000	.031	.000	.000	93.50	229.79
.002278	96.	100.	104.	2	14	0	.00	31.42	261.20

\*SECNO 2800.000

2800.000	4.03	98.03	96.94	.00	98.26	.22	.47	.00	108.00
254.0	.0	254.0	.0	.0	66.7	.0	4.9	2.1	108.00
.20	.00	3.81	.00	.000	.031	.000	.000	94.00	229.90
.002394	195.	200.	204.	2	19	0	.00	31.19	261.09

\*SECNO 3000.000

3000.000	4.02	98.52	97.44	.00	98.75	.23	.49	.00	108.50
254.0	.0	254.0	.0	.0	66.0	.0	5.2	2.3	108.50
.21	.00	3.85	.00	.000	.031	.000	.000	94.50	229.97
.002471	197.	200.	203.	1	19	0	.00	31.04	261.02

\*SECNO 3200.000

3200.000	4.01	99.01	97.94	.00	99.24	.23	.50	.00	109.00
254.0	.0	254.0	.0	.0	65.8	.0	5.5	2.4	109.00
.23	.00	3.86	.00	.000	.031	.000	.000	95.00	229.99
.002493	203.	200.	197.	0	19	0	.00	31.00	261.00

\*SECNO 3400.000

3400.000	4.01	99.51	98.44	.00	99.74	.23	.50	.00	109.50
254.0	.0	254.0	.0	.0	65.9	.0	5.8	2.5	109.50
.24	.00	3.85	.00	.000	.031	.000	.000	95.50	229.98
.002475	202.	200.	198.	0	19	0	.00	31.03	261.01

\*SECNO 3600.000

3600.000	4.00	100.00	98.94	.00	100.23	.23	.50	.00	110.00
254.0	.0	254.0	.0	.0	65.9	.0	6.1	2.7	110.00
.26	.00	3.85	.00	.000	.031	.000	.000	96.00	229.98
.002478	199.	200.	202.	0	19	0	.00	31.03	261.01

\*SECNO 3800.000

3800.000	4.00	100.50	99.44	.00	100.73	.23	.50	.00	110.50
254.0	.0	254.0	.0	.0	65.7	.0	6.4	2.8	110.50
.27	.00	3.87	.00	.000	.031	.000	.000	96.50	230.00
.002500	196.	200.	204.	0	19	0	.00	30.99	260.99

\*SECNO 4000.000

4000.000	4.00	101.00	99.94	.00	101.23	.23	.50	.00	111.00
254.0	.0	254.0	.0	.0	65.7	.0	6.7	3.0	111.00
.28	.00	3.87	.00	.000	.031	.000	.000	97.00	230.00
.002502	199.	200.	201.	0	19	0	.00	30.98	260.99

\*SECNO 4300.000

4300.000	4.20	101.70	100.44	.00	101.89	.19	.66	.00	111.50
254.0	.0	254.0	.0	.0	72.1	.0	7.2	3.2	111.50
.31	.00	3.52	.00	.000	.031	.000	.000	97.50	229.39
.001933	306.	300.	294.	2	19	0	.00	32.20	261.60

\*SECNO 4400.000

4400.000	3.91	101.91	100.78	.00	102.09	.19	.20	.00	112.00
219.0	.0	219.0	.0	.0	63.1	.0	7.4	3.3	112.00
.32	.00	3.47	.00	.000	.031	.000	.000	98.00	230.25
.002076	100.	100.	100.	1	14	0	.00	30.49	260.74

\*SECNO 4600.000

4600.000	3.84	102.34	101.28	.00	102.54	.20	.44	.00	112.50
219.0	.0	219.0	.0	.0	60.7	.0	7.6	3.4	112.50
.33	.00	3.61	.00	.000	.031	.000	.000	98.50	230.49
.002316	199.	200.	201.	2	19	0	.00	30.00	260.50

\*SECNO 4700.000

4700.000	3.82	102.57	101.53	.00	102.77	.21	.24	.00	112.75
219.0	.0	219.0	.0	.0	60.0	.0	7.8	3.5	112.75
.34	.00	3.65	.00	.000	.031	.000	.000	98.75	230.57
.002395	104.	100.	96.	1	19	0	.00	29.86	260.42

\*SECNO 4900.000

4900.000	3.80	103.05	102.03	.00	103.26	.21	.48	.00	113.25
219.0	.0	219.0	.0	.0	59.5	.0	8.1	3.6	113.25
.35	.00	3.68	.00	.000	.031	.000	.000	99.25	230.62
.002451	202.	200.	198.	1	19	0	.00	29.76	260.37

\*SECNO 5200.000

5200.000	3.74	103.74	102.58	.00	103.90	.16	.64	.01	114.00
191.0	.0	191.0	.0	.0	59.5	.0	8.5	3.8	114.00
.38	.00	3.21	.00	.000	.031	.000	.000	100.00	228.57
.001824	307.	300.	294.	2	19	0	.00	29.25	257.82

\*SECNO 5400.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = .40

5400.000	2.69	104.19	104.10	.00	104.77	.58	.74	.13	113.50
191.0	.0	191.0	.0	.0	31.3	.0	8.7	3.9	113.50
.39	.00	6.10	.00	.000	.031	.000	.000	101.50	230.25
.011302	206.	200.	194.	2	19	0	.00	23.25	253.50

\*SECNO 5600.000

5600.000	3.04	106.04	105.64	.00	106.42	.37	1.62	.02	116.00
191.0	.0	191.0	.0	.0	39.0	.0	8.8	4.0	116.00
.40	.00	4.90	.00	.000	.031	.000	.000	103.00	229.85
.006116	200.	200.	200.	2	8	0	.00	25.29	255.14

\*SECNO 5800.000

5800.000	2.89	107.39	107.13	.00	107.85	.46	1.41	.03	117.50
191.0	.0	191.0	.0	.0	35.1	.0	9.0	4.2	117.50
.41	.00	5.45	.00	.000	.031	.000	.000	104.50	230.33
.008226	200.	200.	200.	3	15	0	.00	24.25	254.57

\*SECNO 6000.000

6000.000	2.97	108.97	108.63	.00	109.39	.42	1.53	.00	119.00
191.0	.0	191.0	.0	.0	36.9	.0	9.2	4.3	119.00
.42	.00	5.18	.00	.000	.031	.000	.000	106.00	230.10
.007139	195.	200.	205.	1	11	0	.00	24.79	254.89

\*SECNO 6200.000

6200.000	2.93	110.43	110.13	.00	110.87	.44	1.48	.01	120.50
191.0	.0	191.0	.0	.0	36.1	.0	9.3	4.4	120.50
.43	.00	5.30	.00	.000	.031	.000	.000	107.50	230.20
.007627	199.	200.	201.	2	15	0	.00	24.58	254.79

\*SECNO 6400.000

6400.000	2.95	111.95	111.63	.00	112.38	.43	1.51	.00	119.00
191.0	.0	191.0	.0	.0	36.4	.0	9.5	4.5	119.00
.44	.00	5.25	.00	.000	.031	.000	.000	109.00	221.16
.007438	206.	200.	195.	1	15	0	.00	24.66	245.83

\*SECNO 6600.000

6600.000	2.75	113.25	112.47	.00	113.39	.13	.98	.03	120.50
93.0	.0	93.0	.0	.0	31.7	.0	9.7	4.6	120.50
.46	.00	2.93	.00	.000	.031	.000	.000	110.50	221.75
.002544	203.	200.	197.	2	19	0	.00	23.07	244.81

\*SECNO 6700.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = .62

6700.000	2.29	113.54	113.22	.00	113.82	.28	.39	.04	121.25
93.0	.0	93.0	.0	.0	22.1	.0	9.7	4.7	121.25
.47	.00	4.21	.00	.000	.031	.000	.000	111.25	223.12
.006686	101.	100.	99.	2	19	0	.00	19.24	242.36

\*SECNO 6800.000

6800.000	2.24	114.24	113.97	.00	114.54	.30	.71	.01	122.00
93.0	.0	93.0	.0	.0	21.0	.0	9.8	4.7	122.00
.47	.00	4.43	.00	.000	.031	.000	.000	112.00	223.29
.007647	101.	100.	99.	2	15	0	.00	18.77	242.05

\*SECNO 6900.000

6900.000	2.25	115.00	114.72	.00	115.30	.30	.76	.00	122.75
93.0	.0	93.0	.0	.0	21.1	.0	9.8	4.7	122.75
.48	.00	4.40	.00	.000	.031	.000	.000	112.75	223.26
.007505	100.	100.	100.	2	15	0	.00	18.83	242.10

\*SECNO 7000.000

7000.000	2.14	115.64	114.68	.00	115.67	.03	.35	.03	123.50
26.0	.0	26.0	.0	.0	19.3	.0	9.9	4.8	123.50
.50	.00	1.35	.00	.000	.031	.000	.000	113.50	223.57
.000753	100.	100.	100.	2	11	0	.00	17.97	241.54

\*SECNO 7100.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = .38

7100.000	1.49	115.74	115.44	.00	115.86	.12	.16	.03	124.25
26.0	.0	26.0	.0	.0	9.4	.0	9.9	4.8	124.25
.51	.00	2.77	.00	.000	.031	.000	.000	114.25	225.51
.005134	100.	100.	100.	2	11	0	.00	12.54	238.05

\*SECNO 7200.000

7200.000	1.35	116.35	116.18	.00	116.53	.18	.66	.02	125.00
26.0	.0	26.0	.0	.0	7.7	.0	9.9	4.8	125.00
.52	.00	3.38	.00	.000	.031	.000	.000	115.00	225.94
.008708	100.	100.	100.	2	15	0	.00	11.36	237.29

\*SECNO 7300.000

7300.000	1.41	117.16	116.93	.00	117.31	.15	.78	.00	123.75
26.0	.0	26.0	.0	.0	8.4	.0	9.9	4.9	123.75
.53	.00	3.11	.00	.000	.031	.000	.000	115.75	219.76
.006963	99.	100.	101.	2	15	0	.00	11.84	231.61

\*SECNO 7400.000

7400.000	1.38	117.88	117.68	.00	118.04	.16	.73	.00	124.50
26.0	.0	26.0	.0	.0	8.1	.0	10.0	4.9	124.50
.54	.00	3.22	.00	.000	.031	.000	.000	116.50	219.84
.007671	99.	100.	101.	1	15	0	.00	11.63	231.47

\*SECNO 7500.000  
 7500.000 1.39 118.64 118.43 .00 118.80 .16 .75 .00 125.25  
 26.0 .0 26.0 .0 .0 8.2 .0 10.0 4.9 125.25  
 .55 .00 3.19 .00 .000 .031 .000 .000 117.25 219.81  
 .007432 99. 100. 101. 1 15 0 .00 11.70 231.51

\*SECNO 7600.000  
 7600.000 1.39 119.39 119.18 .00 119.55 .16 .75 .00 126.00  
 26.0 .0 26.0 .0 .0 8.1 .0 10.0 5.0 126.00  
 .55 .00 3.21 .00 .000 .031 .000 .000 118.00 219.83  
 .007577 99. 100. 101. 1 15 0 .00 11.66 231.49

\*SECNO 7800.000  
 7800.000 1.39 120.89 120.68 .00 121.05 .16 1.49 .00 127.50  
 26.0 .0 26.0 .0 .0 8.2 .0 10.0 5.0 127.50  
 .57 .00 3.18 .00 .000 .031 .000 .000 119.50 219.81  
 .007374 196. 200. 204. 1 15 0 .00 11.72 231.52

\*SECNO 8000.000  
 8000.000 1.39 122.39 122.18 .00 122.55 .16 1.50 .00 129.00  
 26.0 .0 26.0 .0 .0 8.1 .0 10.1 5.1 129.00  
 .59 .00 3.22 .00 .000 .031 .000 .000 121.00 219.84  
 .007648 199. 200. 201. 2 15 0 .00 11.64 231.47

\*SECNO 8200.000  
 8200.000 1.40 123.90 123.68 .00 124.05 .16 1.50 .00 130.50  
 26.0 .0 26.0 .0 .0 8.2 .0 10.1 5.1 130.50  
 .61 .00 3.18 .00 .000 .031 .000 .000 122.50 219.81  
 .007392 193. 200. 204. 2 15 0 .00 11.71 231.52

\*SECNO 8400.000  
 8400.000 1.32 125.32 125.09 .00 125.45 .13 1.39 .00 132.00  
 21.0 .0 21.0 .0 .0 7.3 .0 10.1 5.2 132.00  
 .63 .00 2.87 .00 .000 .031 .000 .000 124.00 220.03  
 .006465 192. 200. 203. 2 15 0 .00 11.08 231.12

\*SECNO 8600.000  
 8600.000 1.26 126.76 126.59 .00 126.91 .15 1.46 .01 133.50  
 21.0 .0 21.0 .0 .0 6.7 .0 10.2 5.2 133.50  
 .64 .00 3.14 .00 .000 .031 .000 .000 125.50 220.21  
 .008272 210. 200. 190. 2 15 0 .00 10.58 230.80

\*SECNO 8800.000  
 8800.000 1.04 128.04 127.79 .00 128.11 .07 1.19 .01 128.00  
 21.0 .0 21.0 .0 .0 10.0 .0 10.2 5.3 135.00  
 .67 .03 2.11 .00 .031 .031 .000 .000 127.00 210.36  
 .004494 198. 200. 202. 4 14 0 .00 19.25 229.61

THIS RUN EXECUTED 14DEC99 13:16:07

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HEC-2 WATER SURFACE PROFILES

Version 4.6.2; May 1991

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NOTE- ASTERISK (\*) AT LEFT OF CROSS-SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST

NO SEDIMENT IN CHANNEL.

SUMMARY PRINTOUT

	SECNO	Q	CWSEL	ELMIN	DEPTH	TOPWID	AREA	VCH	FRCH	
A-H	.000	312.00	91.34	87.00	4.34	33.03	76.62	4.07	.47	
	200.000	312.00	91.83	87.50	4.33	32.99	76.39	4.08	.47	
	400.000	312.00	92.33	88.00	4.33	32.97	76.27	4.09	.47	
	600.000	312.00	92.83	88.50	4.33	32.97	76.28	4.09	.47	
	700.000	312.00	93.08	89.00	4.08	31.48	68.25	4.57	.55	
	800.000	312.00	93.44	89.00	4.44	33.64	80.00	3.90	.45	
G-H	1000.000	312.00	93.89	89.50	4.39	33.28	78.00	4.00	.46	
	1200.000	312.00	94.36	90.00	4.36	33.09	76.94	4.05	.47	
	1400.000	312.00	94.85	90.50	4.35	33.05	76.71	4.07	.47	
	1600.000	312.00	95.34	91.00	4.34	33.01	76.50	4.08	.47	
	1800.000	312.00	95.84	91.50	4.34	32.99	76.40	4.08	.47	
F-G	2000.000	309.00	96.33	92.00	4.33	32.98	76.34	4.05	.47	
	2300.000	309.00	97.03	92.50	4.53	34.16	82.91	3.73	.42	
E-F	2500.000	254.00	97.43	93.00	4.43	33.58	79.68	3.19	.36	
	2600.000	254.00	97.57	93.50	4.07	31.42	67.95	3.74	.45	
	2800.000	254.00	98.03	94.00	4.03	31.19	66.74	3.81	.46	
	3000.000	254.00	98.52	94.50	4.02	31.04	65.98	3.85	.47	
	3200.000	254.00	99.01	95.00	4.01	31.00	65.78	3.86	.47	
	3400.000	254.00	99.51	95.50	4.01	31.03	65.94	3.85	.47	
	3600.000	254.00	100.00	96.00	4.00	31.03	65.91	3.85	.47	
	3800.000	254.00	100.50	96.50	4.00	30.99	65.71	3.87	.47	
		4000.000	254.00	101.00	97.00	4.00	30.98	65.68	3.87	.47
		4300.000	254.00	101.70	97.50	4.20	32.20	72.09	3.52	.41
D-E	4400.000	219.00	101.91	98.00	3.91	30.49	63.14	3.47	.42	
	4600.000	219.00	102.34	98.50	3.84	30.00	60.70	3.61	.45	
	4700.000	219.00	102.57	98.75	3.82	29.86	59.98	3.65	.45	
	4900.000	219.00	103.05	99.25	3.80	29.76	59.48	3.68	.46	
	5200.000	191.00	103.74	100.00	3.74	29.25	59.51	3.21	.40	
	* 5400.000	191.00	104.19	101.50	2.69	23.25	31.31	6.10	.93	
	5600.000	191.00	106.04	103.00	3.04	25.29	38.97	4.90	.70	
	5800.000	191.00	107.39	104.50	2.89	24.25	35.05	5.45	.80	

6000.000	191.00	108.97	106.00	2.97	24.79	36.90	5.18	.75
6200.000	191.00	110.43	107.50	2.93	24.58	36.06	5.30	.77
6400.000	191.00	111.95	109.00	2.95	24.66	36.38	5.25	.76
6600.000	93.00	113.25	110.50	2.75	23.07	31.73	2.93	.44
* 6700.000	93.00	113.54	111.25	2.29	19.24	22.08	4.21	.69
6800.000	93.00	114.24	112.00	2.24	18.77	21.00	4.43	.74
6900.000	93.00	115.00	112.75	2.25	18.83	21.15	4.40	.73
7000.000	26.00	115.64	113.50	2.14	17.97	19.25	1.35	.23
* 7100.000	26.00	115.74	114.25	1.49	12.54	9.37	2.77	.57
7200.000	26.00	116.35	115.00	1.35	11.36	7.69	3.38	.72
7300.000	26.00	117.16	115.75	1.41	11.84	8.36	3.11	.65
7400.000	26.00	117.88	116.50	1.38	11.63	8.06	3.22	.68
7500.000	26.00	118.64	117.25	1.39	11.70	8.16	3.19	.67
7600.000	26.00	119.39	118.00	1.39	11.66	8.10	3.21	.68
7800.000	26.00	120.89	119.50	1.39	11.72	8.18	3.18	.67
8000.000	26.00	122.39	121.00	1.39	11.64	8.07	3.22	.68
8200.000	26.00	123.90	122.50	1.40	11.71	8.18	3.18	.67
8400.000	21.00	125.32	124.00	1.32	11.08	7.33	2.87	.62
8600.000	21.00	126.76	125.50	1.26	10.58	6.68	3.14	.70
8800.000	21.00	128.04	127.00	1.04	19.25	9.97	2.11	.51

SUMMARY OF ERRORS AND SPECIAL NOTES

WARNING SECNO= 5400.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

WARNING SECNO= 6700.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

WARNING SECNO= 7100.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE



\*\*\*\*\*  
 \* HEC-2 WATER SURFACE PROFILES \*  
 \* \*  
 \* Version 4.6.2; May 1991 \*  
 \* \*  
 \* RUN DATE 13DEC99 TIME 14:09:09 \*  
 \*\*\*\*\*

\*\*\*\*\*  
 \* U.S. ARMY CORPS OF ENGINEERS \*  
 \* HYDROLOGIC ENGINEERING CENTER \*  
 \* 609 SECOND STREET, SUITE D \*  
 \* DAVIS, CALIFORNIA 95616-4687 \*  
 \* (916) 756-1104 \*  
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THIS RUN EXECUTED 13DEC99 14:09:09

\*\*\*\*\*  
 HEC-2 WATER SURFACE PROFILES  
 Version 4.6.2; May 1991  
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T1 INPUT FILE NAME: P:\317\TASK31\SEDIMENT\HEC2\NOSED.IH2-----SUBCRITICAL  
 T2 DATE: 12/99 DIVERSION CHANNEL ASSUMING GRASS LINED CHAN ("n"=0.031) AND  
 T3 NO SEDIMENT IN CHANNEL. 1000 YR STORM EVENT

J1 ICHECK INQ NINV IDIR STRT METRIC HVINS Q WSEL FQ  
 0 2 0 .0025 0 0 0 100.48

J2 NPROF IPLOT PRFVS XSECV XSECH FN ALLDC IBW CHNIM ITRACE  
 -1 -1 -1

J3 VARIABLE CODES FOR SUMMARY PRINTOUT

	38	43	1	42	8	4	25	26	68			
NC	.031	.031	.031	0.1	0.3							
QT	1	698										
X1	0	6	200	290.99	0	0	0					
GR	111	170	101	200	87	242	89.93	257.78	101	290.99		
GR	111	320.99										
X1	200	6	200	290.99	201.61	198.48	200					
GR	111.5	170	101.5	200	87.5	242	90.43	257.78	101.5	290.99		
GR	111.5	320.99										
X1	400	6	200	290.99	203.24	197.46	200					
GR	112	170	102	200	88	242	90.93	257.78	102	290.99		
GR	112	320.99										
X1	600	6	200	290.99	200	200	200					

GR 112.5 170 102.5 200 88.5 242 91.43 257.78 102.5 290.99  
GR 112.5 320.99

X1 700 6 200 290.99 100.99 99.26 100  
GR 112.75 170 103 200 89 242 91.93 257.78 103 290.99  
GR 112.75 320.99

X1 800 6 200 290.99 100.99 99.26 100  
GR 113 170 103 200 89 242 91.93 257.78 103 290.99  
GR 113 320.99

QT 1 698  
X1 1000 6 200 290.99 195.76 204.31 200  
GR 113.5 170 103.5 200 89.5 242 92.43 257.78 103.5 290.99  
GR 113.5 320.99

X1 1200 6 200 290.99 196.98 203.14 200  
GR 114 170 104 200 90 242 92.93 257.78 104 290.99  
GR 114 320.99

X1 1400 6 200 290.99 200.1 199.78 200  
GR 114.5 170 104.5 200 90.5 242 93.43 257.78 104.5 290.99  
GR 114.5 320.99

X1 1600 6 200 290.99 203.63 196 200  
GR 115 170 105 200 91 242 93.93 257.78 105 290.99  
GR 115 320.99

X1 1800 6 200 290.99 204.45 195.93 200  
GR 115.5 137 105.5 200 91.5 242 94.43 257.78 105.5 290.99  
GR 115.5 320.99

QT 1 693  
X1 2000 8 200 290.99 206.03 194.18 200  
GR 116 170 106 200 92 242 94.93 257.78 106 290.99  
GR 109.5 303.99 112 333.99 116 338.99

X1 2300 6 200 290.99 302.91 296.91 300  
GR 116.75 20 106.5 200 92.5 242 95.43 257.78 106.5 290.99  
GR 116.75 320.99

QT 1 570  
X1 2500 8 200 290.99 197.98 202.05 200  
GR 117.25 133.25 112 166.75 110 191.75 107 200 93 242  
GR 95.93 257.78 107 290.99 117.25 320.99

X1 2600 6 200 290.99 95.84 104.14 100  
GR 117.5 170 107.5 200 93.5 242 96.43 257.78 107.5 290.99  
GR 117.5 320.99

X1 2800 6 200 290.99 195.31 203.77 200  
GR 118 170 108 200 94 242 96.93 257.78 108 290.99  
GR 118 320.99

X1 3000 6 200 290.99 196.79 203.1 200  
GR 118.5 170 108.5 200 94.5 242 97.43 257.78 108.5 290.99  
GR 118.5 320.99

X1	3200	6	200	290.99	202.86	196.79	200				
GR	119	170	109	200	95	242	97.93	257.78	109	290.99	
GR	119	320.99									

X1	3400	7	200	290.99	202.15	197.69	200			
GR	119.5	131	114.5	185.5	109.5	200	95.5	242	98.43	257.78
GR	109.5	290.99	119.5	320.99						

X1	3600	6	200	290.99	198.73	201.73	200			
GR	120	170	110	200	96	242	98.93	257.78	110	290.99
GR	120	320.99								

X1	3800	6	200	290.99	196.34	203.70	200			
GR	120.5	170	110.5	200	96.5	242	99.43	257.78	110.5	290.99
GR	120.5	320.99								

X1	4000	8	200	290.99	199.39	200.58	200			
GR	121	117	114.5	158	112	184	111	200	97	242
GR	99.93	257.78	111	290.99	121	320.99				

X1	4300	6	200	290.99	305.81	294.2	300			
GR	121.75	170	111.5	200	97.5	242	100.43	257.78	111.5	290.99
GR	121.75	320.99								

QT	1	490								
X1	4400	6	200	290.99	99.71	100.29	100			
GR	122	170	112	200	98	242	100.93	257.78	112	290.99
GR	122	320.99								

X1	4600	6	200	290.99	199.21	200.78	200			
GR	122.5	170	112.5	200	98.5	242	101.43	257.78	112.5	290.99
GR	122.5	320.99								

X1	4700	6	200	290.99	103.78	96.24	100			
GR	122.75	170	112.75	200	98.75	242	101.68	257.78	112.75	290.99
GR	122.75	320.99								

X1	4900	6	200	290.99	202.21	197.8	200			
GR	123	118	113.25	200	99.25	242	102.18	257.78	113.25	290.99
GR	125.25	320.99								

QT	1	428								
X1	5200	6	200	285.89	306.91	294	300			
GR	122	130	114	200	100	239	102.63	254.78	114	285.89
GR	124	315.89								

X1	5400	6	200	284.99	205.7	194.4	200			
GR	123.5	170	113.5	200	101.5	239	104.43	254.78	113.5	284.99
GR	123.5	314.99								

X1	5600	6	200	284.99	199.85	200.1	200			
GR	126	170	116	200	103	239	105.93	254.78	116	284.99
GR	126	314.99								

X1	5800	6	200	284.99	199.86	200.45	200			
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GR 127.5 170 117.5 200 104.5 239 107.43 254.78 117.5 284.99  
GR 127.5 314.99

X1 6000 6 200 284.99 195.23 204.69 200  
GR 129 170 119 200 106 239 108.93 254.78 119 284.99  
GR 129 314.99

X1 6200 6 200 284.99 199.44 200.6 200  
GR 128 115 120.5 200 107.5 239 110.43 254.78 120.5 284.99  
GR 130.5 314.99

X1 6400 6 200 266.99 205.51 194.59 200  
GR 129 120 119 200 109 230 111.93 245.78 119 266.99  
GR 129 296.99

QT 1 208  
X1 6600 6 200 266.99 203.03 196.86 200  
GR 130 108 120.5 200 110.5 230 113.43 245.78 120.5 266.99  
GR 130.5 296.99

X1 6700 6 200 266.99 101.2 98.8 100  
GR 131.25 170 121.25 200 111.25 230 114.18 245.78 121.25 266.99  
GR 131.25 296.99

X1 6800 6 200 266.99 101.19 98.8 100  
GR 132 170 122 200 112 230 114.93 245.78 122 266.99  
GR 132 296.99

X1 6900 6 200 266.99 99.68 100.32 100  
GR 132.75 170 122.75 200 112.75 230 115.68 245.78 122.75 266.99  
GR 132.75 296.99

QT 1 58  
X1 7000 6 200 266.99 99.67 100.33 100  
GR 133.5 170 123.5 200 113.5 230 116.43 245.78 123.5 266.99  
GR 133.5 296.99

X1 7100 6 200 266.99 99.84 100.16 100  
GR 134.25 170 124.25 200 114.25 230 117.18 245.78 124.25 266.99  
GR 134.25 296.99

X1 7200 6 200 266.99 99.83 100.16 100  
GR 135 170 125 200 115 230 117.93 245.78 125 266.99  
GR 135 296.99

X1 7300 6 200 254.99 98.84 101.17 100  
GR 133.75 170 123.75 200 115.75 224 118.68 239.78 123.75 254.99  
GR 133.75 284.99

X1 7400 6 200 254.99 98.85 101.17 100  
GR 134.5 170 124.5 200 116.5 224 119.43 239.78 124.5 254.99  
GR 134.5 284.99

X1 7500 6 200 254.99 99.47 100.52 100  
GR 135.25 170 125.25 200 117.25 224 120.18 239.78 125.25 254.99  
GR 135.25 284.99

X1	7600	6	200	254.99	99.46	100.53	100			
GR	136	170	126	200	118	224	120.93	239.78	126	254.99
GR	136	284.99								

X1	7800	6	200	254.99	196.29	203.71	200			
GR	137.5	170	127.5	200	119.5	224	122.43	239.78	127.5	254.99
GR	137.5	284.99								

X1	8000	6	200	254.99	199.4	200.6	200			
GR	139	170	129	200	121	224	123.93	239.78	129	254.99
GR	139	284.99								

X1	8200	7	200	254.99	192.62	203.75	200			
GR	135	75	130.5	100	130.5	200	122.5	224	125.43	239.78
GR	130.5	254.99	140.5	284.99						

QT	1	47								
X1	8400	6	200	254.99	191.51	203.39	200			
GR	142	135	132	200	124	224	126.93	239.78	132	254.99
GR	142	284.99								

X1	8600	6	200	254.99	210.12	190.02	200			
GR	139	135	133.5	200	125.5	224	128.43	239.78	133.5	254.99
GR	143.5	284.99								

X1	8800	8	211	254.99	198.47	201.87	200			
GR	136	37	134	98	130	180	128	211	127	224
GR	129.93	239.78	135	254.99	145	284.99				

\*PROF 1

CRITICAL DEPTH TO BE CALCULATED AT ALL CROSS SECTIONS

CCHV= .100 CEHV= .300

\*SECNO .000

.000	5.96	92.96	91.42	100.48	93.36	.40	.00	.00	101.00
698.0	.0	698.0	.0	.0	138.0	.0	.0	.0	101.00
.00	.00	5.06	.00	.000	.031	.000	.000	87.00	224.12
.002459	0.	0.	0.	0	17	6	.00	42.76	266.87

\*SECNO 200.000

200.000	5.95	93.45	91.91	.00	93.85	.40	.49	.00	101.50
698.0	.0	698.0	.0	.0	137.7	.0	.6	.2	101.50
.01	.00	5.07	.00	.000	.031	.000	.000	87.50	224.14
.002476	202.	200.	198.	0	19	0	.00	42.71	266.85

\*SECNO 400.000

400.000	5.95	93.95	92.41	.00	94.35	.40	.50	.00	102.00
698.0	.0	698.0	.0	.0	137.4	.0	1.3	.4	102.00
.02	.00	5.08	.00	.000	.031	.000	.000	88.00	224.16
.002488	203.	200.	197.	0	19	0	.00	42.67	266.83

\*SECNO 600.000

600.000	5.95	94.45	92.91	.00	94.85	.40	.50	.00	102.50
698.0	.0	698.0	.0	.0	137.3	.0	1.9	.6	102.50
.03	.00	5.08	.00	.000	.031	.000	.000	88.50	224.17
.002493	200.	200.	200.	0	19	0	.00	42.66	266.82

\*SECNO 700.000

700.000	5.67	94.67	93.41	.00	95.15	.48	.28	.02	103.00
698.0	.0	698.0	.0	.0	125.9	.0	2.2	.7	103.00
.04	.00	5.54	.00	.000	.031	.000	.000	89.00	224.98
.003156	101.	100.	99.	2	19	0	.00	41.03	266.01

\*SECNO 800.000

800.000	6.05	95.05	93.41	.00	95.43	.38	.27	.01	103.00
698.0	.0	698.0	.0	.0	141.9	.0	2.5	.8	103.00
.04	.00	4.92	.00	.000	.031	.000	.000	89.00	223.85
.002281	101.	100.	99.	2	15	0	.00	43.30	267.14

\*SECNO 1000.000

1000.000	6.01	95.51	93.91	.00	95.89	.39	.47	.00	103.50
698.0	.0	698.0	.0	.0	139.9	.0	3.2	1.0	103.50
.05	.00	4.99	.00	.000	.031	.000	.000	89.50	223.99
.002372	196.	200.	204.	2	19	0	.00	43.01	267.00

\*SECNO 1200.000

1200.000	5.98	95.98	94.41	.00	96.38	.39	.48	.00	104.00
698.0	.0	698.0	.0	.0	138.8	.0	3.8	1.2	104.00
.07	.00	5.03	.00	.000	.031	.000	.000	90.00	224.07
.002423	197.	200.	203.	1	19	0	.00	42.86	266.92

\*SECNO 1400.000

1400.000	5.97	96.47	94.91	.00	96.87	.40	.49	.00	104.50
698.0	.0	698.0	.0	.0	138.1	.0	4.4	1.4	104.50
.08	.00	5.05	.00	.000	.031	.000	.000	90.50	224.11
.002456	200.	200.	200.	0	19	0	.00	42.77	266.88

\*SECNO 1600.000

1600.000	5.96	96.96	95.41	.00	97.36	.40	.49	.00	105.00
698.0	.0	698.0	.0	.0	137.8	.0	5.1	1.6	105.00
.09	.00	5.06	.00	.000	.031	.000	.000	91.00	224.13
.002470	204.	200.	196.	0	19	0	.00	42.73	266.86

\*SECNO 1800.000

1800.000	5.95	97.45	95.91	.00	97.85	.40	.49	.00	105.50
698.0	.0	698.0	.0	.0	137.7	.0	5.7	1.8	105.50
.10	.00	5.07	.00	.000	.031	.000	.000	91.50	224.14
.002477	204.	200.	196.	0	19	0	.00	42.71	266.85

\*SECNO 2000.000

2000.000	5.95	97.95	96.40	.00	98.35	.39	.49	.00	106.00
693.0	.0	693.0	.0	.0	137.5	.0	6.3	2.0	106.00
.11	.00	5.04	.00	.000	.031	.000	.000	92.00	224.15
.002450	206.	200.	194.	0	19	0	.00	42.68	266.84

\*SECNO 2300.000

2300.000	6.17	98.67	96.90	.00	99.02	.34	.67	.01	106.50
693.0	.0	693.0	.0	.0	147.6	.0	7.3	2.3	106.50
.13	.00	4.69	.00	.000	.031	.000	.000	92.50	223.45
.002019	303.	300.	297.	1	19	0	.00	44.08	267.54

\*SECNO 2500.000

2500.000	6.13	99.13	97.07	.00	99.37	.24	.34	.01	107.00
570.0	.0	570.0	.0	.0	145.0	.0	8.0	2.5	107.00
.14	.00	3.93	.00	.000	.031	.000	.000	93.00	223.64
.001435	198.	200.	202.	0	14	0	.00	43.72	267.35

\*SECNO 2600.000

2600.000	5.75	99.25	97.57	.00	99.56	.30	.17	.02	107.50
570.0	.0	570.0	.0	.0	129.3	.0	8.3	2.6	107.50
.15	.00	4.41	.00	.000	.031	.000	.000	93.50	224.74
.001959	96.	100.	104.	2	14	0	.00	41.51	266.25

\*SECNO 2800.000

2800.000	5.65	99.65	98.06	.00	99.97	.32	.41	.01	108.00
570.0	.0	570.0	.0	.0	125.0	.0	8.9	2.7	108.00
.16	.00	4.56	.00	.000	.031	.000	.000	94.00	225.05
.002149	195.	200.	204.	2	19	0	.00	40.89	265.94

\*SECNO 3000.000

3000.000	5.58	100.08	98.56	.00	100.42	.34	.44	.00	108.50
570.0	.0	570.0	.0	.0	122.2	.0	9.4	2.9	108.50
.17	.00	4.66	.00	.000	.031	.000	.000	94.50	225.25
.002283	197.	200.	203.	2	19	0	.00	40.48	265.74

\*SECNO 3200.000

3200.000	5.54	100.54	99.06	.00	100.89	.35	.47	.00	109.00
570.0	.0	570.0	.0	.0	120.3	.0	10.0	3.1	109.00
.18	.00	4.74	.00	.000	.031	.000	.000	95.00	225.40
.002386	203.	200.	197.	1	19	0	.00	40.19	265.59

\*SECNO 3400.000

3400.000	5.52	101.02	99.56	.00	101.37	.35	.48	.00	109.50
570.0	.0	570.0	.0	.0	119.5	.0	10.6	3.3	109.50
.20	.00	4.77	.00	.000	.031	.000	.000	95.50	225.46
.002429	202.	200.	198.	1	19	0	.00	40.07	265.53

\*SECNO 3600.000

3600.000	5.51	101.51	100.06	.00	101.86	.36	.49	.00	110.00
570.0	.0	570.0	.0	.0	119.0	.0	11.1	3.5	110.00
.21	.00	4.79	.00	.000	.031	.000	.000	96.00	225.50
.002457	199.	200.	202.	0	19	0	.00	40.00	265.49

\*SECNO 3800.000

3800.000	5.50	102.00	100.56	.00	102.36	.36	.49	.00	110.50
570.0	.0	570.0	.0	.0	118.7	.0	11.6	3.7	110.50
.22	.00	4.80	.00	.000	.031	.000	.000	96.50	225.52
.002472	196.	200.	204.	0	19	0	.00	39.96	265.47

\*SECNO 4000.000

4000.000	5.49	102.49	101.06	.00	102.85	.36	.50	.00	111.00
570.0	.0	570.0	.0	.0	118.6	.0	12.2	3.9	111.00
.23	.00	4.81	.00	.000	.031	.000	.000	97.00	225.53
.002481	199.	200.	201.	0	19	0	.00	39.93	265.46

\*SECNO 4300.000

4300.000	5.72	103.22	101.56	.00	103.52	.31	.67	.01	111.50
570.0	.0	570.0	.0	.0	128.1	.0	13.0	4.1	111.50
.25	.00	4.45	.00	.000	.031	.000	.000	97.50	224.82
.002008	306.	300.	294.	1	19	0	.00	41.34	266.17

\*SECNO 4400.000

4400.000	5.45	103.45	101.81	.00	103.72	.27	.20	.00	112.00
490.0	.0	490.0	.0	.0	116.6	.0	13.3	4.2	112.00
.26	.00	4.20	.00	.000	.031	.000	.000	98.00	225.68
.001919	100.	100.	100.	0	19	0	.00	39.64	265.31

\*SECNO 4600.000

4600.000	5.34	103.84	102.31	.00	104.13	.29	.40	.01	112.50
490.0	.0	490.0	.0	.0	112.7	.0	13.8	4.4	112.50
.27	.00	4.35	.00	.000	.031	.000	.000	98.50	225.97
.002105	199.	200.	201.	0	19	0	.00	39.04	265.02

\*SECNO 4700.000

4700.000	5.30	104.05	102.56	.00	104.35	.30	.22	.00	112.75
490.0	.0	490.0	.0	.0	110.7	.0	14.1	4.5	112.75
.27	.00	4.43	.00	.000	.031	.000	.000	98.75	226.13
.002214	104.	100.	96.	0	19	0	.00	38.73	264.86

\*SECNO 4900.000

4900.000	5.24	104.49	103.06	.00	104.81	.32	.45	.00	113.25
490.0	.0	490.0	.0	.0	108.6	.0	14.6	4.7	113.25
.29	.00	4.51	.00	.000	.031	.000	.000	99.25	226.29
.002332	202.	200.	198.	1	19	0	.00	38.40	264.70

\*SECNO 5200.000

5200.000	5.18	105.18	103.54	.00	105.43	.25	.62	.01	114.00
428.0	.0	428.0	.0	.0	107.1	.0	15.3	4.9	114.00
.31	.00	4.00	.00	.000	.031	.000	.000	100.00	224.58
.001792	307.	300.	294.	0	19	0	.00	37.16	261.74

\*SECNO 5400.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = .54

5400.000	4.05	105.55	105.07	.00	106.14	.59	.61	.10	113.50
428.0	.0	428.0	.0	.0	69.7	.0	15.8	5.1	113.50
.32	.00	6.14	.00	.000	.031	.000	.000	101.50	225.82
.006240	206.	200.	194.	2	19	0	.00	32.71	258.53

\*SECNO 5600.000

5600.000	3.88	106.88	106.61	.00	107.62	.74	1.43	.05	116.00
428.0	.0	428.0	.0	.0	62.1	.0	16.1	5.2	116.00
.32	.00	6.89	.00	.000	.031	.000	.000	103.00	227.35
.008297	200.	200.	200.	2	15	0	.00	30.28	257.64

\*SECNO 5800.000

5800.000	4.00	108.50	108.11	.00	109.16	.66	1.54	.01	117.50
428.0	.0	428.0	.0	.0	65.6	.0	16.4	5.4	117.50
.33	.00	6.52	.00	.000	.031	.000	.000	104.50	227.01
.007132	200.	200.	200.	2	11	0	.00	30.97	257.98

\*SECNO 6000.000

6000.000	3.95	109.95	109.61	.00	110.65	.69	1.47	.01	119.00
428.0	.0	428.0	.0	.0	64.1	.0	16.6	5.5	119.00
.34	.00	6.68	.00	.000	.031	.000	.000	106.00	227.16
.007620	195.	200.	205.	2	15	0	.00	30.67	257.83

\*SECNO 6200.000

6200.000	3.97	111.47	111.11	.00	112.15	.68	1.50	.00	120.50
428.0	.0	428.0	.0	.0	64.7	.0	16.9	5.7	120.50
.35	.00	6.61	.00	.000	.031	.000	.000	107.50	227.10
.007403	199.	200.	201.	2	11	0	.00	30.80	257.89

\*SECNO 6400.000

6400.000	3.95	112.95	112.61	.00	113.64	.68	1.49	.00	119.00
428.0	.0	428.0	.0	.0	64.5	.0	17.2	5.8	119.00
.36	.00	6.64	.00	.000	.031	.000	.000	109.00	218.12



.007474 206. 200. 195. 1 11 0 .00 30.75 248.87

\*SECNO 6600.000

3301 HV CHANGED MORE THAN HVINS

6600.000 3.88 114.38 113.22 .00 114.56 .17 .87 .05 120.50  
208.0 .0 208.0 .0 .0 62.1 .0 17.5 5.9 120.50  
.37 .00 3.35 .00 .000 .031 .000 .000 110.50 218.35  
.001958 203. 200. 197. 2 19 0 .00 30.29 248.64

\*SECNO 6700.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = .66

6700.000 3.32 114.57 113.97 .00 114.88 .32 .28 .04 121.25  
208.0 .0 208.0 .0 .0 46.1 .0 17.7 6.0 121.25  
.38 .00 4.51 .00 .000 .031 .000 .000 111.25 220.03  
.004512 101. 100. 99. 2 19 0 .00 26.93 246.96

\*SECNO 6800.000

6800.000 3.04 115.04 114.72 .00 115.49 .44 .56 .04 122.00  
208.0 .0 208.0 .0 .0 38.9 .0 17.8 6.1 122.00  
.39 .00 5.34 .00 .000 .031 .000 .000 112.00 220.86  
.007267 101. 100. 99. 2 15 0 .00 25.28 246.13

\*SECNO 6900.000

6900.000 3.03 115.78 115.47 .00 116.23 .45 .74 .00 122.75  
208.0 .0 208.0 .0 .0 38.4 .0 17.8 6.1 122.75  
.39 .00 5.41 .00 .000 .031 .000 .000 112.75 220.91  
.007540 100. 100. 100. 2 15 0 .00 25.16 246.08

\*SECNO 7000.000

7000.000 3.04 116.54 115.13 .00 116.58 .03 .30 .04 123.50  
58.0 .0 58.0 .0 .0 38.8 .0 17.9 6.2 123.50  
.41 .00 1.50 .00 .000 .031 .000 .000 113.50 220.87  
.000571 100. 100. 100. 2 11 0 .00 25.24 246.12

\*SECNO 7100.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = .50

7100.000 2.35 116.60 115.88 .00 116.70 .10 .10 .02 124.25  
58.0 .0 58.0 .0 .0 23.2 .0 18.0 6.2 124.25  
.42 .00 2.50 .00 .000 .031 .000 .000 114.25 222.94  
.002271 100. 100. 100. 2 11 0 .00 19.74 242.68

\*SECNO 7200.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = .55

7200.000 1.88 116.88 116.63 .00 117.12 .24 .38 .04 125.00  
58.0 .0 58.0 .0 .0 14.9 .0 18.0 6.3 125.00  
.43 .00 3.89 .00 .000 .031 .000 .000 115.00 224.34  
.007423 100. 100. 100. 2 19 0 .00 15.81 240.15

\*SECNO 7300.000

7300.000 1.88 117.63 117.38 .00 117.87 .24 .75 .00 123.75  
58.0 .0 58.0 .0 .0 14.8 .0 18.1 6.3 123.75  
.44 .00 3.92 .00 .000 .031 .000 .000 115.75 218.37  
.007588 99. 100. 101. 2 15 0 .00 15.74 234.11

\*SECNO 7400.000

7400.000	1.89	118.39	118.13	.00	118.62	.24	.76	.00	124.50
58.0	.0	58.0	.0	.0	14.8	.0	18.1	6.4	124.50
.44	.00	3.92	.00	.000	.031	.000	.000	116.50	218.37
.007571	99.	100.	101.	0	15	0	.00	15.75	234.12

\*SECNO 7500.000

7500.000	1.89	119.14	118.88	.00	119.37	.23	.74	.00	125.25
58.0	.0	58.0	.0	.0	15.0	.0	18.1	6.4	125.25
.45	.00	3.86	.00	.000	.031	.000	.000	117.25	218.32
.007242	99.	100.	101.	0	15	0	.00	15.88	234.20

\*SECNO 7600.000

7600.000	1.87	119.87	119.63	.00	120.11	.24	.74	.00	126.00
58.0	.0	58.0	.0	.0	14.8	.0	18.2	6.4	126.00
.46	.00	3.93	.00	.000	.031	.000	.000	118.00	218.37
.007626	99.	100.	101.	0	15	0	.00	15.73	234.10

\*SECNO 7800.000

7800.000	1.88	121.38	121.13	.00	121.62	.24	1.51	.00	127.50
58.0	.0	58.0	.0	.0	14.9	.0	18.2	6.5	127.50
.47	.00	3.90	.00	.000	.031	.000	.000	119.50	218.35
.007442	196.	200.	204.	2	15	0	.00	15.80	234.15

\*SECNO 8000.000

8000.000	1.89	122.89	122.63	.00	123.13	.24	1.51	.00	129.00
58.0	.0	58.0	.0	.0	14.8	.0	18.3	6.6	129.00
.49	.00	3.93	.00	.000	.031	.000	.000	121.00	218.37
.007610	199.	200.	201.	1	15	0	.00	15.74	234.11

\*SECNO 8200.000

8200.000	1.89	124.39	124.13	.00	124.62	.23	1.49	.00	130.50
58.0	.0	58.0	.0	.0	15.0	.0	18.4	6.6	130.50
.50	.00	3.87	.00	.000	.031	.000	.000	122.50	218.33
.007313	193.	200.	204.	1	15	0	.00	15.85	234.18

\*SECNO 8400.000

8400.000	1.80	125.80	125.50	.00	125.98	.19	1.36	.00	132.00
47.0	.0	47.0	.0	.0	13.6	.0	18.5	6.7	132.00
.52	.00	3.46	.00	.000	.031	.000	.000	124.00	218.60
.006261	192.	200.	203.	2	15	0	.00	15.08	233.69

\*SECNO 8600.000

8600.000	1.70	127.20	127.00	.00	127.44	.23	1.44	.01	133.50
47.0	.0	47.0	.0	.0	12.2	.0	18.5	6.8	133.50
.53	.00	3.86	.00	.000	.031	.000	.000	125.50	218.89
.008332	210.	200.	190.	2	15	0	.00	14.30	233.18

\*SECNO 8800.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = 1.49

8800.000	1.42	128.42	128.10	.00	128.52	.10	1.08	.01	128.00
47.0	1.4	45.6	.0	1.4	17.5	.0	18.6	6.9	135.00
.55	1.04	2.61	.00	.031	.031	.000	.000	127.00	204.44
.003754	198.	200.	202.	3	14	0	.00	27.22	231.66

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HEC-2 WATER SURFACE PROFILES

Version 4.6.2; May 1991

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NOTE- ASTERISK (\*) AT LEFT OF CROSS-SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST

NO SEDIMENT IN CHANNEL.  
SUMMARY PRINTOUT

	SECNO	Q	CWSEL	ELMIN	DEPTH	TOPWID	AREA	VCH	FRCH	
	.000	698.00	92.96	87.00	5.96	42.76	138.02	5.06	.50	
D-14	200.000	698.00	93.45	87.50	5.95	42.71	137.68	5.07	.50	
	400.000	698.00	93.95	88.00	5.95	42.67	137.43	5.08	.50	
	600.000	698.00	94.45	88.50	5.95	42.66	137.34	5.08	.50	
	700.000	698.00	94.67	89.00	5.67	41.03	125.95	5.54	.56	
	800.000	698.00	95.05	89.00	6.05	43.30	141.89	4.92	.48	
D-13	1000.000	698.00	95.51	89.50	6.01	43.01	139.86	4.99	.49	
	1200.000	698.00	95.98	90.00	5.98	42.86	138.77	5.03	.49	
	1400.000	698.00	96.47	90.50	5.97	42.77	138.10	5.05	.50	
	1600.000	698.00	96.96	91.00	5.96	42.73	137.81	5.06	.50	
	1800.000	698.00	97.45	91.50	5.95	42.71	137.67	5.07	.50	
D-12	2000.000	693.00	97.95	92.00	5.95	42.68	137.49	5.04	.49	
	2300.000	693.00	98.67	92.50	6.17	44.08	147.63	4.69	.45	
D-11	2500.000	570.00	99.13	93.00	6.13	43.72	144.97	3.93	.38	
	2600.000	570.00	99.25	93.50	5.75	41.51	129.31	4.41	.44	
	2800.000	570.00	99.65	94.00	5.65	40.89	124.99	4.56	.46	
	3000.000	570.00	100.08	94.50	5.58	40.48	122.25	4.66	.47	
	3200.000	570.00	100.54	95.00	5.54	40.19	120.27	4.74	.48	
	3400.000	570.00	101.02	95.50	5.52	40.07	119.49	4.77	.49	
	3600.000	570.00	101.51	96.00	5.51	40.00	118.99	4.79	.49	
	3800.000	570.00	102.00	96.50	5.50	39.96	118.73	4.80	.49	
		4000.000	570.00	102.49	97.00	5.49	39.93	118.57	4.81	.49
		4300.000	570.00	103.22	97.50	5.72	41.34	128.12	4.45	.45
D-10	4400.000	490.00	103.45	98.00	5.45	39.64	116.61	4.20	.43	
	4600.000	490.00	103.84	98.50	5.34	39.04	112.73	4.35	.45	
	4700.000	490.00	104.05	98.75	5.30	38.73	110.66	4.43	.46	
		4900.000	490.00	104.49	99.25	5.24	38.40	108.58	4.51	.47
	5200.000	428.00	105.18	100.00	5.18	37.16	107.07	4.00	.41	
	* 5400.000	428.00	105.55	101.50	4.05	32.71	69.70	6.14	.74	

C-D	5600.000	428.00	106.88	103.00	3.88	30.28	62.12	6.89	.85
	5800.000	428.00	108.50	104.50	4.00	30.97	65.60	6.52	.79
	6000.000	428.00	109.95	106.00	3.95	30.67	64.05	6.68	.81
	6200.000	428.00	111.47	107.50	3.97	30.80	64.72	6.61	.80
	6400.000	428.00	112.95	109.00	3.95	30.75	64.50	6.64	.81
B-C	6600.000	208.00	114.38	110.50	3.88	30.29	62.14	3.35	.41
	* 6700.000	208.00	114.57	111.25	3.32	26.93	46.10	4.51	.61
	6800.000	208.00	115.04	112.00	3.04	25.28	38.94	5.34	.76
A-B	6900.000	208.00	115.78	112.75	3.03	25.16	38.44	5.41	.77
	7000.000	58.00	116.54	113.50	3.04	25.24	38.79	1.50	.21
	* 7100.000	58.00	116.60	114.25	2.35	19.74	23.23	2.50	.41
	* 7200.000	58.00	116.88	115.00	1.88	15.81	14.90	3.89	.71
	7300.000	58.00	117.63	115.75	1.88	15.74	14.78	3.92	.71
	7400.000	58.00	118.39	116.50	1.89	15.75	14.79	3.92	.71
	7500.000	58.00	119.14	117.25	1.89	15.88	15.04	3.86	.70
	7600.000	58.00	119.87	118.00	1.87	15.73	14.75	3.93	.72
	7800.000	58.00	121.38	119.50	1.88	15.80	14.89	3.90	.71
	8000.000	58.00	122.89	121.00	1.89	15.74	14.76	3.93	.71
Above A	8200.000	58.00	124.39	122.50	1.89	15.85	14.99	3.87	.70
	8400.000	47.00	125.80	124.00	1.80	15.08	13.57	3.46	.64
	* 8600.000	47.00	127.20	125.50	1.70	14.30	12.19	3.86	.74
	* 8800.000	47.00	128.42	127.00	1.42	27.22	18.84	2.61	.50

SUMMARY OF ERRORS AND SPECIAL NOTES

WARNING SECNO= 5400.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

WARNING SECNO= 6700.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

WARNING SECNO= 7100.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

WARNING SECNO= 7200.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

WARNING SECNO= 8800.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

\*\*\*\*\*  
 \* HEC-2 WATER SURFACE PROFILES \*  
 \* \*  
 \* Version 4.6.2; May 1991 \*  
 \* \*  
 \* RUN DATE 14DEC99 TIME 13:26:58 \*  
 \*\*\*\*\*

\*\*\*\*\*  
 \* U.S. ARMY CORPS OF ENGINEERS \*  
 \* HYDROLOGIC ENGINEERING CENTER \*  
 \* 609 SECOND STREET, SUITE D \*  
 \* DAVIS, CALIFORNIA 95616-4687 \*  
 \* (916) 756-1104 \*  
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X X XXXXXXXX XXXXX XXXXX
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THIS RUN EXECUTED 14DEC99 13:26:58

\*\*\*\*\*  
 HEC-2 WATER SURFACE PROFILES

Version 4.6.2; May 1991

\*\*\*\*\*

T1 INPUT FILE NAME: P:\317\TASK31\SEDIMENT\HEC2\10000.IH2-----SUBCRITICAL  
 T2 DATE: 12/99 DIVERSION CHANNEL ASSUMING GRASS LINED CHAN ("n"=0.031) AND  
 T3 NO SEDIMENT IN CHANNEL. 10000 YR STORM EVENT

J1 ICHECK INQ NINV IDIR STRT METRIC HVINS Q WSEL FQ  
 0 2 0 .0025 0 0 0 100.48

J2 NPROF IPLOT PRFVS XSECV XSECH FN ALLDC IBW CHNIM ITRACE  
 -1 -1 -1

J3 VARIABLE CODES FOR SUMMARY PRINTOUT

	38	43	1	42	8	4	25	26	68
NC	.031	.031	.031	0.1	0.3				
QT	1	1271							
X1	0	6	200	290.99	0	0	0		
GR	111	170	101	200	87	242	89.93	257.78	101 290.99
GR	111	320.99							
X1	200	6	200	290.99	201.61	198.48	200		
GR	111.5	170	101.5	200	87.5	242	90.43	257.78	101.5 290.99
GR	111.5	320.99							
X1	400	6	200	290.99	203.24	197.46	200		
GR	112	170	102	200	88	242	90.93	257.78	102 290.99
GR	112	320.99							
X1	600	6	200	290.99	200	200	200		
GR	112.5	170	102.5	200	88.5	242	91.43	257.78	102.5 290.99
GR	112.5	320.99							
X1	700	6	200	290.99	100.99	99.26	100		

GR 112.75 170 103 200 89 242 91.93 257.78 103 290.99  
GR 112.75 320.99

X1 800 6 200 290.99 100.99 99.26 100  
GR 113 170 103 200 89 242 91.93 257.78 103 290.99  
GR 113 320.99

QT 1 1272  
X1 1000 6 200 290.99 195.76 204.31 200  
GR 113.5 170 103.5 200 89.5 242 92.43 257.78 103.5 290.99  
GR 113.5 320.99

X1 1200 6 200 290.99 196.98 203.14 200  
GR 114 170 104 200 90 242 92.93 257.78 104 290.99  
GR 114 320.99

X1 1400 6 200 290.99 200.1 199.78 200  
GR 114.5 170 104.5 200 90.5 242 93.43 257.78 104.5 290.99  
GR 114.5 320.99

X1 1600 6 200 290.99 203.63 196 200  
GR 115 170 105 200 91 242 93.93 257.78 105 290.99  
GR 115 320.99

X1 1800 6 200 290.99 204.45 195.93 200  
GR 115.5 137 105.5 200 91.5 242 94.43 257.78 105.5 290.99  
GR 115.5 320.99

QT 1 1264  
X1 2000 8 200 290.99 206.03 194.18 200  
GR 116 170 106 200 92 242 94.93 257.78 106 290.99  
GR 109.5 303.99 112 333.99 116 338.99

X1 2300 6 200 290.99 302.91 296.91 300  
GR 116.75 20 106.5 200 92.5 242 95.43 257.78 106.5 290.99  
GR 116.75 320.99

QT 1 1039  
X1 2500 8 200 290.99 197.98 202.05 200  
GR 117.25 133.25 112 166.75 110 191.75 107 200 93 242  
GR 95.93 257.78 107 290.99 117.25 320.99

X1 2600 6 200 290.99 95.84 104.14 100  
GR 117.5 170 107.5 200 93.5 242 96.43 257.78 107.5 290.99  
GR 117.5 320.99

X1 2800 6 200 290.99 195.31 203.77 200  
GR 118 170 108 200 94 242 96.93 257.78 108 290.99  
GR 118 320.99

X1 3000 6 200 290.99 196.79 203.1 200  
GR 118.5 170 108.5 200 94.5 242 97.43 257.78 108.5 290.99  
GR 118.5 320.99

X1 3200 6 200 290.99 202.86 196.79 200  
GR 119 170 109 200 95 242 97.93 257.78 109 290.99  
GR 119 320.99

XI	3400	7	200	290.99	202.15	197.69	200				
GR	119.5	131	114.5	185.5	109.5	200	95.5	242	98.43	257.78	
GR	109.5	290.99	119.5	320.99							
XI	3600	6	200	290.99	198.73	201.73	200				
GR	120	170	110	200	96	242	98.93	257.78	110	290.99	
GR	120	320.99									
XI	3800	6	200	290.99	196.34	203.70	200				
GR	120.5	170	110.5	200	96.5	242	99.43	257.78	110.5	290.99	
GR	120.5	320.99									
XI	4000	8	200	290.99	199.39	200.58	200				
GR	121	117	114.5	158	112	184	111	200	97	242	
GR	99.93	257.78	111	290.99	121	320.99					
XI	4300	6	200	290.99	305.81	294.2	300				
GR	121.75	170	111.5	200	97.5	242	100.43	257.78	111.5	290.99	
GR	121.75	320.99									
QT	1	893									
XI	4400	6	200	290.99	99.71	100.29	100				
GR	122	170	112	200	98	242	100.93	257.78	112	290.99	
GR	122	320.99									
XI	4600	6	200	290.99	199.21	200.78	200				
GR	122.5	170	112.5	200	98.5	242	101.43	257.78	112.5	290.99	
GR	122.5	320.99									
XI	4700	6	200	290.99	103.78	96.24	100				
GR	122.75	170	112.75	200	98.75	242	101.68	257.78	112.75	290.99	
GR	122.75	320.99									
XI	4900	6	200	290.99	202.21	197.8	200				
GR	123	118	113.25	200	99.25	242	102.18	257.78	113.25	290.99	
GR	125.25	320.99									
QT	1	779									
XI	5200	6	200	285.89	306.91	294	300				
GR	122	130	114	200	100	239	102.63	254.78	114	285.89	
GR	124	315.89									
XI	5400	6	200	284.99	205.7	194.4	200				
GR	123.5	170	113.5	200	101.5	239	104.43	254.78	113.5	284.99	
GR	123.5	314.99									
XI	5600	6	200	284.99	199.85	200.1	200				
GR	126	170	116	200	103	239	105.93	254.78	116	284.99	
GR	126	314.99									
XI	5800	6	200	284.99	199.86	200.45	200				
GR	127.5	170	117.5	200	104.5	239	107.43	254.78	117.5	284.99	
GR	127.5	314.99									
XI	6000	6	200	284.99	195.23	204.69	200				

GR	129	170	119	200	106	239	108.93	254.78	119	284.99
GR	129	314.99								

X1	6200	6	200	284.99	199.44	200.6	200			
GR	128	115	120.5	200	107.5	239	110.43	254.78	120.5	284.99
GR	130.5	314.99								

X1	6400	6	200	266.99	205.51	194.59	200			
GR	129	120	119	200	109	230	111.93	245.78	119	266.99
GR	129	296.99								

QT	1	377								
X1	6600	6	200	266.99	203.03	196.86	200			
GR	130	108	120.5	200	110.5	230	113.43	245.78	120.5	266.99
GR	130.5	296.99								

X1	6700	6	200	266.99	101.2	98.8	100			
GR	131.25	170	121.25	200	111.25	230	114.18	245.78	121.25	266.99
GR	131.25	296.99								

X1	6800	6	200	266.99	101.19	98.8	100			
GR	132	170	122	200	112	230	114.93	245.78	122	266.99
GR	132	296.99								

X1	6900	6	200	266.99	99.68	100.32	100			
GR	132.75	170	122.75	200	112.75	230	115.68	245.78	122.75	266.99
GR	132.75	296.99								

QT	1	105								
X1	7000	6	200	266.99	99.67	100.33	100			
GR	133.5	170	123.5	200	113.5	230	116.43	245.78	123.5	266.99
GR	133.5	296.99								

X1	7100	6	200	266.99	99.84	100.16	100			
GR	134.25	170	124.25	200	114.25	230	117.18	245.78	124.25	266.99
GR	134.25	296.99								

X1	7200	6	200	266.99	99.83	100.16	100			
GR	135	170	125	200	115	230	117.93	245.78	125	266.99
GR	135	296.99								

X1	7300	6	200	254.99	98.84	101.17	100			
GR	133.75	170	123.75	200	115.75	224	118.68	239.78	123.75	254.99
GR	133.75	284.99								

X1	7400	6	200	254.99	98.85	101.17	100			
GR	134.5	170	124.5	200	116.5	224	119.43	239.78	124.5	254.99
GR	134.5	284.99								

X1	7500	6	200	254.99	99.47	100.52	100			
GR	135.25	170	125.25	200	117.25	224	120.18	239.78	125.25	254.99
GR	135.25	284.99								

X1	7600	6	200	254.99	99.46	100.53	100			
GR	136	170	126	200	118	224	120.93	239.78	126	254.99
GR	136	284.99								



X1	7800	6	200	254.99	196.29	203.71	200				
GR	137.5	170	127.5	200	119.5	224	122.43	239.78	127.5	254.99	
GR	137.5	284.99									

X1	8000	6	200	254.99	199.4	200.6	200				
GR	139	170	129	200	121	224	123.93	239.78	129	254.99	
GR	139	284.99									

X1	8200	7	200	254.99	192.62	203.75	200				
GR	135	75	130.5	100	130.5	200	122.5	224	125.43	239.78	
GR	130.5	254.99	140.5	284.99							

QT	1	86									
X1	8400	6	200	254.99	191.51	203.39	200				
GR	142	135	132	200	124	224	126.93	239.78	132	254.99	
GR	142	284.99									

X1	8600	6	200	254.99	210.12	190.02	200				
GR	139	135	133.5	200	125.5	224	128.43	239.78	133.5	254.99	
GR	143.5	284.99									

X1	8800	8	211	254.99	198.47	201.87	200				
GR	136	37	134	98	130	180	128	211	127	224	
GR	129.93	239.78	135	254.99	145	284.99					

SECNO DEPTH CWSEL CRIWS WSELK EG HV HL OLOSS L-BANK ELEV  
 Q QLOB QCH QROB ALOB ACH AROB VOL TWA R-BANK ELEV  
 TIME VLOB VCH VROB XNL XNCH XNR WTN ELMIN SSTA  
 SLOPE XLOBL XLCH XLOBR ITRIAL IDC ICONT CORAR TOPWID ENDST

\*PROF 1

CRITICAL DEPTH TO BE CALCULATED AT ALL CROSS SECTIONS

CCHV= .100 CEHV= .300

\*SECNO .000

.000	7.55	94.55	92.68	100.48	95.10	.55	.00	.00	101.00		
1271.0	.0	1271.0	.0	.0	213.6	.0	.0	.0	101.00		
.00	.00	5.95	.00	.000	.031	.000	.000	87.00	219.35		
.002498	0.	0.	0.	0	18	6	.00	52.29	271.64		

\*SECNO 200.000

200.000	7.55	95.05	93.18	.00	95.60	.55	.50	.00	101.50		
1271.0	.0	1271.0	.0	.0	213.5	.0	1.0	.2	101.50		
.01	.00	5.95	.00	.000	.031	.000	.000	87.50	219.35		
.002498	202.	200.	198.	0	19	0	.00	52.29	271.64		

\*SECNO 400.000

400.000	7.55	95.55	93.68	.00	96.10	.55	.50	.00	102.00		
1271.0	.0	1271.0	.0	.0	213.5	.0	2.0	.5	102.00		
.02	.00	5.95	.00	.000	.031	.000	.000	88.00	219.35		
.002498	203.	200.	197.	0	19	0	.00	52.29	271.64		

\*SECNO 600.000

600.000	7.55	96.05	94.18	.00	96.60	.55	.50	.00	102.50		
1271.0	.0	1271.0	.0	.0	213.5	.0	2.9	.7	102.50		
.03	.00	5.95	.00	.000	.031	.000	.000	88.50	219.35		
.002500	200.	200.	200.	0	19	0	.00	52.28	271.64		

\*SECNO 700.000

700.000	7.26	96.26	94.68	.00	96.90	.63	.27	.03	103.00
1271.0	.0	1271.0	.0	.0	198.9	.0	3.4	.8	103.00
.03	.00	6.39	.00	.000	.031	.000	.000	89.00	220.21
.003028	101.	100.	99.	2	19	0	.00	50.58	270.78

\*SECNO 800.000

800.000	7.65	96.65	94.68	.00	97.17	.52	.26	.01	103.00
1271.0	.0	1271.0	.0	.0	218.9	.0	3.9	1.0	103.00
.04	.00	5.81	.00	.000	.031	.000	.000	89.00	219.04
.002336	101.	100.	99.	2	15	0	.00	52.90	271.95

\*SECNO 1000.000

1000.000	7.62	97.12	95.18	.00	97.65	.53	.47	.00	103.50
1272.0	.0	1272.0	.0	.0	216.9	.0	4.9	1.2	103.50
.05	.00	5.87	.00	.000	.031	.000	.000	89.50	219.16
.002400	196.	200.	204.	2	19	0	.00	52.67	271.83

\*SECNO 1200.000

1200.000	7.60	97.60	95.68	.00	98.14	.54	.48	.00	104.00
1272.0	.0	1272.0	.0	.0	215.7	.0	5.9	1.4	104.00
.06	.00	5.90	.00	.000	.031	.000	.000	90.00	219.23
.002435	197.	200.	203.	0	19	0	.00	52.54	271.76

\*SECNO 1400.000

1400.000	7.58	98.08	96.18	.00	98.63	.54	.49	.00	104.50
1272.0	.0	1272.0	.0	.0	215.1	.0	6.9	1.7	104.50
.07	.00	5.91	.00	.000	.031	.000	.000	90.50	219.26
.002453	200.	200.	200.	0	19	0	.00	52.47	271.73

\*SECNO 1600.000

1600.000	7.57	98.57	96.68	.00	99.12	.55	.49	.00	105.00
1272.0	.0	1272.0	.0	.0	214.7	.0	7.9	1.9	105.00
.07	.00	5.92	.00	.000	.031	.000	.000	91.00	219.28
.002466	204.	200.	196.	0	19	0	.00	52.42	271.71

\*SECNO 1800.000

1800.000	7.57	99.07	97.18	.00	99.61	.55	.49	.00	105.50
1272.0	.0	1272.0	.0	.0	214.4	.0	8.8	2.2	105.50
.08	.00	5.93	.00	.000	.031	.000	.000	91.50	219.30
.002476	204.	200.	196.	0	19	0	.00	52.39	271.69

\*SECNO 2000.000

2000.000	7.57	99.57	97.66	.00	100.11	.54	.49	.00	106.00
1264.0	.0	1264.0	.0	.0	214.1	.0	9.8	2.4	106.00
.09	.00	5.90	.00	.000	.031	.000	.000	92.00	219.32
.002452	206.	200.	194.	0	19	0	.00	52.36	271.67

\*SECNO 2300.000

2300.000	7.81	100.31	98.16	.00	100.79	.48	.68	.01	106.50
1264.0	.0	1264.0	.0	.0	227.1	.0	11.4	2.8	106.50
.11	.00	5.57	.00	.000	.031	.000	.000	92.50	218.59
.002093	303.	300.	297.	0	19	0	.00	53.82	272.40

\*SECNO 2500.000

2500.000	7.83	100.83	98.21	.00	101.15	.32	.34	.02	107.00
1039.0	.0	1039.0	.0	.0	228.5	.0	12.4	3.0	107.00
.12	.00	4.55	.00	.000	.031	.000	.000	93.00	218.51
.001391	198.	200.	202.	2	19	0	.00	53.98	272.48

\*SECNO 2600.000

2600.000	7.44	100.94	98.73	.00	101.33	.39	.16	.02	107.50
1039.0	.0	1039.0	.0	.0	207.9	.0	12.9	3.1	107.50
.13	.00	5.00	.00	.000	.031	.000	.000	93.50	219.68
.001794	96.	100.	104.	2	14	0	.00	51.64	271.31

\*SECNO 2800.000

2800.000	7.30	101.30	99.21	.00	101.71	.42	.38	.01	108.00
1039.0	.0	1039.0	.0	.0	200.5	.0	13.8	3.4	108.00
.14	.00	5.18	.00	.000	.031	.000	.000	94.00	220.11
.001978	195.	200.	204.	2	19	0	.00	50.78	270.88

\*SECNO 3000.000

3000.000	7.19	101.69	99.71	.00	102.13	.44	.41	.01	108.50
1039.0	.0	1039.0	.0	.0	194.8	.0	14.7	3.6	108.50
.15	.00	5.33	.00	.000	.031	.000	.000	94.50	220.45
.002141	197.	200.	203.	1	19	0	.00	50.09	270.54

\*SECNO 3200.000

3200.000	7.12	102.12	100.21	.00	102.58	.46	.44	.01	109.00
1039.0	.0	1039.0	.0	.0	191.2	.0	15.6	3.8	109.00
.16	.00	5.44	.00	.000	.031	.000	.000	95.00	220.67
.002252	203.	200.	197.	1	19	0	.00	49.66	270.32

\*SECNO 3400.000

3400.000	7.07	102.57	100.71	.00	103.04	.47	.46	.00	109.50
1039.0	.0	1039.0	.0	.0	188.8	.0	16.5	4.1	109.50
.17	.00	5.50	.00	.000	.031	.000	.000	95.50	220.81
.002329	202.	200.	198.	1	19	0	.00	49.37	270.18

\*SECNO 3600.000

3600.000	7.03	103.03	101.21	.00	103.51	.48	.47	.00	110.00
1039.0	.0	1039.0	.0	.0	187.0	.0	17.4	4.3	110.00
.18	.00	5.56	.00	.000	.031	.000	.000	96.00	220.92
.002389	199.	200.	202.	0	19	0	.00	49.15	270.07

\*SECNO 3800.000

3800.000	7.01	103.51	101.71	.00	104.00	.48	.48	.00	110.50
1039.0	.0	1039.0	.0	.0	186.0	.0	18.2	4.5	110.50
.19	.00	5.59	.00	.000	.031	.000	.000	96.50	220.98
.002426	196.	200.	204.	0	19	0	.00	49.03	270.01

\*SECNO 4000.000

4000.000	7.00	104.00	102.21	.00	104.48	.49	.49	.00	111.00
1039.0	.0	1039.0	.0	.0	185.4	.0	19.1	4.7	111.00
.20	.00	5.61	.00	.000	.031	.000	.000	97.00	221.02
.002448	199.	200.	201.	0	19	0	.00	48.95	269.97

\*SECNO 4300.000

4300.000	7.23	104.73	102.71	.00	105.17	.43	.68	.01	111.50
1039.0	.0	1039.0	.0	.0	197.0	.0	20.4	5.1	111.50
.21	.00	5.27	.00	.000	.031	.000	.000	97.50	220.32
.002076	306.	300.	294.	0	19	0	.00	50.36	270.67

\*SECNO 4400.000

4400.000	7.01	105.01	102.89	.00	105.37	.36	.19	.01	112.00
893.0	.0	893.0	.0	.0	186.1	.0	20.8	5.2	112.00
.22	.00	4.80	.00	.000	.031	.000	.000	98.00	220.97
.001788	100.	100.	100.	2	19	0	.00	49.04	270.02

\*SECNO 4600.000  
 4600.000 6.87 105.37 103.39 .00 105.75 .39 .38 .01 112.50  
 893.0 .0 893.0 .0 .0 179.1 .0 21.7 5.4 112.50  
 .23 .00 4.98 .00 .000 .031 .000 .000 98.50 221.40  
 .001983 199. 200. 201. 2 19 0 .00 48.18 269.59

\*SECNO 4700.000  
 4700.000 6.81 105.56 103.64 .00 105.96 .40 .20 .00 112.75  
 893.0 .0 893.0 .0 .0 176.2 .0 22.1 5.5 112.75  
 .24 .00 5.07 .00 .000 .031 .000 .000 98.75 221.59  
 .002074 104. 100. 96. 0 19 0 .00 47.81 269.40

\*SECNO 4900.000  
 4900.000 6.72 105.97 104.14 .00 106.39 .42 .43 .01 113.25  
 893.0 .0 893.0 .0 .0 172.0 .0 22.9 5.7 113.25  
 .25 .00 5.19 .00 .000 .031 .000 .000 99.25 221.86  
 .002216 202. 200. 198. 0 19 0 .00 47.28 269.13

\*SECNO 5200.000  
 5200.000 6.66 106.66 104.56 .00 106.99 .33 .59 .01 114.00  
 779.0 .0 779.0 .0 .0 168.2 .0 24.0 6.1 114.00  
 .26 .00 4.63 .00 .000 .031 .000 .000 100.00 220.45  
 .001725 307. 300. 294. 2 19 0 .00 45.35 265.80

\*SECNO 5400.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = .63

5400.000 5.47 106.97 106.06 .00 107.60 .63 .52 .09 113.50  
 779.0 .0 779.0 .0 .0 122.8 .0 24.7 6.3 113.50  
 .27 .00 6.35 .00 .000 .031 .000 .000 101.50 221.21  
 .004395 206. 200. 194. 2 19 0 .00 42.05 263.26

\*SECNO 5600.000  
 5600.000 4.87 107.87 107.62 .00 108.91 1.04 1.19 .12 116.00  
 779.0 .0 779.0 .0 .0 95.2 .0 25.2 6.4 116.00  
 .28 .00 8.18 .00 .000 .031 .000 .000 103.00 224.37  
 .008453 200. 200. 200. 2 15 0 .00 36.25 260.62

\*SECNO 5800.000  
 5800.000 5.05 109.55 109.12 .00 110.46 .91 1.54 .01 117.50  
 779.0 .0 779.0 .0 .0 101.7 .0 25.7 6.6 117.50  
 .29 .00 7.66 .00 .000 .031 .000 .000 104.50 223.84  
 .007044 200. 200. 200. 3 11 0 .00 37.32 261.15

\*SECNO 6000.000  
 6000.000 4.98 110.98 110.62 .00 111.94 .96 1.46 .02 119.00  
 779.0 .0 779.0 .0 .0 99.0 .0 26.1 6.8 119.00  
 .29 .00 7.87 .00 .000 .031 .000 .000 106.00 224.06  
 .007596 195. 200. 205. 3 11 0 .00 36.87 260.93

\*SECNO 6200.000  
 6200.000 5.00 112.50 112.12 .00 113.45 .95 1.51 .00 120.50  
 779.0 .0 779.0 .0 .0 99.5 .0 26.6 7.0 120.50  
 .30 .00 7.83 .00 .000 .031 .000 .000 107.50 224.01  
 .007480 199. 200. 201. 2 11 0 .00 36.96 260.98

\*SECNO 6400.000  
 6400.000 5.00 114.00 113.62 .00 114.94 .95 1.49 .00 119.00  
 779.0 .0 779.0 .0 .0 99.8 .0 27.0 7.1 119.00

.31 .00 7.81 .00 .000 .031 .000 .000 109.00 215.00  
.007433 206. 200. 195. 1 11 0 .00 37.00 251.99

\*SECNO 6600.000

3301 HV CHANGED MORE THAN HVINS

6600.000 5.09 115.59 113.93 .00 115.80 .21 .78 .07 120.50  
377.0 .0 377.0 .0 .0 103.1 .0 27.5 7.3 120.50  
.32 .00 3.66 .00 .000 .031 .000 .000 110.50 214.73  
.001591 203. 200. 197. 2 19 0 .00 37.53 252.26

\*SECNO 6700.000

6700.000 4.47 115.72 114.68 .00 116.05 .34 .22 .04 121.25  
377.0 .0 377.0 .0 .0 80.9 .0 27.7 7.4 121.25  
.33 .00 4.66 .00 .000 .031 .000 .000 111.25 216.59  
.003094 101. 100. 99. 2 19 0 .00 33.81 250.40

\*SECNO 6800.000

6800.000 4.00 116.00 115.43 .00 116.51 .51 .40 .05 122.00  
377.0 .0 377.0 .0 .0 65.7 .0 27.9 7.5 122.00  
.33 .00 5.74 .00 .000 .031 .000 .000 112.00 218.00  
.005515 101. 100. 99. 2 19 0 .00 30.98 248.99

\*SECNO 6900.000

6900.000 3.80 116.55 116.18 .00 117.17 .62 .63 .03 122.75  
377.0 .0 377.0 .0 .0 59.7 .0 28.0 7.5 122.75  
.34 .00 6.32 .00 .000 .031 .000 .000 112.75 218.60  
.007195 100. 100. 100. 2 15 0 .00 29.80 248.39

\*SECNO 7000.000

3301 HV CHANGED MORE THAN HVINS

7000.000 3.95 117.45 115.57 .00 117.49 .04 .27 .06 123.50  
105.0 .0 105.0 .0 .0 64.2 .0 28.2 7.6 123.50  
.36 .00 1.64 .00 .000 .031 .000 .000 113.50 218.15  
.000456 100. 100. 100. 2 11 0 .00 30.69 248.84

\*SECNO 7100.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = .59

7100.000 3.24 117.49 116.32 .00 117.58 .09 .07 .01 124.25  
105.0 .0 105.0 .0 .0 44.0 .0 28.3 7.7 124.25  
.37 .00 2.39 .00 .000 .031 .000 .000 114.25 220.27  
.001315 100. 100. 100. 2 11 0 .00 26.44 246.72

\*SECNO 7200.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = .56

7200.000 2.62 117.62 117.07 .00 117.83 .21 .22 .03 125.00  
105.0 .0 105.0 .0 .0 28.9 .0 28.4 7.7 125.00  
.38 .00 3.64 .00 .000 .031 .000 .000 115.00 222.13  
.004167 100. 100. 100. 2 14 0 .00 22.01 244.13

\*SECNO 7300.000

7300.000 2.34 118.09 117.82 .00 118.41 .32 .55 .03 123.75  
105.0 .0 105.0 .0 .0 23.1 .0 28.4 7.8 123.75  
.38 .00 4.54 .00 .000 .031 .000 .000 115.75 216.95  
.007537 99. 100. 101. 2 15 0 .00 19.69 236.65

\*SECNO 7400.000

7400.000	2.35	118.85	118.57	.00	119.17	.32	.75	.00	124.50
105.0	.0	105.0	.0	.0	23.1	.0	28.5	7.8	124.50
.39	.00	4.55	.00	.000	.031	.000	.000	116.50	216.96
.007563	99.	100.	101.	2	15	0	.00	19.68	236.64

\*SECNO 7500.000

7500.000	2.35	119.60	119.32	.00	119.92	.32	.75	.00	125.25
105.0	.0	105.0	.0	.0	23.2	.0	28.5	7.8	125.25
.39	.00	4.53	.00	.000	.031	.000	.000	117.25	216.95
.007487	99.	100.	101.	0	15	0	.00	19.72	236.66

\*SECNO 7600.000

7600.000	2.35	120.35	120.07	.00	120.67	.32	.75	.00	126.00
105.0	.0	105.0	.0	.0	23.2	.0	28.6	7.9	126.00
.40	.00	4.52	.00	.000	.031	.000	.000	118.00	216.94
.007442	99.	100.	101.	0	15	0	.00	19.74	236.68

\*SECNO 7800.000

7800.000	2.35	121.85	121.57	.00	122.17	.32	1.50	.00	127.50
105.0	.0	105.0	.0	.0	23.1	.0	28.7	8.0	127.50
.41	.00	4.54	.00	.000	.031	.000	.000	119.50	216.96
.007546	196.	200.	204.	1	15	0	.00	19.69	236.65

\*SECNO 8000.000

8000.000	2.35	123.35	123.07	.00	123.67	.32	1.50	.00	129.00
105.0	.0	105.0	.0	.0	23.2	.0	28.8	8.1	129.00
.42	.00	4.53	.00	.000	.031	.000	.000	121.00	216.94
.007474	199.	200.	201.	1	15	0	.00	19.72	236.67

\*SECNO 8200.000

8200.000	2.35	124.85	124.57	.00	125.17	.32	1.50	.00	130.50
105.0	.0	105.0	.0	.0	23.2	.0	28.9	8.2	130.50
.44	.00	4.53	.00	.000	.031	.000	.000	122.50	216.95
.007507	193.	200.	204.	1	15	0	.00	19.71	236.66

\*SECNO 8400.000

8400.000	2.28	126.28	125.91	.00	126.52	.24	1.35	.01	132.00
86.0	.0	86.0	.0	.0	21.8	.0	29.0	8.3	132.00
.45	.00	3.95	.00	.000	.031	.000	.000	124.00	217.17
.005947	192.	200.	203.	1	15	0	.00	19.10	236.27

\*SECNO 8600.000

8600.000	2.13	127.63	127.41	.00	127.95	.31	1.40	.02	133.50
86.0	.0	86.0	.0	.0	19.1	.0	29.1	8.3	133.50
.46	.00	4.50	.00	.000	.031	.000	.000	125.50	217.60
.008407	210.	200.	190.	1	15	0	.00	17.90	235.50

\*SECNO 8800.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = 1.62

8800.000	1.82	128.82	128.40	.00	128.95	.13	.98	.02	128.00
86.0	8.0	78.0	.0	5.3	26.3	.0	29.2	8.5	135.00
.48	1.51	2.97	.00	.031	.031	.000	.000	127.00	198.15
.003217	198.	200.	202.	2	14	0	.00	35.70	233.85

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HEC-2 WATER SURFACE PROFILES

Version 4.6.2; May 1991

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NOTE- ASTERISK (\*) AT LEFT OF CROSS-SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST  
NO SEDIMENT IN CHANNEL.

SUMMARY PRINTOUT

	SECNO	Q	CWSEL	ELMIN	DEPTH	TOPWID	AREA	VCH	FRCH
Bebw H	.000	1271.00	94.55	87.00	7.55	52.29	213.55	5.95	.52
	200.000	1271.00	95.05	87.50	7.55	52.29	213.53	5.95	.52
	400.000	1271.00	95.55	88.00	7.55	52.29	213.53	5.95	.52
	600.000	1271.00	96.05	88.50	7.55	52.28	213.49	5.95	.52
	700.000	1271.00	96.26	89.00	7.26	50.58	198.87	6.39	.57
	800.000	1271.00	96.65	89.00	7.65	52.90	218.90	5.81	.50
G-H	1000.000	1272.00	97.12	89.50	7.62	52.67	216.86	5.87	.51
	1200.000	1272.00	97.60	90.00	7.60	52.54	215.69	5.90	.51
	1400.000	1272.00	98.08	90.50	7.58	52.47	215.12	5.91	.51
	1600.000	1272.00	98.57	91.00	7.57	52.42	214.69	5.92	.52
	1800.000	1272.00	99.07	91.50	7.57	52.39	214.37	5.93	.52
	2000.000	1264.00	99.57	92.00	7.57	52.36	214.12	5.90	.51
	2300.000	1264.00	100.31	92.50	7.81	53.82	227.06	5.57	.48
E-F	2500.000	1039.00	100.83	93.00	7.83	53.98	228.47	4.55	.39
	2600.000	1039.00	100.94	93.50	7.44	51.64	207.91	5.00	.44
	2800.000	1039.00	101.30	94.00	7.30	50.78	200.54	5.18	.46
	3000.000	1039.00	101.69	94.50	7.19	50.09	194.76	5.33	.48
	3200.000	1039.00	102.12	95.00	7.12	49.66	191.16	5.44	.49
	3400.000	1039.00	102.57	95.50	7.07	49.37	188.80	5.50	.50
	3600.000	1039.00	103.03	96.00	7.03	49.15	187.02	5.56	.50
	3800.000	1039.00	103.51	96.50	7.01	49.03	185.98	5.59	.51
	4000.000	1039.00	104.00	97.00	7.00	48.95	185.36	5.61	.51
	4300.000	1039.00	104.73	97.50	7.23	50.36	197.01	5.27	.47
D-E	4400.000	893.00	105.01	98.00	7.01	49.04	186.13	4.80	.43
	4600.000	893.00	105.37	98.50	6.87	48.18	179.15	4.98	.46
	4700.000	893.00	105.56	98.75	6.81	47.81	176.20	5.07	.47
	4900.000	893.00	105.97	99.25	6.72	47.28	171.95	5.19	.48
	5200.000	779.00	106.66	100.00	6.66	45.35	168.23	4.63	.42

\* 5400.000 779.00 106.97 101.50 5.47 42.05 122.77 6.35 .65

5600.000 779.00 107.87 103.00 4.87 36.25 95.20 8.18 .89

5800.000 779.00 109.55 104.50 5.05 37.32 101.74 7.66 .82

6000.000 779.00 110.98 106.00 4.98 36.87 98.98 7.87 .85

6200.000 779.00 112.50 107.50 5.00 36.96 99.54 7.83 .84

6400.000 779.00 114.00 109.00 5.00 37.00 99.76 7.81 .84

6600.000 377.00 115.59 110.50 5.09 37.53 103.09 3.66 .39

6700.000 377.00 115.72 111.25 4.47 33.81 80.94 4.66 .53

6800.000 377.00 116.00 112.00 4.00 30.98 65.67 5.74 .69

6900.000 377.00 116.55 112.75 3.80 29.80 59.69 6.32 .79

7000.000 105.00 117.45 113.50 3.95 30.69 64.17 1.64 .20

\* 7100.000 105.00 117.49 114.25 3.24 26.44 43.96 2.39 .33

\* 7200.000 105.00 117.62 115.00 2.62 22.01 28.88 3.64 .56

7300.000 105.00 118.09 115.75 2.34 19.69 23.12 4.54 .74

7400.000 105.00 118.85 116.50 2.35 19.68 23.10 4.55 .74

7500.000 105.00 119.60 117.25 2.35 19.72 23.18 4.53 .74

7600.000 105.00 120.35 118.00 2.35 19.74 23.23 4.52 .73

7800.000 105.00 121.85 119.50 2.35 19.69 23.11 4.54 .74

8000.000 105.00 123.35 121.00 2.35 19.72 23.20 4.53 .74

8200.000 105.00 124.85 122.50 2.35 19.71 23.16 4.53 .74

8400.000 86.00 126.28 124.00 2.28 19.10 21.76 3.95 .65

8600.000 86.00 127.63 125.50 2.13 17.90 19.11 4.50 .77

\* 8800.000 86.00 128.82 127.00 1.82 35.70 31.61 2.97 .49

C-D

B-C

A-B

ABOVE  
A

SUMMARY OF ERRORS AND SPECIAL NOTES

WARNING SECNO= 5400.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

WARNING SECNO= 7100.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

WARNING SECNO= 7200.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

WARNING SECNO= 8800.000 PROFILE= 1 CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE



**RIPRAP SIZING CALCULATIONS FOR 100-YR, 1,000-YR, AND 10,000-YR EVENTS**

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_2.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACHS F-G & G-H (100 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 370  
INPUT: topwidth of flow (ft)= 32.99  
R/W= 11.2  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 4.080  
INPUT: depth (ft)= 4.340

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.16667	2.00	0.156	0.156	0.749	0.654	4.80	4.19
0.08333	1.00	0.124	0.124	0.374	0.327	3.01	2.63
0.0625	0.75	0.114	0.114	0.281	0.245	2.46	2.15
0.04167	0.50	0.101	0.101	0.187	0.164	1.85	1.61
0.02083	0.25	0.084	0.084	0.094	0.082	1.11	0.97
0.01042	0.13	0.071	0.071	0.047	0.041	0.66	0.57

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

file: P:\03-317\RESP1299\RIPRAP\COE70\_2.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACHS F-G & G-H (100 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 293  
INPUT: topwidth of flow (ft)= 32.99  
R/W= 8.9  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 4.080  
INPUT: depth (ft)= 4.340

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.16667	2.00	0.156	0.165	0.749	0.654	4.54	3.97
0.08333	1.00	0.124	0.131	0.374	0.327	2.85	2.49
0.0625	0.75	0.114	0.121	0.281	0.245	2.33	2.04
0.04167	0.50	0.101	0.107	0.187	0.164	1.75	1.53
0.02083	0.25	0.084	0.089	0.094	0.082	1.05	0.92
0.01042	0.13	0.071	0.075	0.047	0.041	0.62	0.54

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_2.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
BELOW REACH H (100 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 705  
INPUT: topwidth of flow (ft)= 31.48  
R/W= 22.4  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 4.570  
INPUT: depth (ft)= 4.080

	D-50	BOUNDARY	BEND	BOTTOM	SIDE SLOPE		
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	BOTTOM	SIDE SLOPE
		(psf)	(psf)	(psf)	(psf)	SF	SF
0.16667	2.00	0.200	0.200	0.749	0.654	3.74	3.27
0.08333	1.00	0.159	0.159	0.374	0.327	2.35	2.06
0.0625	0.75	0.146	0.146	0.281	0.245	1.93	1.68
0.04167	0.50	0.129	0.129	0.187	0.164	1.45	1.26
0.02083	0.25	0.107	0.107	0.094	0.082	0.87	0.76
0.01042	0.13	0.091	0.091	0.047	0.041	0.52	0.45

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_2.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH E-F (100 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 385  
INPUT: topwidth of flow (ft)= 30.98  
R/W= 12.4  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 3.870  
INPUT: depth (ft)= 4.000

	D-50	BOUNDARY	BEND	BOTTOM	SIDE SLOPE		
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	BOTTOM	SIDE SLOPE
		(psf)	(psf)	(psf)	(psf)	SF	SF
0.16667	2.00	0.145	0.145	0.749	0.654	5.18	4.53
0.08333	1.00	0.115	0.115	0.374	0.327	3.26	2.85
0.0625	0.75	0.105	0.105	0.281	0.245	2.67	2.33
0.04167	0.50	0.093	0.093	0.187	0.164	2.00	1.75
0.02083	0.25	0.077	0.077	0.094	0.082	1.21	1.06
0.01042	0.13	0.065	0.065	0.047	0.041	0.72	0.63

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_2.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH E-F (100 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 357  
INPUT: topwidth of flow (ft)= 30.98  
R/W= 11.5  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 3.870  
INPUT: depth (ft)= 4.000

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.16667	2.00	0.145	0.145	0.749	0.654	5.18	4.53
0.08333	1.00	0.115	0.115	0.374	0.327	3.26	2.85
0.0625	0.75	0.105	0.105	0.281	0.245	2.67	2.33
0.04167	0.50	0.093	0.093	0.187	0.164	2.00	1.75
0.02083	0.25	0.077	0.077	0.094	0.082	1.21	1.06
0.01042	0.13	0.065	0.065	0.047	0.041	0.72	0.63

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_2.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH E-F (100 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 317  
INPUT: topwidth of flow (ft)= 30.98  
R/W= 10.2  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 3.870  
INPUT: depth (ft)= 4.000

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.16667	2.00	0.145	0.145	0.749	0.654	5.18	4.53
0.08333	1.00	0.115	0.115	0.374	0.327	3.26	2.85
0.0625	0.75	0.105	0.105	0.281	0.245	2.67	2.33
0.04167	0.50	0.093	0.093	0.187	0.164	2.00	1.75
0.02083	0.25	0.077	0.077	0.094	0.082	1.21	1.06
0.01042	0.13	0.065	0.065	0.047	0.041	0.72	0.63

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_2.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH D-E (100 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 260  
INPUT: topwidth of flow (ft)= 29.76  
R/W= 8.7  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 3.680  
INPUT: depth (ft)= 3.800

	D-50	BOUNDARY	BEND	BOTTOM	SIDE SLOPE		
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	BOTTOM	SIDE SLOPE
		(psf)	(psf)	(psf)	(psf)	SF	SF
0.16667	2.00	0.133	0.142	0.749	0.654	5.28	4.61
0.08333	1.00	0.106	0.112	0.374	0.327	3.33	2.91
0.0625	0.75	0.097	0.103	0.281	0.245	2.73	2.39
0.04167	0.50	0.086	0.091	0.187	0.164	2.05	1.79
0.02083	0.25	0.071	0.076	0.094	0.082	1.24	1.08
0.01042	0.13	0.060	0.064	0.047	0.041	0.74	0.64



RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_2.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH D-E (100 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 484  
INPUT: topwidth of flow (ft)= 29.76  
R/W= 16.3  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 3.680  
INPUT: depth (ft)= 3.800

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.16667	2.00	0.133	0.133	0.749	0.654	5.63	4.92
0.08333	1.00	0.106	0.106	0.374	0.327	3.55	3.10
0.0625	0.75	0.097	0.097	0.281	0.245	2.91	2.54
0.04167	0.50	0.086	0.086	0.187	0.164	2.18	1.91
0.02083	0.25	0.071	0.071	0.094	0.082	1.32	1.15
0.01042	0.13	0.060	0.060	0.047	0.041	0.78	0.68

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_2.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH C-D (100 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 260  
INPUT: topwidth of flow (ft)= 23.25  
R/W= 11.2  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 6.100  
INPUT: depth (ft)= 2.690

D-50		BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM	SIDE SLOPE
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	SF	SF
		(psf)	(psf)	(psf)	(psf)		
0.25	3.00	0.487	0.487	1.123	0.982	2.31	2.02
0.16667	2.00	0.415	0.415	0.749	0.654	1.80	1.58
0.08333	1.00	0.324	0.324	0.374	0.327	1.15	1.01
0.0625	0.75	0.295	0.295	0.281	0.245	0.95	0.83
0.04167	0.50	0.260	0.260	0.187	0.164	0.72	0.63
0.02083	0.25	0.214	0.214	0.094	0.082	0.44	0.38

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_2.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH B-C (100 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 367  
INPUT: topwidth of flow (ft)= 18.77  
R/W= 19.6  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 4.430  
INPUT: depth (ft)= 2.240

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.16667	2.00	0.235	0.235	0.749	0.654	3.19	2.79
0.08333	1.00	0.182	0.182	0.374	0.327	2.06	1.80
0.0625	0.75	0.165	0.165	0.281	0.245	1.70	1.49
0.04167	0.50	0.145	0.145	0.187	0.164	1.29	1.13
0.02083	0.25	0.119	0.119	0.094	0.082	0.79	0.69
0.01042	0.13	0.099	0.099	0.047	0.041	0.47	0.41

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_2.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH A-B (100 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 222  
INPUT: topwidth of flow (ft)= 11.36  
R/W= 19.5  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 3.380  
INPUT: depth (ft)= 1.350

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.16667	2.00	0.169	0.169	0.749	0.654	4.44	3.88
0.08333	1.00	0.127	0.127	0.374	0.327	2.94	2.57
0.0625	0.75	0.114	0.114	0.281	0.245	2.45	2.14
0.04167	0.50	0.099	0.099	0.187	0.164	1.88	1.64
0.02083	0.25	0.080	0.080	0.094	0.082	1.17	1.02
0.01042	0.13	0.066	0.066	0.047	0.041	0.71	0.62

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_2.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
ABOVE REACH A (100 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 714  
INPUT: topwidth of flow (ft)= 10.58  
R/W= 67.5  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 3.140  
INPUT: depth (ft)= 1.260

	D-50	BOUNDARY	BEND	BOTTOM	SIDE SLOPE		
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	BOTTOM	SIDE SLOPE
		(psf)	(psf)	(psf)	(psf)	SF	SF
0.16667	2.00	0.150	0.150	0.749	0.654	4.99	4.36
0.08333	1.00	0.113	0.113	0.374	0.327	3.32	2.90
0.0625	0.75	0.101	0.101	0.281	0.245	2.77	2.42
0.04167	0.50	0.088	0.088	0.187	0.164	2.13	1.86
0.02083	0.25	0.070	0.070	0.094	0.082	1.33	1.16
0.01042	0.13	0.058	0.058	0.047	0.041	0.81	0.71

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_3.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
ABOVE REACH A (1,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 714  
INPUT: topwidth of flow (ft)= 14.30  
R/W= 49.9  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 3.860  
INPUT: depth (ft)= 1.700

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.25	3.00	0.238	0.238	1.123	0.982	4.73	4.13
0.16667	2.00	0.199	0.199	0.749	0.654	3.76	3.28
0.08333	1.00	0.152	0.152	0.374	0.327	2.46	2.15
0.0625	0.75	0.138	0.138	0.281	0.245	2.04	1.78
0.04167	0.50	0.120	0.120	0.187	0.164	1.56	1.36
0.02083	0.25	0.097	0.097	0.094	0.082	0.96	0.84

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_3.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH A-B (1,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 222  
INPUT: topwidth of flow (ft)= 15.73  
R/W= 14.1  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 3.930  
INPUT: depth (ft)= 1.870

	D-50	BOUNDARY	BEND	BOTTOM	SIDE SLOPE		
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	BOTTOM	SIDE SLOPE
		(psf)	(psf)	(psf)	(psf)	SF	SF
0.25	3.00	0.236	0.236	1.123	0.982	4.76	4.16
0.16667	2.00	0.199	0.199	0.749	0.654	3.77	3.29
0.08333	1.00	0.153	0.153	0.374	0.327	2.45	2.14
0.0625	0.75	0.138	0.138	0.281	0.245	2.03	1.78
0.04167	0.50	0.121	0.121	0.187	0.164	1.55	1.35
0.02083	0.25	0.098	0.098	0.094	0.082	0.95	0.83

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_3.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH B-C (1,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 367  
INPUT: topwidth of flow (ft)= 25.16  
R/W= 14.6  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 5.410  
INPUT: depth (ft)= 3.030

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.25	3.00	0.365	0.365	1.123	0.982	3.08	2.69
0.16667	2.00	0.312	0.312	0.749	0.654	2.40	2.10
0.08333	1.00	0.245	0.245	0.374	0.327	1.53	1.33
0.0625	0.75	0.224	0.224	0.281	0.245	1.26	1.10
0.04167	0.50	0.198	0.198	0.187	0.164	0.95	0.83
0.02083	0.25	0.163	0.163	0.094	0.082	0.57	0.50



RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_3.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH C-D (1,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 260  
INPUT: topwidth of flow (ft)= 30.28  
R/W= 8.6  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 6.890  
INPUT: depth (ft)= 3.880

D-50		BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM	SIDE SLOPE
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	SF	SF
		(psf)	(psf)	(psf)	(psf)		
0.25	3.00	0.537	0.578	1.123	0.982	1.94	1.70
0.16667	2.00	0.463	0.498	0.749	0.654	1.50	1.31
0.08333	1.00	0.367	0.395	0.374	0.327	0.95	0.83
0.0625	0.75	0.336	0.361	0.281	0.245	0.78	0.68
0.04167	0.50	0.299	0.321	0.187	0.164	0.58	0.51
0.02083	0.25	0.247	0.266	0.094	0.082	0.35	0.31

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_3.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH D-E (1,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 484  
INPUT: topwidth of flow (ft)= 38.40  
R/W= 12.6  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 4.510  
INPUT: depth (ft)= 5.240

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.25	3.00	0.206	0.206	1.123	0.982	5.45	4.76
0.16667	2.00	0.179	0.179	0.749	0.654	4.19	3.66
0.08333	1.00	0.143	0.143	0.374	0.327	2.61	2.28
0.0625	0.75	0.132	0.132	0.281	0.245	2.13	1.86
0.04167	0.50	0.118	0.118	0.187	0.164	1.59	1.39
0.02083	0.25	0.098	0.098	0.094	0.082	0.95	0.83

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_3.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH D-E (1,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 260  
INPUT: topwidth of flow (ft)= 38.40  
R/W= 6.8  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 4.510  
INPUT: depth (ft)= 5.240

D-50		BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM	SIDE SLOPE
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	SF	SF
		(psf)	(psf)	(psf)	(psf)		
0.25	3.00	0.206	0.249	1.123	0.982	4.50	3.94
0.16667	2.00	0.179	0.217	0.749	0.654	3.46	3.02
0.08333	1.00	0.143	0.174	0.374	0.327	2.16	1.88
0.0625	0.75	0.132	0.160	0.281	0.245	1.76	1.54
0.04167	0.50	0.118	0.142	0.187	0.164	1.31	1.15
0.02083	0.25	0.098	0.119	0.094	0.082	0.79	0.69

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_3.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH E-F (1,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 317  
INPUT: topwidth of flow (ft)= 39.93  
R/W= 7.9  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 4.810  
INPUT: depth (ft)= 5.490

	D-50	BOUNDARY	BEND	BOTTOM	SIDE SLOPE		
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	BOTTOM	SIDE SLOPE
		(psf)	(psf)	(psf)	(psf)	SF	SF
0.25	3.00	0.230	0.258	1.123	0.982	4.36	3.81
0.16667	2.00	0.200	0.224	0.749	0.654	3.34	2.92
0.08333	1.00	0.161	0.180	0.374	0.327	2.08	1.82
0.0625	0.75	0.148	0.165	0.281	0.245	1.70	1.48
0.04167	0.50	0.132	0.148	0.187	0.164	1.27	1.11
0.02083	0.25	0.110	0.123	0.094	0.082	0.76	0.66

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_3.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH E-F (1,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 357  
INPUT: topwidth of flow (ft)= 39.93  
R/W= 8.9  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 4.810  
INPUT: depth (ft)= 5.490

D-50		BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM	SIDE SLOPE
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	SF	SF
		(psf)	(psf)	(psf)	(psf)		
0.25	3.00	0.230	0.243	1.123	0.982	4.63	4.04
0.16667	2.00	0.200	0.211	0.749	0.654	3.55	3.10
0.08333	1.00	0.161	0.170	0.374	0.327	2.21	1.93
0.0625	0.75	0.148	0.156	0.281	0.245	1.80	1.57
0.04167	0.50	0.132	0.139	0.187	0.164	1.34	1.18
0.02083	0.25	0.110	0.116	0.094	0.082	0.80	0.70

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_3.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH E-F (1,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 385  
INPUT: topwidth of flow (ft)= 39.93  
R/W= 9.6  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 4.810  
INPUT: depth (ft)= 5.490

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.25	3.00	0.230	0.234	1.123	0.982	4.80	4.20
0.16667	2.00	0.200	0.203	0.749	0.654	3.68	3.22
0.08333	1.00	0.161	0.163	0.374	0.327	2.29	2.00
0.0625	0.75	0.148	0.150	0.281	0.245	1.87	1.63
0.04167	0.50	0.132	0.134	0.187	0.164	1.40	1.22
0.02083	0.25	0.110	0.112	0.094	0.082	0.84	0.73

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_3.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACHS F-G & G-H (1,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 293  
INPUT: topwidth of flow (ft)= 42.71  
R/W= 6.9  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 5.070  
INPUT: depth (ft)= 5.950

	D-50	BOUNDARY	BEND	BOTTOM	SIDE SLOPE		
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	BOTTOM	SIDE SLOPE
		(psf)	(psf)	(psf)	(psf)	SF	SF
0.25	3.00	0.249	0.299	1.123	0.982	3.75	3.28
0.16667	2.00	0.217	0.261	0.749	0.654	2.87	2.51
0.08333	1.00	0.175	0.210	0.374	0.327	1.78	1.56
0.0625	0.75	0.161	0.193	0.281	0.245	1.45	1.27
0.04167	0.50	0.144	0.173	0.187	0.164	1.08	0.95
0.02083	0.25	0.120	0.145	0.094	0.082	0.65	0.57

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_3.WB3  
 Date: 12/14/99  
 Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
 REACHS F-G & G-H (1,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 370  
 INPUT: topwidth of flow (ft)= 42.71  
 R/W= 8.7  
 INPUT: side slope (xH:1V)= 3  
 side slope angle (deg)= 18.4  
 INPUT: angle of repose (deg)= 40.5  
 INPUT: rock specific gravity= 2.8  
  
 INPUT: velocity (fps)= 5.070  
 INPUT: depth (ft)= 5.950

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.25	3.00	0.249	0.266	1.123	0.982	4.22	3.69
0.16667	2.00	0.217	0.232	0.749	0.654	3.23	2.82
0.08333	1.00	0.175	0.187	0.374	0.327	2.00	1.75
0.0625	0.75	0.161	0.172	0.281	0.245	1.63	1.43
0.04167	0.50	0.144	0.154	0.187	0.164	1.22	1.06
0.02083	0.25	0.120	0.129	0.094	0.082	0.73	0.64



RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_3.WB3  
Date: 12/14/99  
Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
BELOW REACH H (1,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 705  
INPUT: topwidth of flow (ft)= 41.03  
R/W= 17.2  
INPUT: side slope (xH:1V)= 3  
side slope angle (deg)= 18.4  
INPUT: angle of repose (deg)= 40.5  
INPUT: rock specific gravity= 2.8  
  
INPUT: velocity (fps)= 5.540  
INPUT: depth (ft)= 5.670

D-50 (ft)	BOUNDARY SHEAR (psf)	BEND SHEAR (psf)	BOTTOM SHEAR (psf)	SIDE SLOPE SHEAR (psf)	BOTTOM SF	SIDE SLOPE SF
0.25	0.302	0.302	1.123	0.982	3.72	3.25
0.16667	0.263	0.263	0.749	0.654	2.85	2.49
0.08333	0.211	0.211	0.374	0.327	1.77	1.55
0.0625	0.194	0.194	0.281	0.245	1.44	1.26
0.04167	0.174	0.174	0.187	0.164	1.08	0.94
0.02083	0.145	0.145	0.094	0.082	0.64	0.56

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_4.WB3

Date: 12/14/99

Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
ABOVE REACH A (10,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 714  
 INPUT: topwidth of flow (ft)= 17.90  
 R/W= 39.9  
 INPUT: side slope (xH:1V)= 3  
 side slope angle (deg)= 18.4  
 INPUT: angle of repose (deg)= 40.5  
 INPUT: rock specific gravity= 2.8  
  
 INPUT: velocity (fps)= 4.500  
 INPUT: depth (ft)= 2.130

	D-50	BOUNDARY	BEND	BOTTOM	SIDE SLOPE		
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	BOTTOM	SIDE SLOPE
		(psf)	(psf)	(psf)	(psf)	SF	SF
0.25	3.00	0.292	0.292	1.123	0.982	3.84	3.36
0.16667	2.00	0.247	0.247	0.749	0.654	3.03	2.65
0.08333	1.00	0.191	0.191	0.374	0.327	1.96	1.71
0.0625	0.75	0.173	0.173	0.281	0.245	1.62	1.42
0.04167	0.50	0.152	0.152	0.187	0.164	1.23	1.07
0.02083	0.25	0.124	0.124	0.094	0.082	0.75	0.66

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_4.WB3

Date: 12/14/99

Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH A-B (10,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 222  
 INPUT: topwidth of flow (ft)= 19.68  
 R/W= 11.3  
 INPUT: side slope (xH:1V)= 3  
 side slope angle (deg)= 18.4  
 INPUT: angle of repose (deg)= 40.5  
 INPUT: rock specific gravity= 2.8  
  
 INPUT: velocity (fps)= 4.550  
 INPUT: depth (ft)= 2.350

	D-50	BOUNDARY	BEND	BOTTOM	SIDE SLOPE		
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	BOTTOM	SIDE SLOPE
		(psf)	(psf)	(psf)	(psf)	SF	SF
0.25	3.00	0.287	0.287	1.123	0.982	3.92	3.43
0.16667	2.00	0.243	0.243	0.749	0.654	3.08	2.69
0.08333	1.00	0.189	0.189	0.374	0.327	1.98	1.73
0.0625	0.75	0.172	0.172	0.281	0.245	1.64	1.43
0.04167	0.50	0.151	0.151	0.187	0.164	1.24	1.08
0.02083	0.25	0.123	0.123	0.094	0.082	0.76	0.66

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_4.WB3

Date: 12/14/99

Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH B-C (10,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 367  
 INPUT: topwidth of flow (ft)= 29.80  
 R/W= 12.3  
 INPUT: side slope (xH:1V)= 3  
 side slope angle (deg)= 18.4  
 INPUT: angle of repose (deg)= 40.5  
 INPUT: rock specific gravity= 2.8

INPUT: velocity (fps)= 6.320  
 INPUT: depth (ft)= 3.800

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.25	3.00	0.456	0.456	1.123	0.982	2.46	2.15
0.16667	2.00	0.393	0.393	0.749	0.654	1.91	1.67
0.08333	1.00	0.311	0.311	0.374	0.327	1.20	1.05
0.0625	0.75	0.285	0.285	0.281	0.245	0.99	0.86
0.04167	0.50	0.253	0.253	0.187	0.164	0.74	0.65
0.02083	0.25	0.209	0.209	0.094	0.082	0.45	0.39

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_4.WB3

Date: 12/14/99

Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH C-D (10,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 260  
 INPUT: topwidth of flow (ft)= 36.25  
 R/W= 7.2  
 INPUT: side slope (xH:1V)= 3  
 side slope angle (deg)= 18.4  
 INPUT: angle of repose (deg)= 40.5  
 INPUT: rock specific gravity= 2.8  
  
 INPUT: velocity (fps)= 8.180  
 INPUT: depth (ft)= 4.870

	D-50		BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM	SIDE SLOPE
	(ft)	(in)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.25	3.00	0.696	0.819	1.123	0.982	1.37	1.20	
0.22917	2.75	0.674	0.793	1.030	0.900	1.30	1.13	
0.20833	2.50	0.652	0.767	0.936	0.818	1.22	1.07	
0.1875	2.25	0.628	0.739	0.842	0.736	1.14	1.00	
0.16667	2.00	0.603	0.710	0.749	0.654	1.06	0.92	
0.14583	1.75	0.577	0.678	0.655	0.573	0.97	0.84	

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_4.WB3

Date: 12/14/99

Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH D-E (10,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 484  
 INPUT: topwidth of flow (ft)= 47.28  
 R/W= 10.2  
 INPUT: side slope (xH:1V)= 3  
 side slope angle (deg)= 18.4  
 INPUT: angle of repose (deg)= 40.5  
 INPUT: rock specific gravity= 2.8  
  
 INPUT: velocity (fps)= 5.190  
 INPUT: depth (ft)= 6.720

	D-50	BOUNDARY	BEND	BOTTOM	SIDE SLOPE		
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	BOTTOM	SIDE SLOPE
		(psf)	(psf)	(psf)	(psf)	SF	SF
0.25	3.00	0.250	0.250	1.123	0.982	4.49	3.93
0.16667	2.00	0.218	0.218	0.749	0.654	3.43	3.00
0.08333	1.00	0.177	0.177	0.374	0.327	2.12	1.85
0.0625	0.75	0.163	0.163	0.281	0.245	1.73	1.51
0.04167	0.50	0.146	0.146	0.187	0.164	1.28	1.12
0.02083	0.25	0.122	0.122	0.094	0.082	0.76	0.67

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,

EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_4.WB3

Date: 12/14/99

Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH D-E (10,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 260  
 INPUT: topwidth of flow (ft)= 47.28  
 R/W= 5.5  
 INPUT: side slope (xH:1V)= 3  
 side slope angle (deg)= 18.4  
 INPUT: angle of repose (deg)= 40.5  
 INPUT: rock specific gravity= 2.8  
  
 INPUT: velocity (fps)= 5.190  
 INPUT: depth (ft)= 6.720

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.25	3.00	0.250	0.336	1.123	0.982	3.35	2.92
0.16667	2.00	0.218	0.293	0.749	0.654	2.55	2.23
0.08333	1.00	0.177	0.237	0.374	0.327	1.58	1.38
0.0625	0.75	0.163	0.219	0.281	0.245	1.28	1.12
0.04167	0.50	0.146	0.196	0.187	0.164	0.96	0.84
0.02083	0.25	0.122	0.164	0.094	0.082	0.57	0.50

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_4.WB3

Date: 12/14/99

Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH E-F (10,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 317  
 INPUT: topwidth of flow (ft)= 48.95  
 R/W= 6.5  
 INPUT: side slope (xH:1V)= 3  
 side slope angle (deg)= 18.4  
 INPUT: angle of repose (deg)= 40.5  
 INPUT: rock specific gravity= 2.8  
  
 INPUT: velocity (fps)= 5.610  
 INPUT: depth (ft)= 7.000

	D-50	BOUNDARY	BEND	BOTTOM	SIDE SLOPE		
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	BOTTOM	SIDE SLOPE
		(psf)	(psf)	(psf)	(psf)	SF	SF
0.25	3.00	0.288	0.356	1.123	0.982	3.15	2.75
0.16667	2.00	0.252	0.312	0.749	0.654	2.40	2.10
0.08333	1.00	0.204	0.252	0.374	0.327	1.48	1.30
0.0625	0.75	0.188	0.233	0.281	0.245	1.21	1.05
0.04167	0.50	0.168	0.209	0.187	0.164	0.90	0.78
0.02083	0.25	0.142	0.175	0.094	0.082	0.53	0.47



RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

File: P:\03-317\RESP1299\RIPRAP\COE70\_4.WB3

Date: 12/14/99

Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH E-F (10,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 357  
 INPUT: topwidth of flow (ft)= 48.95  
 R/W= 7.3  
 INPUT: side slope (xH:1V)= 3  
 side slope angle (deg)= 18.4  
 INPUT: angle of repose (deg)= 40.5  
 INPUT: rock specific gravity= 2.8  
  
 INPUT: velocity (fps)= 5.610  
 INPUT: depth (ft)= 7.000

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.25	3.00	0.288	0.336	1.123	0.982	3.34	2.92
0.16667	2.00	0.252	0.294	0.749	0.654	2.55	2.23
0.08333	1.00	0.204	0.238	0.374	0.327	1.57	1.38
0.0625	0.75	0.188	0.219	0.281	0.245	1.28	1.12
0.04167	0.50	0.168	0.197	0.187	0.164	0.95	0.83
0.02083	0.25	0.142	0.165	0.094	0.082	0.57	0.50

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

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Date: 12/14/99

Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACH E-F (10,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 385  
 INPUT: topwidth of flow (ft)= 48.95  
 R/W= 7.9  
 INPUT: side slope (xH:1V)= 3  
 side slope angle (deg)= 18.4  
 INPUT: angle of repose (deg)= 40.5  
 INPUT: rock specific gravity= 2.8

INPUT: velocity (fps)= 5.610  
 INPUT: depth (ft)= 7.000

D-50 (ft)	D-50 (in)	BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM SF	SIDE SLOPE SF
		SHEAR (psf)	SHEAR (psf)	SHEAR (psf)	SHEAR (psf)		
0.25	3.00	0.288	0.323	1.123	0.982	3.47	3.04
0.16667	2.00	0.252	0.283	0.749	0.654	2.65	2.31
0.08333	1.00	0.204	0.229	0.374	0.327	1.64	1.43
0.0625	0.75	0.188	0.211	0.281	0.245	1.33	1.16
0.04167	0.50	0.168	0.189	0.187	0.164	0.99	0.86
0.02083	0.25	0.142	0.159	0.094	0.082	0.59	0.51

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

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Date: 12/14/99

Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACHS F-G & G-H (10,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 293  
 INPUT: topwidth of flow (ft)= 52.39  
 R/W= 5.6  
 INPUT: side slope (xH:1V)= 3  
 side slope angle (deg)= 18.4  
 INPUT: angle of repose (deg)= 40.5  
 INPUT: rock specific gravity= 2.8  
  
 INPUT: velocity (fps)= 5.930  
 INPUT: depth (ft)= 7.570

	D-50	BOUNDARY	BEND	BOTTOM	SIDE SLOPE		
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	BOTTOM	SIDE SLOPE
		(psf)	(psf)	(psf)	(psf)	SF	SF
0.25	3.00	0.313	0.417	1.123	0.982	2.69	2.35
0.16667	2.00	0.274	0.365	0.749	0.654	2.05	1.79
0.08333	1.00	0.223	0.297	0.374	0.327	1.26	1.10
0.0625	0.75	0.206	0.274	0.281	0.245	1.03	0.90
0.04167	0.50	0.184	0.246	0.187	0.164	0.76	0.67
0.02083	0.25	0.155	0.207	0.094	0.082	0.45	0.40

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

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Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
REACHS F-G & G-H (10,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 370  
 INPUT: topwidth of flow (ft)= 52.39  
 R/W= 7.1  
 INPUT: side slope (xH:1V)= 3  
 side slope angle (deg)= 18.4  
 INPUT: angle of repose (deg)= 40.5  
 INPUT: rock specific gravity= 2.8

INPUT: velocity (fps)= 5.930  
 INPUT: depth (ft)= 7.570

D-50		BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM	SIDE SLOPE
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	SF	SF
		(psf)	(psf)	(psf)	(psf)		
0.25	3.00	0.313	0.371	1.123	0.982	3.03	2.64
0.16667	2.00	0.274	0.325	0.749	0.654	2.30	2.01
0.08333	1.00	0.223	0.264	0.374	0.327	1.42	1.24
0.0625	0.75	0.206	0.244	0.281	0.245	1.15	1.01
0.04167	0.50	0.184	0.219	0.187	0.164	0.86	0.75
0.02083	0.25	0.155	0.184	0.094	0.082	0.51	0.44

RIPRAP SIZING USING 1970 COE METHOD

Ref: COE, 1970. Hydraulic Design of Flood Control Channels,  
EM 1110-2-1601, pp. 37 - 47.

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Date: 12/14/99

Location: SHERWOOD DIVERION DITCH (See Figure C.1)  
BELOW REACH H (10,000 YR STORM EVENT)

INPUT COEFFICIENTS: (see manual for description)

INPUT: radius of curv. (ft)= 705  
 INPUT: topwidth of flow (ft)= 50.58  
 R/W= 13.9  
 INPUT: side slope (xH:1V)= 3  
 side slope angle (deg)= 18.4  
 INPUT: angle of repose (deg)= 40.5  
 INPUT: rock specific gravity= 2.8  
  
 INPUT: velocity (fps)= 6.390  
 INPUT: depth (ft)= 7.260

D-50		BOUNDARY	BEND	BOTTOM	SIDE SLOPE	BOTTOM	SIDE SLOPE
(ft)	(in)	SHEAR	SHEAR	SHEAR	SHEAR	SF	SF
		(psf)	(psf)	(psf)	(psf)		
0.25	3.00	0.369	0.369	1.123	0.982	3.04	2.66
0.16667	2.00	0.323	0.323	0.749	0.654	2.32	2.03
0.08333	1.00	0.262	0.262	0.374	0.327	1.43	1.25
0.0625	0.75	0.241	0.241	0.281	0.245	1.16	1.02
0.04167	0.50	0.217	0.217	0.187	0.164	0.86	0.76
0.02083	0.25	0.182	0.182	0.094	0.082	0.51	0.45